

CALIFORNIA INSTITUTE OF TECHNOLOGY
MASSACHUSETTS INSTITUTE OF TECHNOLOGY

LIGO



PROJECT

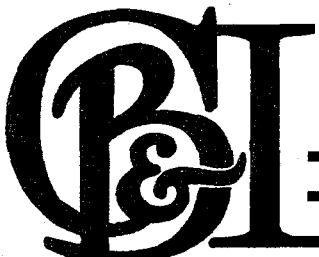
**FINAL DESIGN REVIEW
DATA PACKAGE
BEAM TUBE MODULE
DESIGN & QUALIFICATION TEST
CONTRACT C146**

**CDRL #15
DRD #9 ITEM IV
DESIGN CALCULATIONS & ANALYSES**

Prepared by

**CBI TECHNICAL SERVICES COMPANY
PLAINFIELD, ILLINOIS
APRIL 11, 1994**

CBI CONTRACT 930212



LIGO BEAM TUBE MODULES
TABLE OF CONTENTS FOR DETAILED DESIGN CALCULATIONS

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SUBJECT LIGO Beam Tube Modules Detailed Design Calculations Table of Contents	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY WJC	CHKD BY RJW	MADE BY	CHKD BY	SHT 1 OF 2
	DATE 4/5/94	DATE 4/5/94	DATE	DATE	

LIGO BEAM TUBE MODULES
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IN LIGHT OF CAUTIONARY NOTE OF ASME VIII DIV.1 UG-29 FOR LATERAL BUCKLING OF STIFFENER RINGS, APPLY AISC TYPE CRITERIA FOR A DEPTH TO THICKNESS LIMIT:

FROM AISC 9TH EDITION OF MANUAL OF STEEL CONSTRUCTION, TABLE B5.1, LIMIT TO $95/\sqrt{F_y}$.

$$\frac{b}{t} = \frac{1.75}{.1875} = 9.3 < \frac{95}{\sqrt{F_y}} = \frac{95}{\sqrt{19.5}} = 21.5 \quad \text{OK}$$

SUBJECT LIGO BEAM TUBES STIFFENER RINGS	CBI OFFICE NOE		REVISION		REFERENCE NO. 930212
	MADE BY JEH	CHKD BY RSW	MADE BY	CHKD BY	SHT 1 OF 1
	DATE 11/3/93	DATE 4 NOV 93	DATE	DATE	2.1

DESIGN OF VACUUM VESSELS WITH CIRCUMFERENTIAL STIFFENERS DAVID TAYLOR MODEL BASIN PROCEDURE & MODIFIED ASME DIVISION II

Purpose: This calculation shows the required spacing of the 1.75" x 0.1875" beam tube vacuum stiffeners designed PER THE ASME CODE with a Factor of Safety of 3.00, in order to obtain an allowable design pressure ≥ 14.70 psi, with a < 1 percent permissible variation in the actual and required moments of inertia of the stiffener / shell section. The moment of inertia of the stiffener accounts for the 6 degree rotation that can be expected during construction.

Input Variables, Cylinder

Cylinder O.D, Dc	<u>49.00</u>	Inches
Cylinder Thickness, tc	<u>0.127</u>	Inches
Design Pressure, p	<u>14.70</u>	psi
Design Temperature	<u>300</u>	degrees F
Safety Factor, FS	<u>3.00</u>	ASME Safety Factor

Input Variables, Stiffener

Stiffener Depth, dstiff	<u>1.750</u>	Inches
Stiffener Thick, tstiff	<u>0.1875</u>	Inches
Stiffener Spacing, L	<u>26.500</u>	Inches

Computed Circumferential Stress in Cylinder

Sc = 2,836 psi ---> $(p * Dc) / (2 * tc)$

Determine Tangent Modulus of the Material, Et

Et = 27,000,000 psi ---> (For 304L @ 300F, Table TM-1, p664, ASME Section II Part D)

Determine Collapsing Pressure, Pcr

Pcr = 46.36 psi ---> $(2.6 * Et * ((tc/Dc)^{2.5})) / (L/Dc - 0.45 * ((tc/Dc)^{0.5}))$

Determine Maximum Allowable Pressure, Pall = Pcr / FS

Pall = 15.45 psi ---> Pcr / FS

Determine Maximum Allowable Spacing Between Stiffeners

Lmax = 27.80 inches ---> $Dc * (((tc/Dc)^{0.5}) * (0.45 + ((2.6 * Et)/(FS * p)) * ((tc/Dc)^{0.5})))$

Determine Maximum Allowable Circumferential Stress, Sc_all

Sc_all = 2,981 psi ---> $((2.6/(FS * 2)) * Et * ((tc/Dc)^{1.5})) / ((L/Dc) - 0.45 * ((tc/Dc)^{0.5}))$

Determine Corresponding Minimum Thickness of Cylinder, tc_min

tc_min = 0.051 inches ---> $2 * (Dc/2) * ((2 * p/Et)^{0.5})$

Intermediate Vacuum Stiffeners

A_stiff =	0.328	in**2	---> dstiff * tstiff
A_shell =	0.348	in**2	---> $1.1 * tc * \text{Sqrt}(Dc * tc)$
A_total =	0.677	in**2	---> A_stiff + A_shell
ybar =	0.519	inches abv shell	---> $(A_stiff * (dstiff/2 + tc) + A_shell * (tc/2)) / A_total$
I_stiff =	0.083	in**4	---> Moment of Inertia of stiffener with a 6 degree incline from the vertical.
I_shell =	0.0005	in**4	---> $(1/12) * (1.1 * \text{Sqrt}(Dc * tc)) * tc^3$
I_tot =	0.232	in**4	---> $I_stiff + I_shell + A_stiff * (dstiff/2 + tc - ybar)^2 + A_shell * (ybar - tc/2)^2$
B_stiff =	3,876	psi	---> $0.75 * (p * Dc) / (tc + A_stiff / L)$
A_value =	0.000287		---> $(2 * B_stiff) / Et$
Req'd I_tot =	0.234	in**4	---> $((Dc^2 * L * (tc + A_stiff / L) * A_value) / 10.9) * (FS / 3)$, ASME Div 2 UG-29

Conclusion: For a design meeting the ASME Code with a Factor of Safety of 3.00, the vacuum stiffeners must be spaced no farther than 26.50 inches apart for an allowable design pressure ≥ 14.70 psi, assuming a < 1 percent variation in the actual and required moments of inertia of the stiffener / shell section.

SUBJECT LIGO BEAM TUBE VACUUM STIFFENERS SPACING, ALLOWABLE PRESSURE, AND SIZE WITH ASME & NON-ASME SAFETY FACTORS	OFFICE: NOE		REVISION:		REFERENCE NO. 930212
	MADE BY WJC	CHKD Rjw	MADE BY	CHKD	SHT 1 OF 4
	DATE 3/11/94	DATE 18 MAR 94	DATE	DATE	2.2

DESIGN OF VACUUM VESSELS WITH CIRCUMFERENTIAL STIFFENERS DAVID TAYLOR MODEL BASIN PROCEDURE & MODIFIED ASME DIVISION II

Purpose: This calculation shows the required spacing of the 1.75" x 0.1875" beam tube vacuum stiffeners designed PER THE ASME CODE with a Factor of Safety of 3.00, in order to obtain an allowable design pressure \geq 14.70 psi, with an accepted 5 percent variation in the actual and required moments of inertia of the stiffener / shell section. The moment of inertia of the stiffener accounts for the 6 degree rotation that can be expected during construction.

Input Variables, Cylinder

Cylinder O.D, Dc	49.00	Inches
Cylinder Thickness, tc	0.127	Inches
Design Pressure, p	14.70	psi
Design Temperature	300	degrees F
Safety Factor, FS	3.00	ASME Safety Factor

Input Variables, Stiffener

Stiffener Depth, dstiff	1.750	Inches
Stiffener Thick, tstiff	0.1875	Inches
Stiffener Spacing, L	27.750	Inches

Computed Circumferential Stress in Cylinder

Sc = 2,836 psi $\rightarrow (p * Dc) / (2 * tc)$

Determine Tangent Modulus of the Material, Et

Et = 27,000,000 psi \rightarrow (For 304L @ 300F, Table TM-1, p664, ASME Section II Part D)

Determine Collapsing Pressure, Pcr

Pcr = 44.18 psi $\rightarrow (2.6 * Et * ((tc/Dc)^{2.5})) / (L/Dc - 0.45 * ((tc/Dc)^{0.5}))$

Determine Maximum Allowable Pressure, Pall = Pcr / FS

Pall = 14.73 psi $\rightarrow Pcr / FS$

Determine Maximum Allowable Spacing Between Stiffeners

Lmax = 27.80 inches $\rightarrow Dc * ((tc/Dc)^{0.5}) * (0.45 + ((2.6 * Et)/(FS * p)) * ((tc/Dc)^2))$

Determine Maximum Allowable Circumferential Stress, Sc all

Sc_all = 2,841 psi $\rightarrow ((2.6/(FS * 2)) * Et * ((tc/Dc)^{1.5})) / ((L/Dc) - 0.45 * ((tc/Dc)^{0.5}))$

Determine Corresponding Minimum Thickness of Cylinder, tc_min

tc_min = 0.051 inches $\rightarrow 2 * (Dc/2) * ((2 * p/Et)^{0.5})$

Intermediate Vacuum Stiffeners

A_stiff =	0.328	in**2	$\rightarrow dstiff * tstiff$
A_shell =	0.348	in**2	$\rightarrow 1.1 * tc * Sqrt(Dc * tc)$
A_total =	0.677	in**2	$\rightarrow A_stiff + A_shell$
ybar =	0.519	inches abv shell	$\rightarrow (A_stiff * (dstiff/2 + tc) + A_shell * (tc/2)) / A_total$
I_stiff =	0.083	in**4	\rightarrow Moment of Inertia of stiffener with a 6 degree incline from the vertical.
I_shell =	0.0005	in**4	$\rightarrow (1/12) * (1.1 * Sqrt(Dc * tc)) * tc^3$
I_tot =	0.232	in**4	$\rightarrow I_stiff + I_shell + A_stiff * (dstiff/2 + tc - ybar)^2 + A_shell * (ybar - tc/2)^2$
B_stiff =	3,891	psi	$\rightarrow 0.75 * (p * Dc) / (tc + A_stiff / L)$
A_value =	0.000288		$\rightarrow (2 * B_stiff) / Et$
Req'd I_tot =	0.245	in**4	$\rightarrow ((Dc^2 * L * (tc + A_stiff / L) * A_value) / 10.9) * (FS / 3)$, ASME Div 2 UG-29

Conclusion: For a design meeting the ASME Code with a Factor of Safety of 3.00, the vacuum stiffeners must be spaced no farther than 27.75 inches apart for an allowable design pressure of 14.70 psi, assuming an accepted 5 percent variation in the actual and required moments of inertia of the stiffener / shell section.

SUBJECT LIGO BEAM TUBE VACUUM STIFFENERS SPACING, ALLOWABLE PRESSURE, AND SIZE WITH ASME & NON-ASME SAFETY FACTORS	OFFICE: NOE		REVISION:		REFERENCE NO. 930212
	MADE BY WJC	CHKD RJSW	MADE BY	CHKD	SHT 2 OF 4
	DATE 3/11/94	DATE 10/22/94	DATE	DATE	2.3

**DESIGN OF VACUUM VESSELS WITH CIRCUMFERENTIAL STIFFENERS
DAVID TAYLOR MODEL BASIN PROCEDURE & MODIFIED ASME DIVISION II**

Purpose: This calculation shows the resulting Factor of Safety (Non-ASME) that is obtained by spacing the beam tube vacuum stiffeners (1.75" x 0.1875") at 30", yet still obtaining an allowable design pressure ≥ 14.70 psi, with a < 1 percent permissible variation in the actual and required moments of inertia of the stiffener / shell section.
The moment of Inertia of the stiffener accounts for the 6 degree rotation that can be expected during construction.

Input Variables, Cylinder

Cylinder O.D, Dc 49.00 inches
 Cylinder Thickness, tc 0.127 inches
 Design Pressure, p 14.70 psi
 Design Temperature 300 degrees F
 Safety Factor, FS 2.65 Non-ASME Safety Factor

Input Variables, Stiffener

Stiffener Depth, dstiff 1.750 inches
 Stiffener Thick, tstiff 0.1875 inches
 Stiffener Spacing, L 30.000 inches

Computed Circumferential Stress in Cylinder

Sc = 2,836 psi ---> $(p * Dc) / (2 * tc)$

Determine Tangent Modulus of the Material, Et

Et = 27,000,000 psi ---> (For 304L @ 300F, Table TM-1, p664, ASME Section II Part D)

Determine Collapsing Pressure, Pcr

Pcr = 40.74 psi ---> $(2.6 * Et * ((tc/Dc)^{2.5})) / (L/Dc - 0.45 * ((tc/Dc)^{0.5}))$

Determine Maximum Allowable Pressure, Pall = Pcr / FS

Pall = 15.37 psi ---> Pcr / FS

Determine Maximum Allowable Spacing Between Stiffeners

Lmax = 31.32 inches ---> $Dc * ((tc/Dc)^{0.5}) * (0.45 + ((2.6 * Et)/(FS * p)) * ((tc/Dc)^2))$

Determine Maximum Allowable Circumferential Stress, Sc_all

Sc_all = 2,966 psi ---> $((2.6/(FS * 2)) * Et * ((tc/Dc)^{1.5})) / ((L/Dc) - 0.45 * ((tc/Dc)^{0.5}))$

Determine Corresponding Minimum Thickness of Cylinder, tc_min

tc_min = 0.051 inches ---> $2 * (Dc/2) * ((2 * p/Et)^{0.5})$

Intermediate Vacuum Stiffeners

A_stiff = 0.328 in**2 ---> dstiff * tstiff
 A_shell = 0.348 in**2 ---> $1.1 * tc * \text{Sqrt}(Dc * tc)$
 A_total = 0.677 in**2 ---> A_stiff + A_shell
 ybar = 0.519 inches abv shell ---> $(A_stiff * (dstiff/2 + tc) + A_shell * (tc/2)) / A_total$
 I_stiff = 0.083 in**4 ---> Moment of Inertia of stiffener with a 6 degree incline from the vertical.
 I_shell = 0.0005 in**4 ---> $(1/12) * (1.1 * \text{Sqrt}(Dc * tc)) * tc^3$
 I_tot = 0.232 in**4 ---> $I_stiff + I_shell + A_stiff * (dstiff/2 + tc - ybar)^2 + A_shell * (ybar - tc/2)^2$
 B_stiff = 3,916 psi ---> $0.75 * (p * Dc) / (tc + A_stiff / L)$
 A_value = 0.000290 ---> $(2 * B_stiff) / Et$
 Req'd I_tot = 0.234 in**4 ---> $((Dc^2 * L * (tc + A_stiff / L) * A_value) / 10.9) * (FS / 3)$, ASME Div 2 UG-29

Conclusion: With a 30" spacing of the beam tube vacuum stiffeners, a Factor of Safety of 2.65 will result in a design that provides both an allowable design pressure ≥ 14.70 psi and variation of < 1 percent in the actual and required moments of inertia of the stiffener / shell section. This design does not conform to the requirements of the ASME Code.

