NOTES

I. FOUNDATION AND SOIL REQUIREMENTS ARE BASED ON SOIL REPORT BY DAMES AND MOORE;
REPORT NO. 177-004-0016 DATED: FEBRUARY 10, 1993.

STRUCTURAL STEEL

FOUNDATIONS

- I. THE DESIGN, FABRICATION AND ERECTION OF ALL STRUCTURAL STEEL SHALL CONFORM TO AISC "MANUAL OF STEEL CONSTRUCTION" AND WITH THE SPECIFICATIONS.

 STRUCTURAL STEEL SHAPES & PLATES SHALL CONFORM TO ASTM A36
 UNLESS OTHERWISE NOTED ON PLAN.
- 2. PROVIDE FILLERS AT SPLICES OF PARTS HAVING MORE THAN $\frac{1}{16}$ " DIFFERENCE
- 3. ALL BEARING STIFFENER PLATES SHALL HAVE A CLOSE BEARING AGAINST THE INNER SURFACES OF BOTH FLANGES.

CONNECTIONS

- I. PLATE FOR BOLTED SHEAR PLATE CONNECTIONS SHALL BE THE SAME THICKNESS

 AS THE BEAM WEB WITH A MINIMUM THICKNESS OF & "UNLESS OTHERWISE NOTED.

 DIAGONAL GUSSET PLATE CONNECTIONS SHALL HAVE A MINIMUM THICKNESS OF

 UNLESS OTHERWISE NOTED) AND THE NET AREA THROUGH THE BOLTS HOLES SHALL

 DEVELOP TOTAL SHEAR CAPACITY OF THE BOLTS. ALL CONNECTIONS SHALL HAVE A MINIMUM

 OF TWO BOLTS. LOAD INDICATOR WASHERS SHALL BE USED WITH ALL ASTM A325SC BOLTS.
- 2. ALL BOLTS SHALL BE ASTM A325SC, CLASS A, UNLESS OTHERWISE NOTED. $\frac{3}{4}$ "Ø BOLTS SHALL BE USED FOR MID & END STATION, OSB BUILDING AND MAINTENANCE BUILDING AND $\frac{7}{8}$ "Ø BOLTS SHALL BE USED FOR CORNER LVEA BUILDING, UNLESS OTHERWISE NOTED.
- 3. ALL STIFFENERS SHALL HAVE A MINIMUM THICKNESS OF $\frac{3}{8}$ ", UNLESS OTHERWISE NOTED.
- 4. GIRT CONNECTIONS SHALL HAVE A MINIMUM OF $2-\frac{3}{4}$ " Ø ASTM A307 BOLTS.
- 5. ALL CONNECTIONS REQUIRE SLIP CRITICAL CONNECTION BOLTS EXCEPT GIRT CONNECTIONS.
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- 6. SPLICE CONNECTION FOR CRANE GIRDERS MAY BE AT ANY CONVENIENT LOCATION.

WELDING

- I. ALL WELDING SHALL CONFORM TO THE REQUIREMENTS OF AWS DI. 1-90 STRUCTURAL WELDING CODE AND THE SPECIFICATION.
- 2. ALL WELDING PROCEDURE SPECIFICATIONS AND WELDING PROCEDURE QUALIFICATIONS, WELDERS, AND WELDING OPERATORS SHALL BE FULLY QUALIFIED IN ACCORDANCE WITH AWS DI. 1-90.
- 3. LENGTHS OF WELDS SHOWN ARE EFFECTIVE LENGTHS AS SPECIFIED IN AISC SPECIFICATIONS. WHERE LENGTH OF WELD IS NOT SHOWN, IT SHALL BE FULL LENGTH OF JOINT. ALL BUTT WELDS SHALL BE FULL PENETRATION WELDS, UNLESS OTHERWISE NOTED.
- 4. ALL WELDING ELECTRODES SHALL BE E70XX.
- 5. WITH REFERENCE TO MINIMUM SIZE OF FILLET WELD REQUIREMENTS
 IN SECTION 1.17 OF AISC SPECIFICATIONS, MINIMUM SIZE OF FILLET
 WELDS WHEN NOT SPECIFIED ON WELD SYMBOLS SHALL BE AS FOLLOWS:

 \$\frac{1}{4}\$" WELD FOR MATERIAL THICKNESS UP TO AND INCLUDING \$\frac{3}{4}\$"

 \$\frac{5}{16}\$" WELD FOR MATERIAL THICKNESS OVER \$\frac{3}{4}\$" TO \$1\frac{1}{2}\$"
- 6. WELDING PROCEDURES AND SEQUENCES SHALL BE PLANNED TO MINIMIZE WELD SHRINKAGE THAT COULD RESULT IN LAMELLAR TEARING, AND APPROVED BY OWNER'S REPRESENTATIVE.
- 7. GRIND SMOOTH WELDED JOINTS WHERE FLUSH SURFACE IS REQUIRED.