SUBJECT LIGO BEAM TUBE VACUUM STIFFENERS SPACING, ALLOWABLE PRESSURE, AND SIZE WITH ASME & NON-ASME SAFETY FACTORS	OFFICE: NOE		REVISION:		REFERENCE NO. 930212
	MADE BY WJC	CHKD RWJ	MADE BY	CHKD	SHT 3 OF 4
	DATE 3/10/94	DATE 12/1/94	DATE	DATE	2.4

**DESIGN OF VACUUM VESSELS WITH CIRCUMFERENTIAL STIFFENERS
DAVID TAYLOR MODEL BASIN PROCEDURE & MODIFIED ASME DIVISION II**

Purpose: This calculation shows the resulting Factor of Safety (Non-ASME) that is obtained by spacing the beam tube vacuum stiffeners (1.75" x 0.1875") at 30", yet still obtaining an allowable design pressure ≥ 14.70 psi, with an accepted 5 percent variation in the actual and required moments of inertia of the stiffener / shell section.
The moment of inertia of the stiffener accounts for the 6 degree rotation that can be expected during construction.

Input Variables, Cylinder

Cylinder O.D, Dc 49.00 Inches
 Cylinder Thickness, tc 0.127 Inches
 Design Pressure, p 14.70 psi
 Design Temperature 300 degrees F
 Safety Factor, FS 2.77 Non-ASME Safety Factor

Input Variables, Stiffener

Stiffener Depth, dstiff 1.750 inches
 Stiffener Thick, tstiff 0.1875 inches
 Stiffener Spacing, L 30.000 inches

Computed Circumferential Stress in Cylinder

Sc = 2,836 psi ---> $(p * Dc) / (2 * tc)$

Determine Tangent Modulus of the Material, Et

Et = 27,000,000 psi ---> (For 304L @ 300F, Table TM-1, p664, ASME Section II Part D)

Determine Collapsing Pressure, Pcr

Pcr = 40.74 psi ---> $(2.6 * Et * ((tc/Dc)^{2.5})) / (L/Dc - 0.45 * ((tc/Dc)^{0.5}))$

Determine Maximum Allowable Pressure, Pall = Pcr / FS

Pall = 14.71 psi ---> Pcr / FS

Determine Maximum Allowable Spacing Between Stiffeners

Lmax = 30.01 inches ---> $Dc * ((tc/Dc)^{0.5}) * (0.45 + ((2.6 * Et)/(FS * p)) * ((tc/Dc)^2))$

Determine Maximum Allowable Circumferential Stress, Sc_all

Sc_all = 2,837 psi ---> $((2.6/(FS * 2)) * Et * ((tc/Dc)^{1.5})) / ((L/Dc) - 0.45 * ((tc/Dc)^{0.5}))$

Determine Corresponding Minimum Thickness of Cylinder, tc_min

tc_min = 0.051 inches ---> $2 * (Dc/2) * ((2 * p/Et)^{0.5})$

Intermediate Vacuum Stiffeners

A_stiff = 0.328 in**2 ---> dstiff * tstiff
 A_shell = 0.348 in**2 ---> $1.1 * tc * \text{Sqrt}(Dc * tc)$
 A_total = 0.677 in**2 ---> A_stiff + A_shell
 ybar = 0.519 inches abv shell ---> $(A_stiff * (dstiff/2 + tc) + A_shell * (tc/2)) / A_total$
 I_stiff = 0.083 in**4 ---> Moment of Inertia of stiffener with a 6 degree incline from the vertical.
 I_shell = 0.0005 in**4 ---> $(1/12) * (1.1 * \text{Sqrt}(Dc * tc)) * tc^3$
 I_tot = 0.232 in**4 ---> I_stiff + I_shell + A_stiff * (dstiff/2 + tc - ybar)^2 + A_shell * (ybar - tc/2)^2
 B_stiff = 3,916 psi ---> $0.75 * (p * Dc) / (tc + A_stiff / L)$
 A_value = 0.000290 ---> $(2 * B_stiff) / Et$
 Req'd I_tot = 0.244 in**4 ---> $((Dc^2 * L * (tc + A_stiff / L) * A_value) / 10.9) * (FS / 3)$, ASME Div 2 UG-29

Conclusion: With a 30" spacing of the beam tube vacuum stiffeners, a Factor of Safety of 2.77 will result in a design that provides both an allowable design pressure ≥ 14.70 psi and variation of < 5 percent in the actual and required moments of inertia of the stiffener / shell section. This design does not conform to the requirements of the ASME Code.

SUBJECT LIGO BEAM TUBE VACUUM STIFFENERS SPACING, ALLOWABLE PRESSURE, AND SIZE WITH ASME & NON-ASME SAFETY FACTORS	OFFICE: NOE		REVISION:		REFERENCE NO. 930212
	MADE BY WJC	CHKD R	MADE BY	CHKD	SHT 4 OF 4
	DATE 3/11/94	DATE 10/11/94	DATE	DATE	2.5

BEAM TUBE VACUUM STIFFENERS: ROTATION DURING WELDING

Purpose

Determine the reduction in the moment of inertia of a vacuum stiffener that rotates a maximum of 8 degrees from the vertical when welded to the beam tube. Consider the effect upon the total moment of inertia of the combined stiffener / shell section. The effect of the rotation upon the location of the C.G. of the stiffener is very minor and shall be neglected.

Stiffener Size & Area

1.75" deep x 0.1875" thick 304L bar,

==> $d = 1.750$ in

==> $b = 0.188$ in

==> $A_s = 0.328$ in²

==> α = angle of rotation of stiffener from vertical

Moment of Inertia of Rotated Stiffener, I_{st}

$I_{st} = b * d * ((b^2 * \sin(\alpha)^2) + (d^2 * \cos(\alpha)^2)) / 12$

(AISC Manual, "Properties of Geometric Sections")

Effective Width and Area of Beam Tube Shell

Shell Thickness, t ==> 0.127 in

Shell Outside Diameter, D_o ==> 49.00 in

Effect Width, $L_e = 1.10 * [(D_o * t)^{0.5}]$ ==> 2.744 in

Effective Area, $A_e = L_e * t$ ==> 0.348 in²

Moment of Inertia, $I_{sh} = L_e * t^3 / 12$ ==> 0.0005 in⁴

For the Combined Stiffener / Shell Section

Area, $A_t = 0.677$ in²

$y_{bar} = 0.519$ in above bottom of shell

Moment of Inertia, $I_{comb} = I_{st} + I_{sh} + [A_s * (d/2 + t - y_{bar})^2] + [A_e * (y_{bar} - t/2)^2]$

Stiffener Rotation, α	0 deg	2 deg	4 deg	6 deg	8 deg
Stiffener Inertia, I_s (in ⁴)	0.0837	0.0836	0.0833	0.0828	0.0821
Combined Inertia, I_{comb} (in ⁴)	0.2331	0.2325	0.2322	0.2317	0.2310
Change from $\alpha = 0$ deg	0.00 %	-0.24 %	-0.37 %	-0.59 %	-0.89 %

<==== For the stiffener only

<==== For the stiffener / shell section

==> **0.90 % maximum reduction in moment of inertia of combined section, NEGLIGIBLE**

SUBJECT LIGO BEAM TUBE VACUUM STIFFENERS REDUCTION IN INERTIA DUE TO ROTATION	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD <i>RW</i>	MADE BY	CHKD BY	SHT 1 OF 1
	DATE 3/10/94	DATE <i>19 MAR 94</i>	DATE	DATE	<i>2,6</i>

ATTACHMENT OF STIFFENER RINGS TO TUBE PER UG-30.

USE $\frac{1}{8}$ " FILLET WELD LEGS (MINIMUM PER UG-30(f))

ALLOWABLE FOR FILLET WELD PER UW-18(d) =
 $0.55 \times \text{AREA} \times S$

FOR A 1" LONG, $\frac{1}{8}$ " FILLET, $\text{AREA} = \frac{1}{8} \text{ in}^2$

$$\text{ALLOWABLE} = 0.55 \times \frac{1}{8} \frac{\text{in}^2}{\text{in}} \times 15,400 \text{ psi} = 1059 \text{ \#/in.}$$

ATTACHMENT WELDS ARE SIZED PER UG-30(e) TO RESIST:

- FULL RADIAL PRESSURE LOAD FROM SHELL BETWEEN STIFFENERS, PL_s
- RADIAL SHEAR COMPUTED AS 2% OF RING COMPRESSIVE LOAD, $0.02 PL_s R_o$
- SHEAR LOADS ACTING RADIALLY ACROSS STIFFENER (NOT APPLICABLE)

ASSUME A MAXIMUM VALUE OF $L_s = 28"$

$$PL_s = 14.7 \text{ psi} (28 \text{ in}) = 412 \text{ \#/in RADIAL}$$

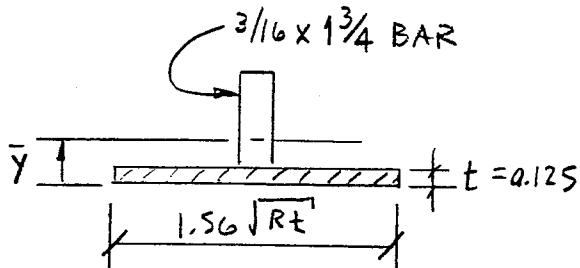
$$0.02 PL_s R_o = 0.02 (14.7 \text{ psi}) (28 \text{ in}) (24.125 \text{ in}) = 199 \text{ \# RADIAL}$$

SUBJECT LIGO BEAM TUBES STIFFENER RINGS	OFFICE CBI NoE		REVISION		REFERENCE NO. 930212
	MADE BY JEN	CHKD BY [Signature]	MADE BY	CHKD BY	SHT 1 OF 3
	DATE 11/3/93	DATE 4/20/95	DATE	DATE	2.7

PARTICIPATING WIDTH OF TUBE =

$$1.56 \sqrt{Rt} = 1.56 \sqrt{24 (.125)} = 2.702 \text{ in}$$

DETERMINE Q, FIRST MOMENT OF AREA:



$$Q = A_{\text{TUBE}} \left(\bar{y} - \frac{t}{2} \right)$$

$$\bar{y} = 0.5246 \text{ in (COMBINED SECTION)}$$

$$Q = 2.702 \times 0.125 \left(0.5246 - \frac{0.125}{2} \right) = 0.156 \text{ in}^3$$

$$\frac{VQ}{I} = \frac{199 \# (0.156 \text{ in}^3)}{0.230 \text{ in}^4} = 135 \#/\text{in. TANGENTIAL}$$

COMBINED LOAD ON WELD:

$$\sqrt{412^2 + 135^2} = 434 \#/\text{in}$$

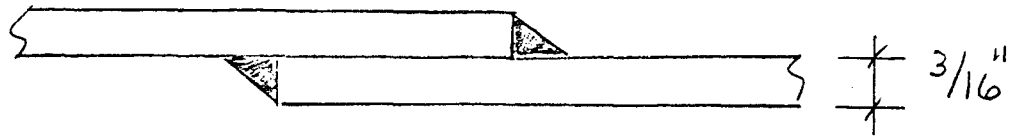
% OF CIRCUMFERENCE WELDED REQ'D FOR ONE-SIDED =

$$\frac{434}{1059} \times 100 = 41\%$$

∴ CONTINUOUS 1/8" FILLET ON ONE SIDE ONLY IS ACCEPTABLE (SOME SMALL LENGTH WILL REMAIN UNWELDED WHERE JOINT IN STIFFENER CROSSES TUBE SEAM)

SUBJECT LIGO BEAM TUBES STIFFENER RINGS	OFFICE CBI NOE		REVISION		REFERENCE NO. 930212
	MADE BY JEH	CHKD BY RWS	MADE BY	CHKD BY	SHT 2 OF 3
	DATE 11/3/93	DATE 6/22/93	DATE	DATE	2.8

EVALUATE LAPPED ATTACHMENT OF STIFFENER ENDS:



ASSUME LOAD TO BE TRANSMITTED ACROSS JOINT IS 15,400 psi MULTIPLIED BY THE FULL STIFFENER AREA

$$\text{LOAD} = 15,400 \text{ psi} \times \frac{3}{16} \times 1.75 = 5053 \#$$

ASSUME FULL THICKNESS FILLETS, $1\frac{1}{2}$ " LONG ARE USED:

$$\text{ALLOWABLE LOAD} = 0.55 \times 15,400 \text{ psi} \times 2(1.5) \times .1875 = 4764 \#$$

$4764 < 5053 \#$ REQ'D \therefore ADD'L WELD IS REQ'D.

DETERMINE LENGTH OF GROOVE WELD ACROSS TOP WHICH WILL SUPPLY THE NECESSARY ADD'L STRENGTH



USE BUTT GROOVE W/O EDGE PREPARATION. ASSUME $\frac{1}{16}$ " PENETRATION. USE A 60% GROOVE WELD EFFICIENCY FOR SHEAR IN ACCORDANCE WITH UW-15(C). IF LAP OF STIFFENERS IS SPECIFIED AS A NOMINAL $1" \pm \frac{1}{2}"$, THE SHORTEST LENGTH IS $\frac{1}{2}"$:

$$.60 \times 15,400 \text{ psi} \times \frac{1}{16} \times \frac{1}{2} = 289 \#$$

$$4764 + 289 = 5053 \# \leq 5053 \# \text{ REQ'D OK}$$

SUBJECT LIGO BEAM TUBES STIFFENER RINGS	OFFICE CBI NoE		REVISION		REFERENCE NO. 930212
	MADE BY JEH	CHKD BY R...	MADE BY	CHKD BY	SHT 3 OF 3
	DATE 11/3/93	DATE 8...	DATE	DATE	2.9

MAXIMUM UN SUPPORTED GAP

$$D_o = 49.004$$

$$t = 0.127$$

$$L = 28.35$$

$$D_o/t = \frac{49.004}{0.127} = 385.9$$

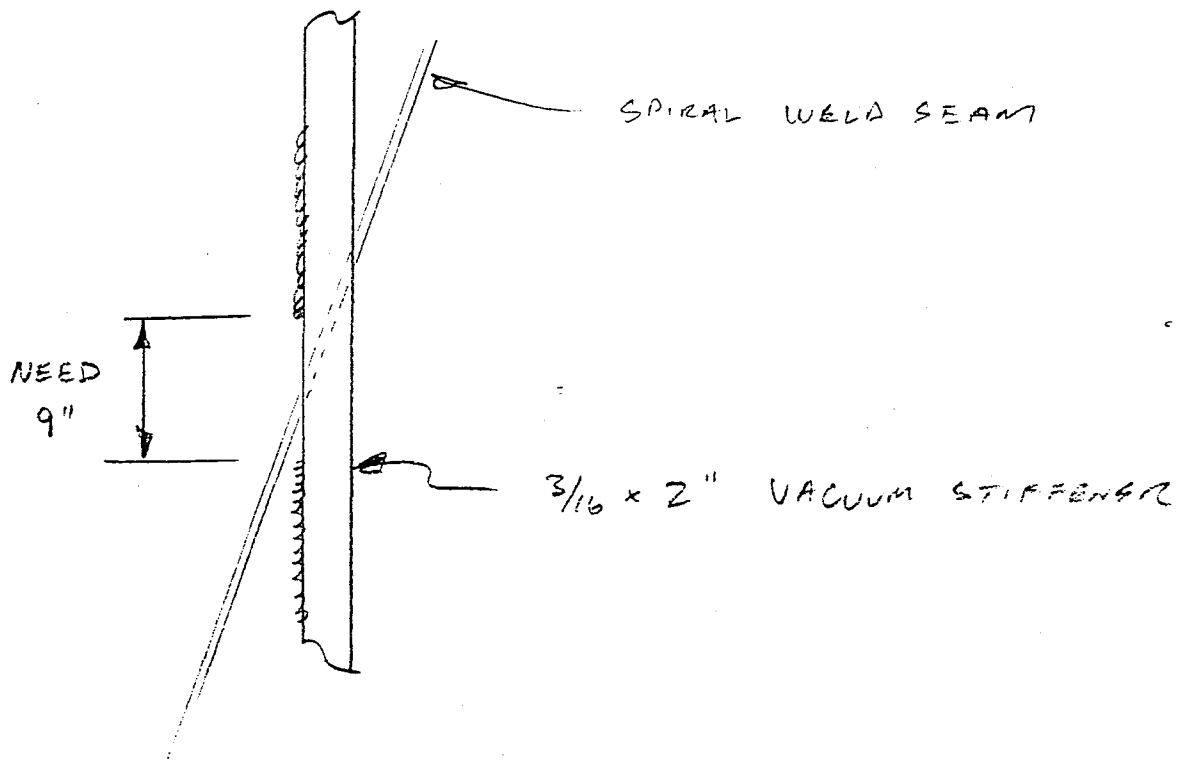
$$L/D_o = \frac{28.35}{49.004} = 0.578$$

FIG UG 29.2

$$ARC = 0.085 D_o = 0.085(49.004) = 4.165 \rightarrow 4 \frac{3}{16}''$$

9" IS PREFERRED

SUBJECT L160 BEAM TUBE MAX GAP OF ST. BEAM HOLE	OFFICE CBI NCB-6		REVISION		REFERENCE NO. 930212
	MADE BY RJJ	CHKD BY WJC	MADE BY	CHKD BY	SHT. 1 OF 3
	DATE 3/23/93	DATE 4-3-94	DATE	DATE	2.10



- PER UG-29 / FIGURE UG 29.2

MAX UNSUPPORTED LENGTH $4\frac{3}{16}$

- LOOK @ TRUE BUCKLING STRENGTH

$$P_{CR} \text{ \#/IN} = \frac{EI(N^2-1)}{L^3}$$

$$= \frac{27.1 \times 10^6 (0.000469)(37^2-1)}{(4.8877/2)^3}$$

$$= 1191 \text{ \#/IN}$$

$$1.56(4.8877/2 \times 1.27)^5 = 2.748''$$

$$E = 27,100,000 \text{ PSI}$$

$$I = \frac{2.478(.127)^3}{12} = 0.000469$$

$$N = \frac{49.5287}{4.165} = 36.96 \Rightarrow 37$$

(TO COMPARE TO ASME)

$$PRESSURE = \frac{1191}{29.35''} = 42.02 \text{ PSI}$$

$$\frac{42.02}{14.7} = FS = 2.86 \approx 3.0$$

SUBJECT	OFFICE CBI NOE-C		REVISION		REFERENCE NO. 930214
	MADE BY RJV	CHKD BY WJC	MADE BY	CHKD BY	SHT <u>2</u> OF <u>3</u>
	DATE 3 Feb 94	DATE 4-3-94	DATE	DATE	2.11

- TRY 9"

$$N = \frac{49.004\pi}{9} = 17.1 \Rightarrow 17$$

$$P_{CR} = \frac{27.1 \times 10^6 (.000469)(17^2 - 1)}{(48.877/2)^3} = 251 \text{ #/IN}$$

$$PRESSURE = \frac{251}{28.35 \text{ STIFF SPA}} = 8.85 \text{ psi} < 14.7 \text{ N.G.}$$

- DETERMINE MAXIMUM UNWIND LENGTH W/ FS=2.5

$$P_{CR} = 2.5(14.7)(28.35) = 1042$$

$$N = \sqrt{1 + \frac{R^3 P_{CR}}{EI}} = \sqrt{1 + \frac{(48.877/2)^3 (1042)}{27.1 \times 10^6 (.000469)}}$$

$$N = 34.6 \Rightarrow 34$$

$$S = \frac{49.004\pi}{N} = 4.5''$$

- MAXIMUM UNWIND LENGTH W/ FS=1.5

$$P_{CR} = 1.5(14.7)(28.35) = 625$$

$$N = \sqrt{1 + \frac{(48.877/2)^3 (625)}{27.1 \times 10^6 (.000469)}} = 26.8 \Rightarrow 27$$

$$S = \frac{49.004\pi}{27} = 5.70''$$

SUBJECT	OFFICE CBI <u>NOA C</u>		REVISION		REFERENCE NO. 930212
	MADE BY <u>RJA</u>	CHKD BY <u>WJC</u>	MADE BY	CHKD BY	SHT <u>3</u> OF <u>3</u>
	DATE 3 FEB 93	DATE 4.3.94	DATE	DATE	2.12

INERTIA OF EXPANSION JOINT

- ONE CONVOLUTE

C-5 EJMA

$$I_{1-1} = N \left[\frac{nt(Zw-g)^3}{48} + 0.49nt(w-0.2g)^2 \right] \quad (C-49)$$

$$N = 1$$

$$n = 1$$

$$t = 0.105$$

$$w = 2.5 - 0.105 = 2.395$$

$$g = 1''$$

$$I = 1 \left[\frac{(1)(.105)(2(2.395) - 1)^3}{48} + 0.4(1)(1)(.105)(2.395 - 0.2(1))^2 \right]$$

$$I_{11} = 1 [0.119 + 0.2002] = .321 \text{ in}^4 > I_{REQ'D} \text{ PER ASME}$$

FOR 9 CONVOLUTES

$$I_{11} = 9(.321) = 2.89 \text{ in}^4$$

CONCLUSION

THE CONVOLUTION OF THE EXPANSION JOINT WILL ACT AS A VACUUM STIFFENER. THE MAXIMUM SPACING CAN BE BASED ON A 0.105 THICKNESS

SUBJECT MOMENT OF INERTIA - EXP'N JT. CONVOLUTE	OFFICE CBI NEE-L		REVISION		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WIC	MADE BY	CHKD BY	SHT <u>1</u> OF <u>1</u>
	DATE 1-20-74	DATE 4-4-74	DATE	DATE	2.13

PUMP PORT NECK THICKNESS

ASME VIII DIV 1
UG-28

Assume:

$t = 0.025$

$L = 3''$ (MAXIMUM NECK LENGTH)

$\phi = 10''$

$D/t = 10'' / 0.025 = 400$

$L/D = 3 / 10 = 0.3$

$A = 0.0006$

FIG 6 PART D
SUBPART 3

$B = 5000$

FIG NA-3

$P = \frac{4B}{3(A/t)} = \frac{4(5000)}{3(400)} = 16.7 \text{ psi} > 14.7$

MAXIMUM OD

FOR EFFECTIVE LIMITS (UG-40) d FROM ϕ OF NOZZLE : OD = 20''

SPACING FROM STIFFENERS 6'' OR $8t$ STD 9301-2

STIFFENER SPACING = 27.7'' SAY 27.5''

$\text{MAX OD} = 27.5 - 3/16 \text{ THICK} - 2(1/8 \text{ FILLETS}) - 2(6'' \text{ SPACING}) = 15.0625''$

THICKNESS REQUIRED

$t_{\text{REQ'D}} = \frac{0.500 \text{ IN}^2 \text{ (AREA REQ'D)}}{15'' - (10 + 2 \times 1.5)} = 0.25''$

SUBJECT 10" ϕ PUMP PORT 2" 60 BEHN TUBER	OFFICE CBI 1100-C		REVISION		REFERENCE NO. 930212
	MADE BY RJM	CHKD BY BTS	MADE BY	CHKD BY	SHT 1 OF 11
	DATE 1 NOV 93	DATE 10 NOV 93	DATE	DATE	3.1

CHECK WELD SIZE BETWEEN NOZZLE & SWELL

- LOAD ON NOZZLE

DUE TO EXTERNAL PRESSURE = $12.7 \times T_r(s)^2 = 1134 \#$

$f = \frac{1134\#}{T_r(10)} = 367 \#/\text{in.}$

DUE TO NOZZLE END LOADS 150# SAY 4" FROM SWELL

$f = \frac{M}{S} = \frac{4(150\#)}{\frac{T_r(10)^2}{4}} = 7.64 \#/\text{in.}$ $S = \frac{T_r d^3}{4}$ P₅ 56
DESIGN OF PL
STRUCTURES

DUE TO SHEAR

$f = \frac{150\#}{T_r 10} = 4.77$

TOTAL STRESS

$f = \sqrt{(4.77)^2 + (36.7 + 7.64)^2} = 44.6 \#/\text{in.}$

- CAPACITY OF WELD

FILLMET = $(\frac{1}{8})(0.49)(13000)(\frac{1}{2}, \text{ FULL 15:00 STITCH OR RESIDUE})$
 = 1194 #/in

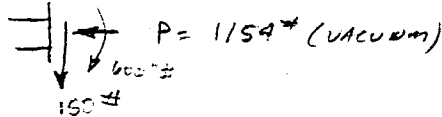
WELD IS ADEQUATE

SUBJECT 10" φ PUMP PORT 2160 BRASS TUBE	OFFICE CBI NOE C		REVISION		REFERENCE NO. 930212
	MADE BY RJM	CHKD BY BTS	MADE BY	CHKD BY	SHT 2 OF 11
	DATE 2/2/93	DATE 12/2/93	DATE	DATE	3.2

CHECK SHELL FOR NOZZLE LOADS

SAE STD 9.57-3.6

LOAD ON NOZZLE



- P = 1154
- V_L = 0
- V_C = 150
- M_C = -600#-in

INITIAL STRESS

$$\sigma_{\theta} = \frac{PR_m}{t} = \frac{14.7(24)}{.125} = 2822 \text{ psi}$$

$$\sigma_x = \frac{PR}{2t} = \frac{T\theta}{2} = 1411.2$$

$$\sigma_x (A_m \rightarrow D_m) = 1411 \text{ psi}$$

$$\sigma_{\theta} (A_m \rightarrow D_m) = 2822$$

ASSUMING SURFACE STRESS = 1/2 x MEMBRANE

$$\sigma_x (A_s \rightarrow D_w) = 1.5(1411.2) = 2117$$

$$\sigma_{\theta} (A_s \rightarrow D_w) = 1.5(2822) = 4234$$

PUNCHING SHEAR

$$\tau_{2.1} = \frac{1.5P}{2F_{AT}} = \frac{1.5(1154)}{2t(5)(.125)} = 441 \text{ psi}$$

@ 54RB

$$\tau_{2.1} = \frac{1.5(1154)}{27(5 + .54\sqrt{24(.125)})} = 376 \text{ psi}$$

SUBJECT 10" φ PUMP PORT L160 ISOTHERM TUBE	OFFICE CBI 11062		REVISION		REFERENCE NO. 93015
	MADE BY RSH	CHKD BY BTS	MADE BY	CHKD BY	SHT 3 OF 11
	DATE 2/10/93	DATE 10 NOV 93	DATE	DATE	3.3

PROGRAM E1027A STD 9107-3-6 I IBM RS/6000 1
 REVISION 14 - 04/22/93 (WRC-107 REV MAR 79)
 STRESS INTENSITIES AT LOADED ATTACHMENTS IN CYLINDERS AND SPHERES
 SEE CBI STANDARD 9107-3-6 FOR INSTRUCTIONS ON USING THIS PROGRAM.