METAL DECK

- I. ALL METAL DECKING SHALL BE IN ACCORDANCE WITH SECTION 5312 OF THE SPECIFICATIONS.
- 2. ROOF DECK SHALL HAVE SINGLE RIBS 3" DEEP AND MADE OUT OF 20 GAGE STEEL WITH MINIMUM I=0.837 IN⁴/FOOT OF WIDTH AND MINIMUM S(+)=0.508 IN ³/FOOT WIDTH. ATTACHMENT OF ROOF DECK TO SUPPORTS SHALL BE PER MANUFACTURER'S RECOMMENDATIONS. THE ATTACHMENT SHALL BE CAPABLE OF RESISTING 80 PSF NET UPLIFT AND 300 POUNDS PER LINEAR FOOT OF SHEAR.
- 3. ACOUSTICAL ROOF DECK OF SAME SECTION PROPERTIES AS INDICATED IN ITEM 2 ABOVE SHALL BE USED FOR THE ROOF OVER MULTIPURPOSE ROOM OF THE OSB BLDG AS INDICATED ON THE DWG.
- 4. ROOF DECK SHALL HAVE A MINIMUM OF TWO (2) SPANS UNLESS OTHERWISE NOTED.

CONCRETE

- I. ALL CONCRETE MATERIALS AND CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF DIVISION 3 (CONCRETE) OF THE SPECIFICATIONS. (REGULAR WEIGHT AND LIGHT WEIGHT)
- 2. STRUCTURAL CONCRETE SHALL HAVE AN ULTIMATE COMPRESSIVE STRENGTH
 OF 4000 PSI AT 28 DAYS UNLESS OTHERWISE NOTED. ALL CONCRETE SHALL BE
 REGULAR WEIGHT CONCRETE UNLESS OTHERWISE NOTED.

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3. ALL REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A615 GRADE 60 (UNLESS OTHERWISE NOTED).

SPLICES AND HOOKS SHALL CONFORM TO ACI 318-89, SPLICES SHALL BE CLASS B UNLESS OTHERWISE NOTED. MINIMUM LAP SHALL BE 30

DIAMETERS. STIRRUP AND TIE HOOKS SHALL HAVE 135-DEGREE BENDS.

- 4. LOCATION OF ALL CONSTRUCTION JOINTS OR OTHER TYPES OF JOINTS, OTHER THAN SPECIFIED, SHALL BE APPROVED BY THE CONSTRUCTION MANAGER BEFORE PLACING
- 5. MINIMUM CONCRETE COVER PROVIDED FOR REINFORCEMENT SHALL BE IN ACCORDANCE WITH THE SPECIFICATION.
- 6. ALL REINFORCING STEEL, ANCHOR BOLTS, DOWELS AND OTHER INSERTS SHALL BE WELL SECURED IN POSITION AND INSPECTED BY QUALIFIED INSPECTOR PRIOR TO PLACING CONCRETE.
- 7. EXISTING PAVEMENT SHALL BE SAW CUT AND BROKEN OUT TO CLEAN, STRAIGHT EDGES OF DEMOLITION AREAS.
- 8. EXPOSED EDGES OF CONCRETE SHALL HAVE A 3" CHAMFER UNLESS
- 9. EXCAVATING AND BACKFILLING SHALL BE IN ACCORDANCE WITH THE SPECIFICATIONS.
- 10. CONSTRUCTION JOINTS SHALL BE CLEANED AND ROUGHENED TO $\frac{1}{4}$ "AMPLITUDE.
- 11. PROVIDE REINFORCEMENT IN WALLS AT CORNER AND INTERSECTIONS
 AS PER DETAIL (1)
 LA-S-002

ANCHOR BOLTS

I. FOR ANCHOR BOLT DETAILS SEE DRAWING LA-S-003

OTHERWISE NOTED ON DRAWINGS.

MASONR

- I. WALLS SHALL BE LOAD BEARING REGULAR WEIGHT HOLLOW CONCRETE MASONRY UNITS WITH ALL CELLS GROUTED SOLID (UON),
 ACCORDING TO THE SPECIFICATIONS.
- 2. THE MORTAR SHALL BE CEMENT-LIME TYPE 'S' WITH A COMPRESSIVE STRENGTH OF 1800 PSI AT 28 DAYS. CONSISTING OF ONE PART OF CEMENT
- $\frac{1}{4}$ TO $\frac{1}{2}$ PART OF HYDRATED LIME OR LIME PUTTY AND DAMP LOOSE AGGREGATE $2\frac{1}{4}$ TO 3 TIMES THE SUM OF VOLUMES OF CEMENT AND LIME.
- $2\frac{1}{4}$ TO 3 TIMES THE SUM OF VOLUMES OF CEMENT AND LIME. THE GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI AT 28 DAYS.
- 3. THE MINIMUM CLEAR DISTANCE BETWEEN PARALLEL BARS SHALL BE THE BAR'S DIAMETER, BUT NOT LESS THAN I".
- 4. MINIMUM LAP OF REINFORCING STEEL SHALL BE 40 DIAMETERS, BUT MOT LESS THAN 2'-0".
- 5. IF WORK STOPPED FOR ONE HOUR OR MORE, PROVIDE A HORIZONTAL CONSTRUCTION JOINT BY STOPPING THE GROUT $l\frac{1}{2}$ " MINIMUM BELOW THE TOP OF MORTAR OR MASONRY.

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| SSUED FOR CONSTRUCTION | DRAWN | MCS | 11-15-96 | CHECKED | DDM | DDM | 11-15-96 | CHECKED | DDM | D

DESCRIPTION

BY CHKD ENGR PROJ

NO. DATE

DRAWING NO.

DESCRIPTION



100 WEST WALNUT STREET

PASADENA, CALIFORNIA



LASER INTERFEROMETER

GRAVITATIONAL-WAVE OBSERVATORY

SITE NO. 2 - LIVINGSTON, LOUISIANA

STRUCTURAL
GENERAL NOTES,
ABBREVIATIONS & LEGEND

NONE PPI50969 8094

HEET NUMBER REVISION

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