INPUT

ATTCHMNTS 1	VESSEL 1=CYL	LOADING -1=FIXED	ANALYSIS 3D			
KN 1.00	KB 1.00	RM 24.000	T .125	LOC .5000		C .00
RO 5.000	TI .000	TP .000	W .000			
F 1154.00	VL .00	VC -150.00	MC -600.00	ML .00		MT .00

INITIAL STRESSES NEXT TO ATTCHMNT

SX(AU)	SO(AU)	SX(CU)	SO(CU)	SX(AM)	SO(AM)	SX(CM)	SO(CM)
2117.	4234.	2117.	4234.	1411.	2822.	1411.	2822.

INITIAL STRESSES AT LOC*SQRT(R*T)

SX(AU)	SO(AU)	SX(CU)	SO(CU)	SX(AM)	SO(AM)	SX(CM)	SO(CM)
2117.	4234.	2117.	4234.	1411.	2822.	1411.	2822.

INITIAL STRESSES AT EDGE OF REINF

SX(AU)	SO(AU)	SX(CU)	SO(CU)	SX(AM)	SO(AM)	SX(CM)	SO(CM)
0.	0.	0.	0.	0.	0.	0.	0.

INT
PRESS

-15.

RADIAL PUNCHING SHEAR STRESSES

NEXT TO ATTCHMNT
AT A,B,C, AND D
441.

AT LOC*SQRT(R*T)
AT A,B,C, AND D
376.

AT EDGE OF REINF
AT A,B,C, AND D
0.

10" DIAMETER PUMP PORT
930212
ROUND ATTCHMT 10" DIA NOZZLE

ON A CYLINDRICAL VESSEL

SUBJECT	MADE BY	CHKD BY	0	BY	CHARGE NO. 930212
	DATE	DATE		CHKD	
	18NOV93	18NOV93		DATE	

OUTPUT

BIJLAARD COEFICIENTS

	NEXT TO ATTCHMNT		AT LOC*SQRT(R*T)	
	A & B	C & D	A & B	C & D
NX/F	6.727	18.388	5.151	16.181
IX/F	.032	.013*	.025	.009*
NX/MC	16.360		16.360	
MX/MC	.022		.020	
NX/ML	5.532		4.626	
MX/ML	.014		.012	
NO/F	18.388	6.727	16.181	5.151
MO/F	.013	.032*	.009	.025*
NO/MC	5.573		4.670	
MO/MC	.054		.051	
NO/ML	11.680		9.832	
MO/ML	.010		.008	

* MX/P FROM FIG 2C-1 INSTEAD OF 2C &/OR MO/P FROM 1C-1 INSTEAD OF 1C

10" DIAMETER PUMP PORT

930212

ROUND ATTCHMT 10" DIA NOZZLE

ON A CYLINDRICAL VESSEL

SUBJECT	MADE BY	CHKD BY	0	BY	CHARGE NO. 930212
	DATE	DATE		CHKD	
	10 NOV 93	10 NOV 93		DATE	

SURFACE STRESS INTENSITIES NEXT TO ATTCHMNT (U=OUTS);(L=INS)
 RO= 5.000;

	(AU)	(AL)	(BU)	(BL)	(CU)	(CL)	(DU)	(DL)
SX	-16840.	11665.	-16840.	11665.	-10841.	-1810.	-14626.	-1017.
SO	-12733.	-1414.	-12733.	-1414.	-13731.	9066.	-19949.	14265.
TAU	-76.	-76.	76.	76.	0.	0.	0.	0.
SI	16827.	13080.	16827.	13080.	13716.	10876.	19934.	15282.

SURFACE STRESS INTENSITIES AT LOC*SQRT(R*T) (U=OUTS);(L=INS)
 RO= 5.866;

	(AU)	(AL)	(BU)	(BL)	(CU)	(CL)	(DU)	(DL)
SX	-13278.	9315.	-13278.	9315.	-8830.	-2343.	-11879.	-1844.
SO	-10355.	-2094.	-10355.	-2094.	-10786.	7187.	-15770.	11444.
TAU	-65.	-65.	65.	65.	0.	0.	0.	0.
SI	13265.	11410.	13265.	11410.	10771.	9531.	15755.	13287.

OUTPUT INCLUDING INITIAL STRESSES

SURFACE STRESS INTENSITIES NEXT TO ATTCHMNT (U=OUTS);(L=INS)
 RO= 5.000;

	(AU)	(AL)	(BU)	(BL)	(CU)	(CL)	(DU)	(DL)
SX	-14723.	12370.	-14723.	12370.	-8724.	-1105.	-12509.	-312.
SO	-8499.	-4.	-8499.	-4.	-9497.	10476.	-15715.	15675.
TAU	-76.	-76.	76.	76.	0.	0.	0.	0.
SI	14709.	12375.	14709.	12375.	9482.	11581.	15700.	15987.

SURFACE STRESS INTENSITIES AT LOC*SQRT(R*T) (U=OUTS);(L=INS)
 RO= 5.866;

	(AU)	(AL)	(BU)	(BL)	(CU)	(CL)	(DU)	(DL)
SX	-11161.	10020.	-11161.	10020.	-6713.	-1638.	-9762.	-1139.
SO	-6121.	-684.	-6121.	-684.	-6552.	8597.	-11536.	12854.
TAU	-65.	-65.	65.	65.	0.	0.	0.	0.
SI	11147.	10705.	11147.	10705.	6698.	10236.	11521.	13992.

10" DIAMETER PUMP PORT
 930212
 ROUND ATTCHMT 10" DIA NOZZLE

ON A CYLINDRICAL VESSEL

SUBJECT	MADE BY	DATE	0 > 1 2 3 4 5 6 7 8 9 A B C D E F G H I J K L M N O P Q R S T U V W X Y Z	BY	CHARGE NO.
	DATE	DATE		CHKD	930212
	10/20/93	10/10/93		DATE	SHT 6 OF 11

CTRLINE STRESS INTENSITIES NEXT TO ATTCHMNT (M=CTRL)
 RO= 5.000;

	(AM)	(BM)	(CM)	(DM)
SX	-2588.	-2588.	-6326.	-7821.
SD	-7073.	-7073.	-2333.	-2842.
TAU	-76.	76.	0.	0.
SI	7155.	7155.	6413.	7896.

CTRLINE STRESS INTENSITIES AT LOC*SQRT(R*T) (M=CTRL)
 RO= 5.866;

	(AM)	(BM)	(CM)	(DM)
SX	-1981.	-1981.	-5587.	-6862.
SD	-6224.	-6224.	-1799.	-2163.
TAU	-65.	65.	0.	0.
SI	6301.	6301.	5669.	6932.

OUTPUT INCLUDING INITIAL STRESSES
 CTRLINE STRESS INTENSITIES NEXT TO ATTCHMNT (M=CTRL)
 RO= 5.000;

	(AM)	(BM)	(CM)	(DM)
SX	-1177.	-1177.	-4915.	-6410.
SD	-4251.	-4251.	489.	-20.
TAU	-76.	76.	0.	0.
SI	4407.	4407.	5665.	6845.

CTRLINE STRESS INTENSITIES AT LOC*SQRT(R*T) (M=CTRL)
 RO= 5.866;

	(AM)	(BM)	(CM)	(DM)
SX	-570.	-570.	-4176.	-5451.
SD	-3402.	-3402.	1023.	659.
TAU	-65.	65.	0.	0.
SI	3597.	3597.	5322.	6281.

10" DIAMETER PUMP PORT
 930212
 ROUND ATTCHMT 10" DIA NOZZLE ON A CYLINDRICAL VESSEL
 *SYSINI

SUBJECT	DATE	DATE	BY	CHKD	DATE	CHARGE NO.
	10 NOV 93	10 NOV 93				930212
						SHT 7 OP 11

PROGRAM E1027A STD 9107- 3- 6 [IBM RS/6000]
 REVISION 14 - 04/22/93 (WRC-107 REV MAR 79)
 STRESS INTENSITIES AT LOADED ATTACHMENTS IN CYLINDERS AND SPHERES
 SEE CBI STANDARD 9107-3-6 FOR INSTRUCTIONS ON USING THIS PROGRAM.

INPUT

ATTCHMNTS 1	VESSEL 1=CYL	LOADING -1=FIXED	ANALYSIS 3D			
KN 1.00	KB 1.00	RM 24.000	T .125	LOC .5000	C .00	
RO 5.000	TI .000	TP .000	W .000			
F 1154.00	VL .00	VC 150.00	MC 600.00	ML .00	MT .00	

INITIAL STRESSES NEXT TO ATTCHMNT

SX(AU)	SO(AU)	SX(CU)	SO(CU)	SX(AM)	SO(AM)	SX(CM)	SO(CM)
2117.	4234.	2117.	4234.	1411.	2822.	1411.	2822.

INITIAL STRESSES AT LOC*SQRT(R*T)

SX(AU)	SO(AU)	SX(CU)	SO(CU)	SX(AM)	SO(AM)	SX(CM)	SO(CM)
2117.	4234.	2117.	4234.	1411.	2822.	1411.	2822.

INITIAL STRESSES AT EDGE OF REINF

SX(AU)	SO(AU)	SX(CU)	SO(CU)	SX(AM)	SO(AM)	SX(CM)	SO(CM)
0.	0.	0.	0.	0.	0.	0.	0.

INT
PRESS

-15.

RADIAL PUNCHING SHEAR STRESSES
 NEXT TO ATTCHMNT
 AT A,B,C, AND D
 441.

AT LOC*SQRT(R*T)
 AT A,B,C, AND D
 376.

AT EDGE OF REINF
 AT A,B,C, AND D
 0.

10" DIAMETER PUMP PORT
 930212
 ROUND ATTCHMT 10" DIA NOZZLE

ON A CYLINDRICAL VESSEL

SUBJECT	BY	CHARGE NO. 930212
	CHKD	
	DATE	
BTS	RW	
10 NOV 93	10 NOV 93	

OUTPUT

BIJLAARD COEFFICIENTS

	NEXT TO ATTCHMNT		AT LOC*SQRT(R*T)	
	A & B	C & D	A & B	C & D
NX/P	6.727	18.388	5.151	16.181
MX/P	.032	.013*	.025	.009*
NX/MC	16.360		16.360	
MX/MC	.022		.020	
NX/ML	5.532		4.626	
MX/ML	.014		.012	
NO/P	18.388	6.727	16.181	5.151
MO/P	.013	.032*	.009	.025*
NO/MC	5.573		4.670	
MO/MC	.054		.051	
NO/ML	11.680		9.832	
MO/ML	.010		.008	

* MX/P FROM FIG 2C-1 INSTEAD OF 2C &/OR MO/P FROM 1C-1 INSTEAD OF 1C

10" DIAMETER PUMP PORT
 930212
 ROUND ATTCHMT 10" DIA NOZZLE

ON A CYLINDRICAL VESSEL

SUBJECT	MADE BY	CHKD BY	Q	BY	CHARGE NO. 930212
	DATE	DATE		CHKD	
	10 NOV 93	12 NOV 93		DATE	

SURFACE STRESS INTENSITIES NEXT TO ATTCHMNT (U=OUTS); (L=INS)

RO= 5.000;

	(AU)	(AL)	(BU)	(BL)	(CU)	(CL)	(DU)	(DL)
SX	-16840.	11665.	-16840.	11665.	-14626.	-1017.	-10841.	-1810.
SO	-12733.	-1414.	-12733.	-1414.	-19949.	14265.	-13731.	9066.
TAU	76.	76.	-76.	-76.	0.	0.	0.	0.
SI	16827.	13080.	16827.	13080.	19934.	15282.	13716.	10876.

SURFACE STRESS INTENSITIES AT LOC*SQRT(R*T) (U=OUTS); (L=INS)

RO= 5.866;

	(AU)	(AL)	(BU)	(BL)	(CU)	(CL)	(DU)	(DL)
SX	-13278.	9315.	-13278.	9315.	-11879.	-1844.	-8830.	-2343.
SO	-10355.	-2094.	-10355.	-2094.	-15770.	11444.	-10786.	7187.
TAU	65.	65.	-65.	-65.	0.	0.	0.	0.
SI	13265.	11410.	13265.	11410.	15755.	13287.	10771.	9531.

OUTPUT INCLUDING INITIAL STRESSES

SURFACE STRESS INTENSITIES NEXT TO ATTCHMNT (U=OUTS); (L=INS)

RO= 5.000;

	(AU)	(AL)	(BU)	(BL)	(CU)	(CL)	(DU)	(DL)
SX	-14723.	12370.	-14723.	12370.	-12509.	-312.	-8724.	-1105.
SO	-8499.	-4.	-8499.	-4.	-15715.	15675.	-9497.	10476.
TAU	76.	76.	-76.	-76.	0.	0.	0.	0.
SI	14709.	12375.	14709.	12375.	15700.	15987.	9482.	11581.

SURFACE STRESS INTENSITIES AT LOC*SQRT(R*T) (U=OUTS); (L=INS)

RO= 5.866;

	(AU)	(AL)	(BU)	(BL)	(CU)	(CL)	(DU)	(DL)
SX	-11161.	10020.	-11161.	10020.	-9762.	-1139.	-6713.	-1638.
SO	-6121.	-684.	-6121.	-684.	-11536.	12854.	-6552.	8597.
TAU	65.	65.	-65.	-65.	0.	0.	0.	0.
SI	11147.	10705.	11147.	10705.	11521.	13992.	6698.	10236.

10" DIAMETER PUMP PORT

930212

ROUND ATTCHMT 10" DIA NOZZLE

ON A CYLINDRICAL VESSEL

SUBJECT	BTS	RJW	BY	CHARGE NO.
	DATE	DATE	CHKD	930212
	10/01/93	10/01/93	DATE	SHT 10 OF 11

CTRLINE STRESS INTENSITIES NEXT TO ATTCHMNT (M=CTRL)

RO= 5.000;

	(AM)	(BM)	(CM)	(DM)
SX	-2588.	-2588.	-7821.	-6326.
SO	-7073.	-7073.	-2842.	-2333.
TAU	76.	-76.	0.	0.
SI	7155.	7155.	7896.	6413.

CTRLINE STRESS INTENSITIES AT LOC*SQRT(R*T) (M=CTRL)

RO= 5.866;

	(AM)	(BM)	(CM)	(DM)
SX	-1981.	-1981.	-6862.	-5587.
SO	-6224.	-6224.	-2163.	-1799.
TAU	65.	-65.	0.	0.
SI	6301.	6301.	6932.	5669.

OUTPUT INCLUDING INITIAL STRESSES

CTRLINE STRESS INTENSITIES NEXT TO ATTCHMNT (M=CTRL)

RO= 5.000;

	(AM)	(BM)	(CM)	(DM)
SX	-1177.	-1177.	-6410.	-4915.
SO	-4251.	-4251.	-20.	489.
TAU	76.	-76.	0.	0.
SI	4407.	4407.	6845.	5665.

CTRLINE STRESS INTENSITIES AT LOC*SQRT(R*T) (M=CTRL)

RO= 5.866;

	(AM)	(BM)	(CM)	(DM)
SX	-570.	-570.	-5451.	-4176.
SO	-3402.	-3402.	659.	1023.
TAU	65.	-65.	0.	0.
SI	3597.	3597.	6281.	5322.

10" DIAMETER PUMP PORT

930212

ROUND ATTCHMT 10" DIA NOZZLE

ON A CYLINDRICAL VESSEL

P "SYSINI

SUBJECT	MADE BY	CHGD BY	BY	CHARGE NO.
	BTS	Riri		930212
	DATE	DATE	CHKD	
	10NOV93	10NOV93		SHT # OF #
			DATE	

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PATHWAY

ENGINEERING CALCULATIONS	CRANK	PLATE	SHEET
CUSTOMER C241	REV R2	QUOTE JOB NO.	1 of 4
			DATE 11/2/14

BELLOWS TENSION

$$S_s = \frac{2MT}{Nt\pi D_b^2} \quad (\text{ISMA EQ \# C-47})$$

OR

$$Mt = \frac{Nt\pi D_b^2 S_s}{2}$$

where $N = 1$

$$t = 8.11''$$

$$D_b = 48.5''$$

$S_s = 2000$ Psi. FROM TESTING

$$Mt = \frac{1(1) \pi (48.5)^2 2000}{2}$$

$$= 812,819 \text{ IN-LBS (MAX)}$$

E N G I N E E R I N G C A L C U L A T I O N S



PATHWAY

ENGINEERING
CALCULATIONS

DRAW

3

REV

SHEET

2 OF 4

CUSTOMER

CB&I

BY

RKB

DATE JOB NO

DATE

1/7

$$\text{TORSIONAL SPRING RATE} = \frac{\pi G n t D_o^3}{\text{PER COIL} \left(\frac{K t (G)}{N} \right) 229.2 L_d}$$

WHERE $G = 10.8 \times 10^6$ PSI

$$n = 1$$

$$t = .11''$$

$$D_o = 48.5''$$

$$L_d = .5718 + 2W$$

$$= .571(2) + 2(2.39)$$

$$= 5.922''$$

$$\text{TORSIONAL SPRING RATE} = \frac{\pi (10.8 \times 10^6) (1) (.11) (48.5)^3}{\text{PER COIL} 229.2 (5.922)}$$

$$\frac{K t (G)}{N} = 3.14 \times 10^8 \text{ in-LBS / COIL}$$

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PATHWAY

ENGINEERING CALCULATIONS

SHEET 3 of 4

CUSTOMER CB&I

BY RKM

QUOTE JOB NO

DATE 1/7

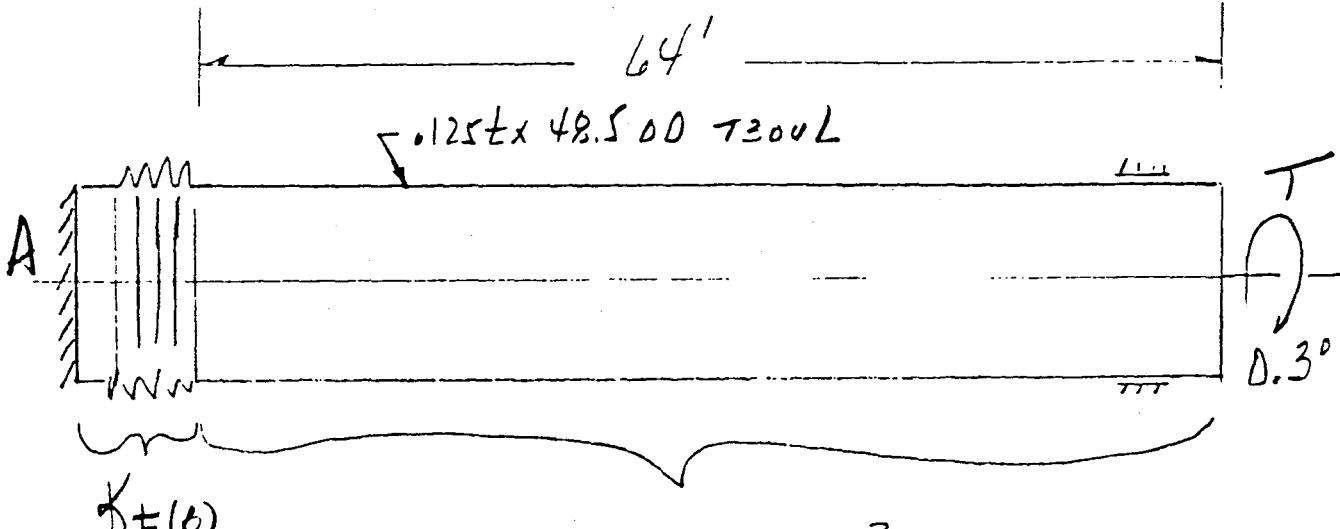
N No. CONS	Mt MAX MOMENT	K(LB) TORSIONAL SPRING RATE	θ (MAX) MAX. TORSIONAL ROTATION
2	812,879	1.57×10^8	0.0052°
4	↓	7.85×10^7	0.0104°
6		5.23×10^7	0.0155°
8		3.93×10^7	0.0207°
10		3.14×10^7	0.0259°

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ENGINEERING CALCULATIONS





$$K_t(B) = \frac{\pi G t D^3}{229.2 L}$$

$$\theta_B = \frac{K_t(P) \theta_P}{K_t(B)}$$

$$= \frac{\pi (10.8 \times 10^6) (.125) (48.5)^3}{229.2 (64) (12)}$$

$$= 2.75 \times 10^6 \text{ in-lbs / Deg}$$

No Rows	Bellows $K_t(B)$	θ_B	Torque on Anchor (T)
2	1.57×10^8	$.0053^\circ$	$8.3 \times 10^5 \text{ in-lbs}$
4	7.85×10^7	$.0105^\circ$	
6	5.23×10^7	$.0158^\circ$	
8	3.93×10^7	$.0210^\circ$	
10	3.14×10^7	$.0263^\circ$	

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ENGINEERING CALCULATIONS



**BEAM TUBE & EXPANSION JOINT PROPERTIES - BEAM ELEMENT EQUIVALENTS
FOR INCLUSION IN RISA-2D BEAM TUBE MODEL**

PURPOSE

CONVERT THE AXIAL AND ANGULAR SPRING RATES OF THE EXPANSION JOINT BELLOWS INTO EQUIVALENT AXIAL AND ROTATIONAL STIFFNESSES FOR A BEAM ELEMENT FOR INCLUSION IN A RISA-2D MODEL OF A PORTION OF THE BEAM TUBE MODULE. CONVERT INTO EQUIVALENT AREA AND MOMENT OF INERTIA. ADDITIONALLY, DETERMINE THE AREA AND MOMENT OF INERTIA OF THE BEAM TUBE FOR INCLUSION IN THE RISA-2D MODEL

EXPANSION JOINT PROPERTIES

AXIAL SPRING RATE	8,316	LB / IN	(PATHWAY)
ANGULAR SPRING RATE	47,191	IN-LB / DEG	(PATHWAY)
ANGULAR SPRING RATE	2,703,845	IN-LB / RAD	
ELASTIC MODULUS AT 70F	28,300	KSI	
ELASTIC MODULUS AT 300F	27,000	KSI	
BELLOWS LENGTH	18	IN	
BELLOWS THICKNESS	0.105	IN	
BELLOWS O.D.	49.00	IN	

BEAM TUBE PROPERTIES

BEAM TUBE O.D	49.00	IN
BEAM TUBE THICKNESS	0.127	IN

FOR THE RISA BEAM ELEMENT REPRESENTING THE BELLOWS:

AXIAL STIFFNESS = AREA * ELASTIC MODULUS / LENGTH

====> AREA = AXIAL STIFFNESS * LENGTH / ELASTIC MODULUS

ROTATIONAL STIFFNESS = ELASTIC MODULUS * MOMENT OF INERTIA / LENGTH

====> MOMENT OF INERTIA = (ROTATIONAL STIFFNESS * LENGTH) / (ELASTIC MODULUS)

RISA BEAM ELEMENT PROPERTIES @ 70F (FOR BELLOWS)

AREA =====>	0.0053	IN**2
INERTIA =====>	1.7198	IN**4

RISA BEAM ELEMENT PROPERTIES @ 300F (FOR BELLOWS)

AREA =====>	0.0055	IN**2
INERTIA =====>	1.8026	IN**4

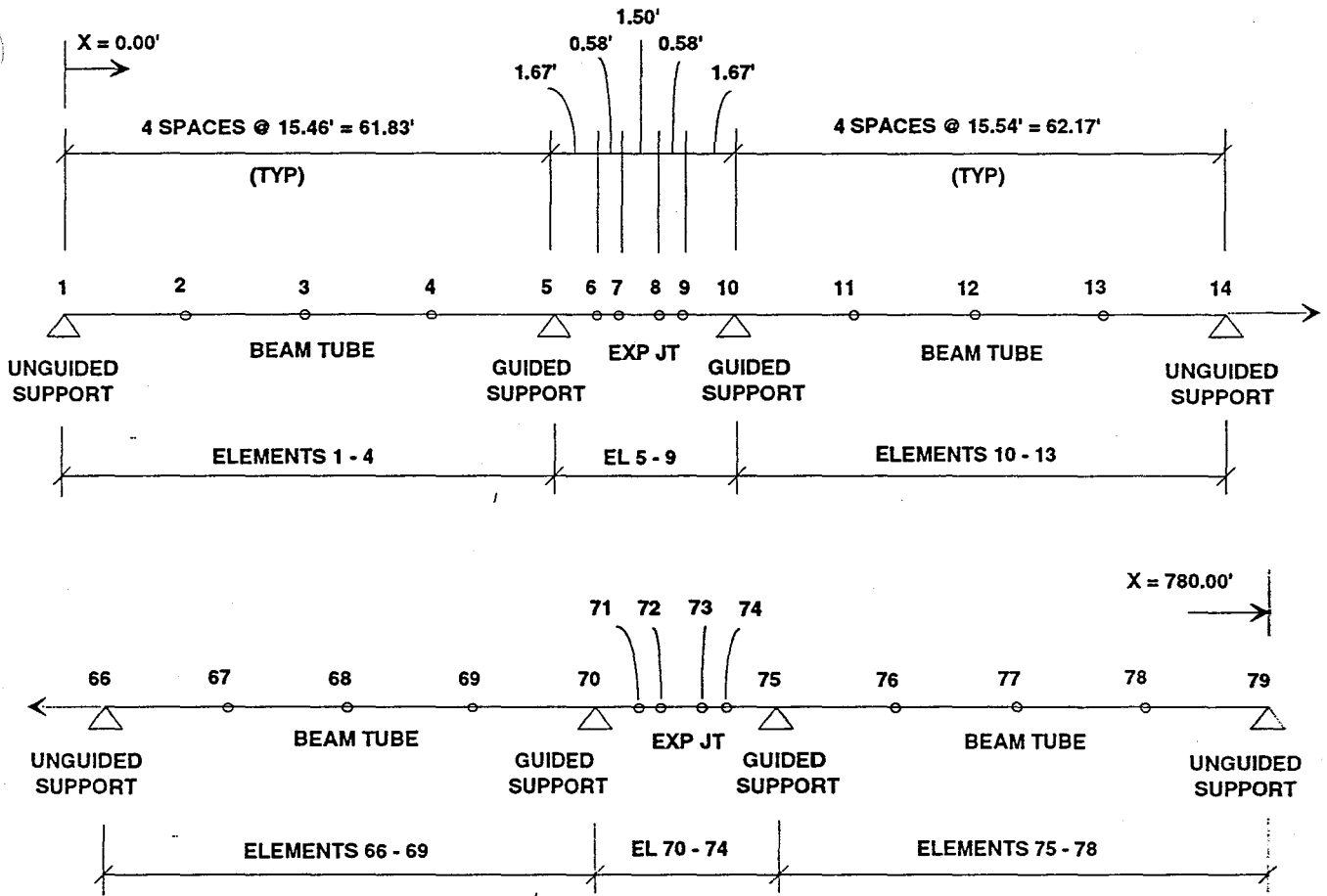
RISA BEAM ELEMENT PROPERTIES (FOR NONCONVOLUTED EXPANSION JOINT END)

AREA =====>	16.13	IN**2
INERTIA =====>	4819.97	IN**4

RISA BEAM ELEMENT PROPERTIES (FOR BEAM TUBE)

AREA =====>	19.50	IN**2	
INERTIA =====>	5823.45	IN**4	NOTE: 5731 IN**4 USED IN MODEL =====> NEGLIGIBLE DIFFERENCE

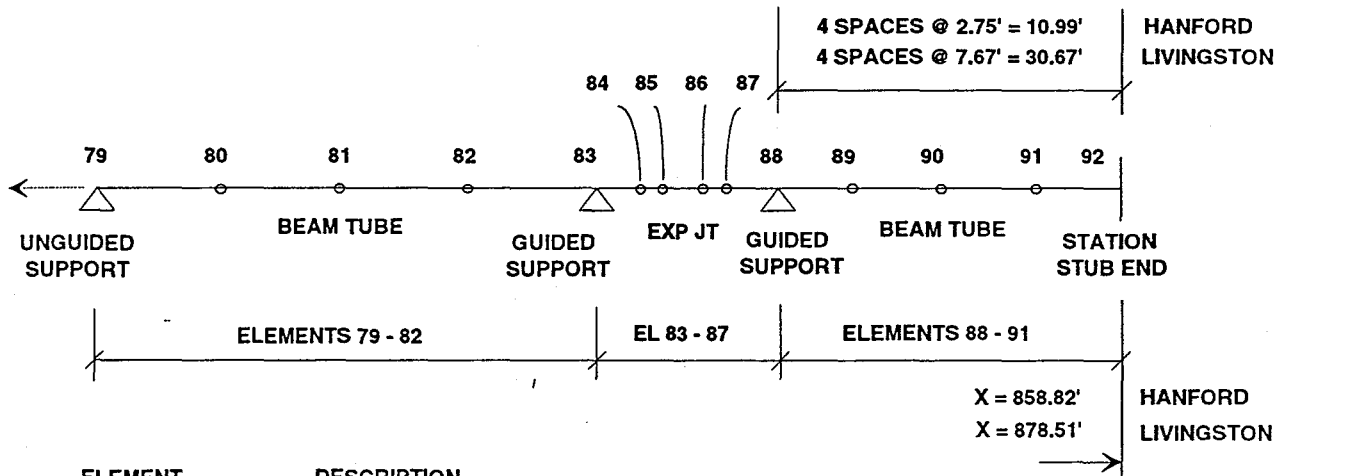
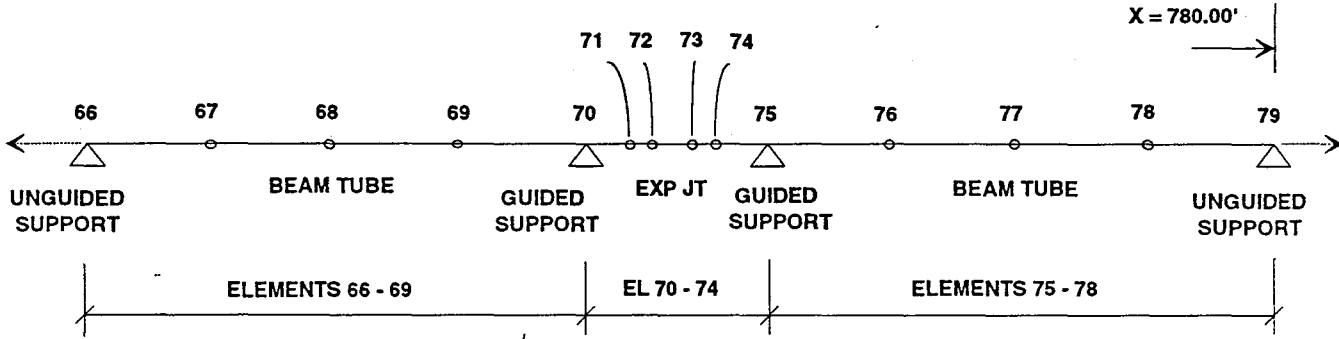
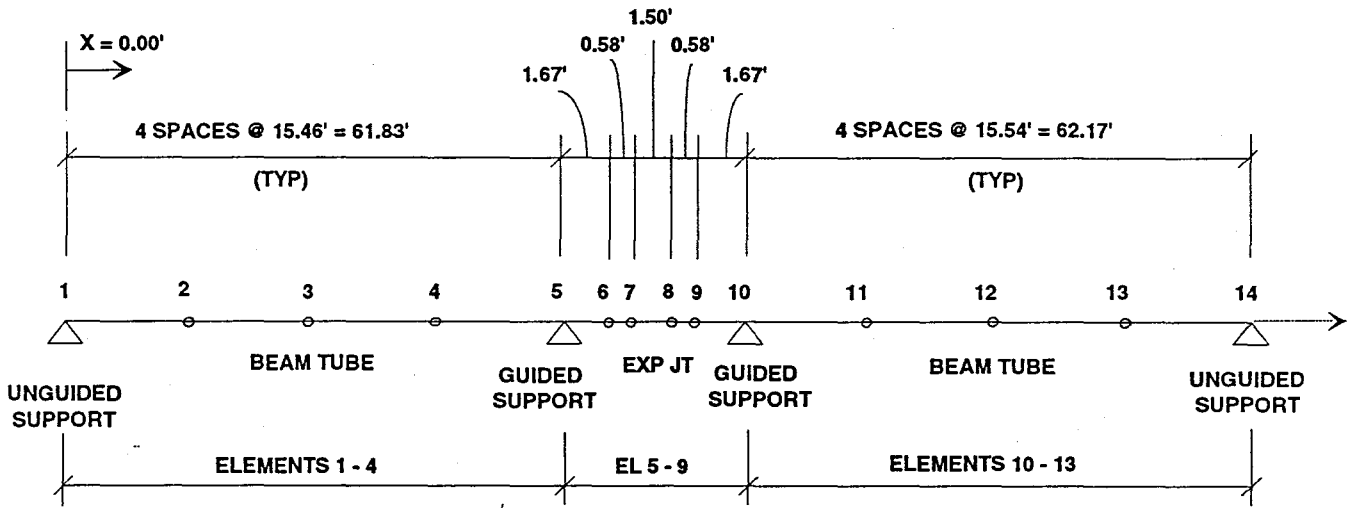
SUBJECT RISA-2D BEAM ELEMENT SECTION PROPERTIES BELLOWS, EXPANSION JOINT END & BEAM TUBE	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RJW	MADE BY	CHKD BY	SHT 1 OF 1
	DATE 2/28/94	DATE 4 MAR 94	DATE	DATE	5.1



ELEMENT	DESCRIPTION
1 - 5, 9 - 13, 66 - 70, 74 - 78	BEAM TUBE
6, 8, 71, 73	EXP JT STUB END
7, 72	EXP JT BELLOWS

NOT TO SCALE

SUBJECT RISA-2D MODEL OF LIGO BEAM TUBE CASE #1, #1A & #1B NODES AND ELEMENTS	CBI OFFICE NOE C	REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD BY RW	MADE BY	CHKD BY
	DATE 2-25-94	DATE 19 MAR 94	DATE	DATE
				SHT 1 OF 2 5.2



ELEMENT	DESCRIPTION
1 - 5, 9 - 13, 66 - 70, 74 - 78, 79 - 83, 87 - 91	BEAM TUBE
6, 8, 71, 73, 84, 86	EXP JT STUB END
7, 72, 85	EXP JT BELLOWS

NOT TO SCALE

SUBJECT RISA-2D MODEL OF LIGO BEAM TUBE CASE #2 -- HANFORD, WA SITE CASE #3 -- LIVINGSTON, LA SITE NODES AND ELEMENTS	CBI OFFICE NOE C	REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD BY RJA	MADE BY	CHKD BY
	DATE 2-26-94	DATE 13 MAR 94	DATE	DATE
				5.3

**RISA-2D MODEL OF LIGO BEAM TUBE MODULE: CASE #1 NODE NUMBERS
CASE #1: SIMPLE SUPPORTS AT BOTH ENDS, TOTAL LENGTH = 780'**

<u>NODE</u>	<u>X (FT)</u>	<u>DESCRIPTION</u>
1	0.000	FIXED SUPP'T, ROTATION FREE
2	15.458	
3	30.917	
4	46.375	
5	61.833	GUIDED SUPP'T
6	63.500	END TUBE / START EJ STUB
7	64.083	END EJ STUB / START BELLOWS
8	65.583	END BELLOWS / START EJ STUB
9	66.167	END EJ STUB / START TUBE
10	67.833	GUIDED SUPP'T
11	83.375	
12	98.917	
13	114.458	
14	130.000	FIXED SUPP'T, ROTATION FREE
15	145.458	
16	160.917	
17	176.375	
18	191.833	GUIDED SUPP'T
19	193.500	END TUBE / START EJ STUB
20	194.083	END EJ STUB / START BELLOWS
21	195.583	END BELLOWS / START EJ STUB
22	196.167	END EJ STUB / START TUBE
23	197.833	GUIDED SUPP'T
24	213.375	
25	228.917	
26	244.458	
27	260.000	FIXED SUPP'T, ROTATION FREE
28	275.458	
29	290.917	
30	306.375	
31	321.833	GUIDED SUPP'T
32	323.500	END TUBE / START EJ STUB
33	324.083	END EJ STUB / START BELLOWS
34	325.583	END BELLOWS / START EJ STUB
35	326.167	END EJ STUB / START TUBE
36	327.833	GUIDED SUPP'T
37	343.375	
38	358.917	
39	374.458	
40	390.000	FIXED SUPP'T, ROTATION FREE
41	405.458	
42	420.917	
43	436.375	
44	451.833	GUIDED SUPP'T
45	453.500	END TUBE / START EJ STUB
46	454.083	END EJ STUB / START BELLOWS
47	455.583	END BELLOWS / START EJ STUB
48	456.167	END EJ STUB / START TUBE
49	457.833	GUIDED SUPP'T

SUBJECT RISA-2D MODEL OF BEAM TUBE MODULE, CASE #1 NODE NUMBERS, COORDINATES & DESCRIPTION	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD Rjw	MADE BY	CHKD BY	SHT 1 OF 2
	DATE 2/28/94	DATE 10/11/94	DATE	DATE	5.4

**RISA-2D MODEL OF LIGO BEAM TUBE MODULE: CASE #1 NODE NUMBERS
CASE #1: SIMPLE SUPPORTS AT BOTH ENDS, TOTAL LENGTH = 780'**

NODE	X(FT)	DESCRIPTION
50	473.375	
51	488.917	
52	504.458	
53	520.000	FIXED SUPPT, ROTATION FREE
54	535.458	
55	550.917	
56	566.375	
57	581.833	GUIDED SUPPT
58	583.500	END TUBE / START EJ STUB
59	584.083	END EJ STUB / START BELLOWS
60	585.583	END BELLOWS / START EJ STUB
61	586.167	END EJ STUB / START TUBE
62	587.833	GUIDED SUPPT
63	603.375	
64	618.917	
65	634.458	
66	650.000	FIXED SUPPT, ROTATION FREE
67	665.458	
68	680.917	
69	696.375	
70	711.833	GUIDED SUPPT
71	713.500	END TUBE / START EJ STUB
72	714.083	END EJ STUB / START BELLOWS
73	715.583	END BELLOWS / START EJ STUB
74	716.167	END EJ STUB / START TUBE
75	717.833	GUIDED SUPPT
76	733.375	
77	748.917	
78	764.458	
79	780.000	FIXED SUPPT, ROTATION FREE

SUBJECT RISA-2D MODEL OF BEAM TUBE MODULE, CASE #1 NODE NUMBERS, COORDINATES & DESCRIPTION	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RJR	MADE BY	CHKD BY	SHT 2 OF 2
	DATE 2/28/94	DATE 18 MAR 94	DATE	DATE	5.5

LIGO BEAM TUBE -- BAKEOUT + SELF WT
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Units Option : US Standard

AISC Code Checks : 9th Edition ASD

Shear Deformation: No

P-Delta Effects : No

Redesign : No

Edge Forces : No

A.S.I.F. : 1.333

SUBJECT	OFFICE CBI Noe C		REVISION		REFERENCE NO.
	MADE BY WJC	CHKD BY RJM	MADE BY	CHKD BY	SHT ___ OF ___
	DATE 3-18-94	DATE 12-11-92	DATE	DATE	

LIGO BEAM TUBE -- BAKEOUT + SELF WT
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Node No	X-Coord (ft)	Y-Coord (ft)	Boundary Conditions			Temp. (F)
			X-dof (in,K/in)	Y-dof (in,K/in)	Rotation (r,K-ft/r)	
1	0.00	0.00	R	R		0.00
2	15.46	0.00				0.00
3	30.92	0.00				0.00
4	46.37	0.00				0.00
5	61.83	0.00		R		0.00
6	62.92	0.00				0.00
7	63.50	0.00				0.00
8	66.17	0.00				0.00
9	66.75	0.00				0.00
10	67.83	0.00		R		0.00
11	83.37	0.00				0.00
12	98.92	0.00				0.00
13	114.46	0.00				0.00
14	130.00	0.00	R	R		0.00
15	145.46	0.00				0.00
16	160.92	0.00				0.00
17	176.37	0.00				0.00
18	191.83	0.00		R		0.00
19	192.92	0.00				0.00
20	193.50	0.00				0.00
21	196.17	0.00				0.00
22	196.75	0.00				0.00
23	197.83	0.00		R		0.00
24	213.37	0.00				0.00
25	228.92	0.00				0.00
26	244.46	0.00				0.00
27	260.00	0.00	R	R		0.00
28	275.46	0.00				0.00
29	290.92	0.00				0.00
30	306.37	0.00				0.00
31	321.83	0.00		R		0.00
32	322.92	0.00				0.00
33	323.50	0.00				0.00
34	326.17	0.00				0.00
35	326.75	0.00				0.00
36	327.83	0.00		R		0.00
37	343.37	0.00				0.00
38	358.92	0.00				0.00
39	374.46	0.00				0.00
40	390.00	0.00	R	R		0.00
41	405.46	0.00				0.00
42	420.92	0.00				0.00
43	436.37	0.00				0.00
44	451.83	0.00		R		0.00
45	452.92	0.00				0.00

3/09/94

LIGO BEAM TUBE -- BAKEOUT + SELF WT
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

=====

Node No	X-Coord (ft)	Y-Coord (ft)	Boundary Conditions			Temp. (F)
			X-dof (in,K/in)	Y-dof (in,K/in)	Rotation (r,K-ft/r)	
46	453.50	0.00				0.00
47	456.17	0.00				0.00
48	456.75	0.00				0.00
49	457.83	0.00				0.00
50	473.37	0.00			R	0.00
51	488.92	0.00				0.00
52	504.46	0.00				0.00
53	520.00	0.00	R	R		0.00
54	535.46	0.00				0.00
55	550.92	0.00				0.00
56	566.37	0.00				0.00
57	581.83	0.00			R	0.00
58	582.92	0.00				0.00
59	583.50	0.00				0.00
60	586.17	0.00				0.00
61	586.75	0.00				0.00
62	587.83	0.00			R	0.00
63	603.37	0.00				0.00
64	618.92	0.00				0.00
65	634.46	0.00				0.00
66	650.00	0.00	R	R		0.00
67	665.46	0.00				0.00
68	680.92	0.00				0.00
69	696.37	0.00				0.00
70	711.83	0.00			R	0.00
71	712.92	0.00				0.00
72	713.50	0.00				0.00
73	716.17	0.00				0.00
74	716.75	0.00				0.00
75	717.83	0.00			R	0.00
76	733.37	0.00				0.00
77	748.92	0.00				0.00
78	764.46	0.00				0.00
79	780.00	0.00	R	R		0.00

3/09/94

LIGO BEAM TUBE -- BAKEOUT + SELF WT
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Material Label	Elastic Modulus (Ksi)	Poisson's Ratio	Thermal Coefficient (F)	Weight Density (K/ft3)	Yield Stress (Fy) (Ksi)
1	27000.00	0.30000	0.65000	0.000	19.200

3/09/94

LIGO BEAM TUBE -- BAKEOUT + SELF WT
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Section Label	Database Shape	Matl. Set	Area (in ²)	Moment of Inertia (in ⁴)	As Coef	y/Y
BEAMTUBE		1	19.19	5731.030	1.20	
STUBEND		1	16.13	4820.000	1.20	
EXPJOINT		1	0.01	1.803	1.20	

3/09/94

LIGO BEAM TUBE -- BAKEOUT + SELF WT
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

I No	I Node	J Node	Section	I Releases			J			End Offsets		Length (ft)
				x	y	z	x	y	z	Sec	Sway	
1	1	-	2	BEAMTUBE								15.46
2	2	-	3	BEAMTUBE								15.46
3	3	-	4	BEAMTUBE								15.45
4	4	-	5	BEAMTUBE								15.46
5	5	-	6	BEAMTUBE								1.09
6	6	-	7	STUBEND								0.58
7	7	-	8	EXPJOINT								2.67
8	8	-	9	STUBEND								0.58
9	9	-	10	BEAMTUBE								1.08
10	10	-	11	BEAMTUBE								15.54
11	11	-	12	BEAMTUBE								15.55
12	12	-	13	BEAMTUBE								15.54
13	13	-	14	BEAMTUBE								15.54
14	14	-	15	BEAMTUBE								15.46
15	15	-	16	BEAMTUBE								15.46
16	16	-	17	BEAMTUBE								15.45
17	17	-	18	BEAMTUBE								15.46
18	18	-	19	BEAMTUBE								1.09
19	19	-	20	STUBEND								0.58
20	20	-	21	EXPJOINT								2.67
21	21	-	22	STUBEND								0.58
22	22	-	23	BEAMTUBE								1.08
23	23	-	24	BEAMTUBE								15.54
24	24	-	25	BEAMTUBE								15.55
25	25	-	26	BEAMTUBE								15.54
26	26	-	27	BEAMTUBE								15.54
27	27	-	28	BEAMTUBE								15.46
28	28	-	29	BEAMTUBE								15.46
29	29	-	30	BEAMTUBE								15.45
30	30	-	31	BEAMTUBE								15.46
31	31	-	32	BEAMTUBE								1.09
32	32	-	33	STUBEND								0.58
33	33	-	34	EXPJOINT								2.67
34	34	-	35	STUBEND								0.58
35	35	-	36	BEAMTUBE								1.08
36	36	-	37	BEAMTUBE								15.54
37	37	-	38	BEAMTUBE								15.55
38	38	-	39	BEAMTUBE								15.54
39	39	-	40	BEAMTUBE								15.54
40	40	-	41	BEAMTUBE								15.46
41	41	-	42	BEAMTUBE								15.46
42	42	-	43	BEAMTUBE								15.45
43	43	-	44	BEAMTUBE								15.46
44	44	-	45	BEAMTUBE								1.09
45	45	-	46	STUBEND								0.58

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LIGO BEAM TUBE -- BAKEOUT + SELF WT
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

No	I Node	J Node	Section	I Releases			J			End Offsets		Length
				x	y	z	x	y	z	Sec	Sway	
46	46	-	47	EXPJOINT								2.67
47	47	-	48	STUBEND								0.58
48	48	-	49	BEAMTUBE								1.08
49	49	-	50	BEAMTUBE								15.54
50	50	-	51	BEAMTUBE								15.55
51	51	-	52	BEAMTUBE								15.54
52	52	-	53	BEAMTUBE								15.54
53	53	-	54	BEAMTUBE								15.46
54	54	-	55	BEAMTUBE								15.46
55	55	-	56	BEAMTUBE								15.45
56	56	-	57	BEAMTUBE								15.46
57	57	-	58	BEAMTUBE								1.09
58	58	-	59	STUBEND								0.58
59	59	-	60	EXPJOINT								2.67
60	60	-	61	STUBEND								0.58
61	61	-	62	BEAMTUBE								1.08
62	62	-	63	BEAMTUBE								15.54
63	63	-	64	BEAMTUBE								15.55
64	64	-	65	BEAMTUBE								15.54
65	65	-	66	BEAMTUBE								15.54
66	66	-	67	BEAMTUBE								15.46
67	67	-	68	BEAMTUBE								15.46
68	68	-	69	BEAMTUBE								15.45
69	69	-	70	BEAMTUBE								15.46
70	70	-	71	BEAMTUBE								1.09
71	71	-	72	STUBEND								0.58
72	72	-	73	EXPJOINT								2.67
73	73	-	74	STUBEND								0.58
74	74	-	75	BEAMTUBE								1.08
75	75	-	76	BEAMTUBE								15.54
76	76	-	77	BEAMTUBE								15.55
77	77	-	78	BEAMTUBE								15.54
78	78	-	79	BEAMTUBE								15.54

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LIGO BEAM TUBE -- BAKEOUT + SELF WT
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

I No	Node	J Node	Unbraced Lengths			K Factors		Bending Coefs	
			Lb-in (ft)	Lb-out (ft)	Lc (ft)	In	Out	Cm	Cb
1	1	-	2						
2	2	-	3						
3	3	-	4						
4	4	-	5						
5	5	-	6						
6	6	-	7						
7	7	-	8						
8	8	-	9						
9	9	-	10						
10	10	-	11						
11	11	-	12						
12	12	-	13						
13	13	-	14						
14	14	-	15						
15	15	-	16						
16	16	-	17						
17	17	-	18						
18	18	-	19						
19	19	-	20						
20	20	-	21						
21	21	-	22						
22	22	-	23						
23	23	-	24						
24	24	-	25						
25	25	-	26						
26	26	-	27						
27	27	-	28						
28	28	-	29						
29	29	-	30						
30	30	-	31						
31	31	-	32						
32	32	-	33						
33	33	-	34						
34	34	-	35						
35	35	-	36						
36	36	-	37						
37	37	-	38						
38	38	-	39						
39	39	-	40						
40	40	-	41						
41	41	-	42						
42	42	-	43						
43	43	-	44						
44	44	-	45						
45	45	-	46						

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LIGO BEAM TUBE -- BAKEOUT + SELF WT
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

I No	Node	J Node	Unbraced Lengths			K Factors		Bending Coefs	
			Lb-in (ft)	Lb-out (ft)	Lc (ft)	In	Out	Cm	Cb
46	46	-	47						
47	47	-	48						
48	48	-	49						
49	49	-	50						
50	50	-	51						
51	51	-	52						
52	52	-	53						
53	53	-	54						
54	54	-	55						
55	55	-	56						
56	56	-	57						
57	57	-	58						
58	58	-	59						
59	59	-	60						
60	60	-	61						
61	61	-	62						
62	62	-	63						
63	63	-	64						
64	64	-	65						
65	65	-	66						
66	66	-	67						
67	67	-	68						
68	68	-	69						
69	69	-	70						
70	70	-	71						
71	71	-	72						
72	72	-	73						
73	73	-	74						
74	74	-	75						
75	75	-	76						
76	76	-	77						
77	77	-	78						
78	78	-	79						

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LIGO BEAM TUBE -- BAKEOUT + SELF WT
780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

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BLC No.	Basic Load Case Description	Load Totals		
		Nodal	Point	Dist.
1	SELF WEIGHT + INSULATION			78

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LIGO BEAM TUBE -- BAKEOUT + SELF WT
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Member Distributed Loads, BLC 1: SELF WEIGHT + INSULATION

Member No	I Node	J Node	Dir	Start Magnitude (K/ft, F)	End Magnitude (K/ft, F)	Start Location (ft)	End Location (ft)
1	1	2	Y	-0.091	-0.091	0.000	15.460
2	2	3	Y	-0.091	-0.091	0.000	15.460
3	3	4	Y	-0.091	-0.091	0.000	15.449
4	4	5	Y	-0.091	-0.091	0.000	15.460
5	5	6	Y	-0.091	-0.091	0.000	1.089
6	6	7	Y	-0.091	-0.091	0.000	0.580
7	7	8	Y	-0.091	-0.091	0.000	2.669
8	8	9	Y	-0.091	-0.091	0.000	0.580
9	9	10	Y	-0.091	-0.091	0.000	1.080
10	10	11	Y	-0.091	-0.091	0.000	15.540
11	11	12	Y	-0.091	-0.091	0.000	15.550
12	12	13	Y	-0.091	-0.091	0.000	15.539
13	13	14	Y	-0.091	-0.091	0.000	15.540
14	14	15	Y	-0.091	-0.091	0.000	15.460
15	15	16	Y	-0.091	-0.091	0.000	15.459
16	16	17	Y	-0.091	-0.091	0.000	15.449
17	17	18	Y	-0.091	-0.091	0.000	15.460
18	18	19	Y	-0.091	-0.091	0.000	1.089
19	19	20	Y	-0.091	-0.091	0.000	0.580
20	20	21	Y	-0.091	-0.091	0.000	2.669
21	21	22	Y	-0.091	-0.091	0.000	0.580
22	22	23	Y	-0.091	-0.091	0.000	1.080
23	23	24	Y	-0.091	-0.091	0.000	15.539
24	24	25	Y	-0.091	-0.091	0.000	15.550
25	25	26	Y	-0.091	-0.091	0.000	15.540
26	26	27	Y	-0.091	-0.091	0.000	15.539
27	27	28	Y	-0.091	-0.091	0.000	15.459
28	28	29	Y	-0.091	-0.091	0.000	15.460
29	29	30	Y	-0.091	-0.091	0.000	15.449
30	30	31	Y	-0.091	-0.091	0.000	15.459
31	31	32	Y	-0.091	-0.091	0.000	1.090
32	32	33	Y	-0.091	-0.091	0.000	0.579
33	33	34	Y	-0.091	-0.091	0.000	2.670
34	34	35	Y	-0.091	-0.091	0.000	0.579
35	35	36	Y	-0.091	-0.091	0.000	1.079
36	36	37	Y	-0.091	-0.091	0.000	15.540
37	37	38	Y	-0.091	-0.091	0.000	15.550
38	38	39	Y	-0.091	-0.091	0.000	15.539
39	39	40	Y	-0.091	-0.091	0.000	15.540
40	40	41	Y	-0.091	-0.091	0.000	15.459
41	41	42	Y	-0.091	-0.091	0.000	15.460
42	42	43	Y	-0.091	-0.091	0.000	15.449
43	43	44	Y	-0.091	-0.091	0.000	15.459
44	44	45	Y	-0.091	-0.091	0.000	1.090

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LIGO BEAM TUBE -- BAKEOUT + SELF WT
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Memb No	I Node	J Node	Dir	Start Magnitude (K/ft,F)	End Magnitude (K/ft,F)	Start Location (ft)	End Location (ft)
45	45	46	Y	-0.091	-0.091	0.000	0.579
46	46	47	Y	-0.091	-0.091	0.000	2.670
47	47	48	Y	-0.091	-0.091	0.000	0.579
48	48	49	Y	-0.091	-0.091	0.000	1.079
49	49	50	Y	-0.091	-0.091	0.000	15.540
50	50	51	Y	-0.091	-0.091	0.000	15.550
51	51	52	Y	-0.091	-0.091	0.000	15.539
52	52	53	Y	-0.091	-0.091	0.000	15.540
53	53	54	Y	-0.091	-0.091	0.000	15.460
54	54	55	Y	-0.091	-0.091	0.000	15.459
55	55	56	Y	-0.091	-0.091	0.000	15.450
56	56	57	Y	-0.091	-0.091	0.000	15.460
57	57	58	Y	-0.091	-0.091	0.000	1.089
58	58	59	Y	-0.091	-0.091	0.000	0.580
59	59	60	Y	-0.091	-0.091	0.000	2.669
60	60	61	Y	-0.091	-0.091	0.000	0.580
61	61	62	Y	-0.091	-0.091	0.000	1.080
62	62	63	Y	-0.091	-0.091	0.000	15.539
63	63	64	Y	-0.091	-0.091	0.000	15.549
64	64	65	Y	-0.091	-0.091	0.000	15.540
65	65	66	Y	-0.091	-0.091	0.000	15.539
66	66	67	Y	-0.091	-0.091	0.000	15.460
67	67	68	Y	-0.091	-0.091	0.000	15.459
68	68	69	Y	-0.091	-0.091	0.000	15.449
69	69	70	Y	-0.091	-0.091	0.000	15.460
70	70	71	Y	-0.091	-0.091	0.000	1.089
71	71	72	Y	-0.091	-0.091	0.000	0.580
72	72	73	Y	-0.091	-0.091	0.000	2.669
73	73	74	Y	-0.091	-0.091	0.000	0.580
74	74	75	Y	-0.091	-0.091	0.000	1.080
75	75	76	Y	-0.091	-0.091	0.000	15.539
76	76	77	Y	-0.091	-0.091	0.000	15.550
77	77	78	Y	-0.091	-0.091	0.000	15.540
78	78	79	Y	-0.091	-0.091	0.000	15.539

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LIGO BEAM TUBE -- BAKEOUT + SELF WT
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

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Load No.	Combination Description	Self Wt Dir Fac	BLC Fac	BLC Fac	BLC Fac	BLC Fac	BLC Fac	DYNA	W S	E V
1	DEAD LD + INSUL'N		1	1						

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LIGO BEAM TUBE -- BAKEOUT + SELF WT
780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

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Dynamic Analysis Data

Number of modes (frequencies) : 15
Basic Load Case for masses : 1
BLC mass direction of action : Y only
Acceleration of Gravity : 32.20 ft/sec**2

Load Combination is 1 : DEAD LD + INSUL'N
Nodal Displacements

Node	Global X (in)	Global Y (in)	Rotation (rad)
1	0.00000	-0.00000	-0.00082
2	0.00000	-0.13569	-0.00056
3	0.00000	-0.19000	0.00000
4	0.00000	-0.13484	0.00056
5	0.00000	-0.00000	0.00081
6	0.00000	0.01058	0.00081
7	0.00000	0.01621	0.00081
8	0.00000	0.00858	-0.00043
9	0.00000	0.00558	-0.00043
10	0.00000	-0.00000	-0.00043
11	0.00000	-0.06767	-0.00024
12	0.00000	-0.08041	0.00011
13	0.00000	-0.03810	0.00029
14	0.00000	-0.00000	0.00001
15	0.00000	-0.03509	-0.00028
16	0.00000	-0.07566	-0.00010
17	0.00000	-0.06390	0.00023
18	0.00000	-0.00000	0.00041
19	0.00000	0.00531	0.00041
20	0.00000	0.00814	0.00041
21	0.00000	0.00834	-0.00042
22	0.00000	0.00543	-0.00042
23	0.00000	-0.00000	-0.00042
24	0.00000	-0.06633	-0.00024
25	0.00000	-0.07900	0.00010
26	0.00000	-0.03734	0.00029
27	0.00000	-0.00000	0.00000
28	0.00000	-0.03549	-0.00028
29	0.00000	-0.07612	-0.00010
30	0.00000	-0.06418	0.00023
31	0.00000	-0.00000	0.00041
32	0.00000	0.00533	0.00041
33	0.00000	0.00817	0.00041
34	0.00000	0.00834	-0.00042
35	0.00000	0.00543	-0.00042
36	0.00000	-0.00000	-0.00042

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LIGO BEAM TUBE -- BAKEOUT + SELF WT
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Node	Global X (in)	Global Y (in)	Rotation (rad)
37	0.00000	-0.06633	-0.00024
38	0.00000	-0.07901	0.00010
39	0.00000	-0.03735	0.00029
40	0.00000	-0.00000	0.00000
41	0.00000	-0.03549	-0.00028
42	0.00000	-0.07611	-0.00010
43	0.00000	-0.06418	0.00023
44	0.00000	-0.00000	0.00041
45	0.00000	0.00533	0.00041
46	0.00000	0.00817	0.00041
47	0.00000	0.00834	-0.00042
48	0.00000	0.00543	-0.00042
49	0.00000	-0.00000	-0.00042
50	0.00000	-0.06633	-0.00024
51	0.00000	-0.07901	0.00010
52	0.00000	-0.03735	0.00029
53	0.00000	-0.00000	0.00000
54	0.00000	-0.03549	-0.00028
55	0.00000	-0.07611	-0.00010
56	0.00000	-0.06418	0.00023
57	0.00000	-0.00000	0.00041
58	0.00000	0.00533	0.00041
59	0.00000	0.00817	0.00041
60	0.00000	0.00831	-0.00042
61	0.00000	0.00541	-0.00042
62	0.00000	-0.00000	-0.00042
63	0.00000	-0.06605	-0.00024
64	0.00000	-0.07854	0.00010
65	0.00000	-0.03694	0.00029
66	0.00000	-0.00000	0.00000
67	0.00000	-0.03625	-0.00029
68	0.00000	-0.07751	-0.00011
69	0.00000	-0.06552	0.00023
70	0.00000	-0.00000	0.00042
71	0.00000	0.00549	0.00042
72	0.00000	0.00841	0.00042
73	0.00000	0.01637	-0.00082
74	0.00000	0.01066	-0.00082
75	0.00000	-0.00000	-0.00082
76	0.00000	-0.13775	-0.00057
77	0.00000	-0.19416	-0.00000
78	0.00000	-0.13863	0.00057
79	0.00000	-0.00000	0.00084

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LIGO BEAM TUBE -- BAKEOUT + SELF WT
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

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 Load Combination is 1 : DEAD LD + INSUL'N
 Spring Reactions

Node	Global X (K)	Global Y (K)	Moment (K-ft)
1	0.00000	2.79271	0.00000
5	0.00000	3.35017	0.00000
10	0.00000	2.15310	0.00000
14	0.00000	7.04581	0.00000
18	0.00000	2.38115	0.00000
23	0.00000	2.41705	0.00000
27	0.00000	7.02891	0.00000
31	0.00000	2.38504	0.00000
36	0.00000	2.41599	0.00000
40	0.00000	7.02898	0.00000
44	0.00000	2.38502	0.00000
49	0.00000	2.41601	0.00000
53	0.00000	7.02891	0.00000
57	0.00000	2.38608	0.00000
62	0.00000	2.41214	0.00000
66	0.00000	7.04593	0.00000
70	0.00000	2.12146	0.00000
75	0.00000	3.37775	0.00000
79	0.00000	2.80777	0.00000
Totals	0.00000	70.98000	0.00000

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LIGO BEAM TUBE -- BAKEOUT + SELF WT
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Load Combination is 1 : DEAD LD + INSUL'N
 Member End Forces

No	Nodes		I-End			J-End		
	I	J	Axial (K)	Shear (K)	Moment (K-ft)	Axial (K)	Shear (K)	Moment (K-ft)
1	1-	2	0.00	2.79	-0.00	0.00	-1.39	32.30
2	2-	3	0.00	1.39	-32.30	0.00	0.02	42.85
3	3-	4	0.00	-0.02	-42.85	0.00	1.43	31.67
4	4-	5	0.00	-1.43	-31.67	0.00	2.83	-1.27
5	5-	6	0.00	0.52	1.27	0.00	-0.42	-0.76
6	6-	7	0.00	0.42	0.76	0.00	-0.36	-0.54
7	7-	8	0.00	0.36	0.54	0.00	-0.12	0.11
8	8-	9	0.00	0.12	-0.11	0.00	-0.07	0.17
9	9-	10	0.00	0.07	-0.17	0.00	0.03	0.19
10	10-	11	0.00	2.12	-0.19	0.00	-0.71	22.20
11	11-	12	0.00	0.71	-22.20	0.00	0.71	22.23
12	12-	13	0.00	-0.71	-22.23	0.00	2.12	0.27
13	13-	14	0.00	-2.12	-0.27	0.00	3.53	-43.66
14	14-	15	0.00	3.51	43.66	0.00	-2.10	-0.24
15	15-	16	0.00	2.10	0.24	0.00	-0.70	21.43
16	16-	17	0.00	0.70	-21.43	0.00	0.71	21.35
17	17-	18	0.00	-0.71	-21.35	0.00	2.11	-0.47
18	18-	19	0.00	0.27	0.47	0.00	-0.17	-0.23
19	19-	20	0.00	0.17	0.23	0.00	-0.11	-0.15
20	20-	21	0.00	0.11	0.15	0.00	0.13	-0.17
21	21-	22	0.00	-0.13	0.17	0.00	0.18	-0.26
22	22-	23	0.00	-0.18	0.26	0.00	0.28	-0.51
23	23-	24	0.00	2.14	0.51	0.00	-0.72	21.72
24	24-	25	0.00	0.72	-21.72	0.00	0.69	21.97
25	25-	26	0.00	-0.69	-21.97	0.00	2.11	0.23
26	26-	27	0.00	-2.11	-0.23	0.00	3.52	-43.48
27	27-	28	0.00	3.51	43.48	0.00	-2.10	-0.11
28	28-	29	0.00	2.10	0.11	0.00	-0.70	21.51
29	29-	30	0.00	0.70	-21.51	0.00	0.71	21.39
30	30-	31	0.00	-0.71	-21.39	0.00	2.12	-0.47
31	31-	32	0.00	0.27	0.47	0.00	-0.17	-0.23
32	32-	33	0.00	0.17	0.23	0.00	-0.12	-0.15
33	33-	34	0.00	0.12	0.15	0.00	0.13	-0.17
34	34-	35	0.00	-0.13	0.17	0.00	0.18	-0.26
35	35-	36	0.00	-0.18	0.26	0.00	0.28	-0.50
36	36-	37	0.00	2.14	0.50	0.00	-0.72	21.72
37	37-	38	0.00	0.72	-21.72	0.00	0.69	21.97
38	38-	39	0.00	-0.69	-21.97	0.00	2.11	0.23
39	39-	40	0.00	-2.11	-0.23	0.00	3.52	-43.48
40	40-	41	0.00	3.51	43.48	0.00	-2.10	-0.11
41	41-	42	0.00	2.10	0.11	0.00	-0.70	21.51
42	42-	43	0.00	0.70	-21.51	0.00	0.71	21.39
43	43-	44	0.00	-0.71	-21.39	0.00	2.12	-0.47

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LIGO BEAM TUBE -- BAKEOUT + SELF WT
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

No	Nodes		I-End			J-End		
	I	J	Axial (K)	Shear (K)	Moment (K-ft)	Axial (K)	Shear (K)	Moment (K-ft)
44	44-	45	0.00	0.27	0.47	0.00	-0.17	-0.23
45	45-	46	0.00	0.17	0.23	0.00	-0.12	-0.15
46	46-	47	0.00	0.12	0.15	0.00	0.13	-0.17
47	47-	48	0.00	-0.13	0.17	0.00	0.18	-0.26
48	48-	49	0.00	-0.18	0.26	0.00	0.28	-0.50
49	49-	50	0.00	2.14	0.50	0.00	-0.72	21.72
50	50-	51	0.00	0.72	-21.72	0.00	0.69	21.97
51	51-	52	0.00	-0.69	-21.97	0.00	2.11	0.23
52	52-	53	0.00	-2.11	-0.23	0.00	3.52	-43.48
53	53-	54	0.00	3.51	43.48	0.00	-2.10	-0.11
54	54-	55	0.00	2.10	0.11	0.00	-0.70	21.51
55	55-	56	0.00	0.70	-21.51	0.00	0.71	21.39
56	56-	57	0.00	-0.71	-21.39	0.00	2.12	-0.47
57	57-	58	0.00	0.27	0.47	0.00	-0.17	-0.23
58	58-	59	0.00	0.17	0.23	0.00	-0.12	-0.15
59	59-	60	0.00	0.12	0.15	0.00	0.13	-0.17
60	60-	61	0.00	-0.13	0.17	0.00	0.18	-0.25
61	61-	62	0.00	-0.18	0.25	0.00	0.28	-0.50
62	62-	63	0.00	2.13	0.50	0.00	-0.72	21.68
63	63-	64	0.00	0.72	-21.68	0.00	0.69	21.88
64	64-	65	0.00	-0.69	-21.88	0.00	2.11	0.10
65	65-	66	0.00	-2.11	-0.10	0.00	3.52	-43.66
66	66-	67	0.00	3.52	43.66	0.00	-2.12	-0.07
67	67-	68	0.00	2.12	0.07	0.00	-0.71	21.77
68	68-	69	0.00	0.71	-21.77	0.00	0.70	21.87
69	69-	70	0.00	-0.70	-21.87	0.00	2.10	0.23
70	70-	71	0.00	0.02	-0.23	0.00	0.08	0.19
71	71-	72	0.00	-0.08	-0.19	0.00	0.13	0.13
72	72-	73	0.00	-0.13	-0.13	0.00	0.38	-0.55
73	73-	74	0.00	-0.38	0.55	0.00	0.43	-0.79
74	74-	75	0.00	-0.43	0.79	0.00	0.53	-1.30
75	75-	76	0.00	2.85	1.30	0.00	-1.44	31.99
76	76-	77	0.00	1.44	-31.99	0.00	-0.02	43.31
77	77-	78	0.00	0.02	-43.31	0.00	1.39	32.64
78	78-	79	0.00	-1.39	-32.64	0.00	2.81	-0.00

3/09/94

LIGO BEAM TUBE -- BAKEOUT + SELF WT
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

=====
 Load Combination is 1 : DEAD LD + INSUL'N
 AISC Code Checks

No	Nodes		Maximum	0	Member Quarter Points			L	Shear
	I	J			1/4	1/2	3/4		
1	1-	2			-	Not Calculated	-		
2	2-	3			-	Not Calculated	-		
3	3-	4			-	Not Calculated	-		
4	4-	5			-	Not Calculated	-		
5	5-	6			-	Not Calculated	-		
6	6-	7			-	Not Calculated	-		
7	7-	8			-	Not Calculated	-		
8	8-	9			-	Not Calculated	-		
9	9-	10			-	Not Calculated	-		
10	10-	11			-	Not Calculated	-		
11	11-	12			-	Not Calculated	-		
12	12-	13			-	Not Calculated	-		
13	13-	14			-	Not Calculated	-		
14	14-	15			-	Not Calculated	-		
15	15-	16			-	Not Calculated	-		
16	16-	17			-	Not Calculated	-		
17	17-	18			-	Not Calculated	-		
18	18-	19			-	Not Calculated	-		
19	19-	20			-	Not Calculated	-		
20	20-	21			-	Not Calculated	-		
21	21-	22			-	Not Calculated	-		
22	22-	23			-	Not Calculated	-		
23	23-	24			-	Not Calculated	-		
24	24-	25			-	Not Calculated	-		
25	25-	26			-	Not Calculated	-		
26	26-	27			-	Not Calculated	-		
27	27-	28			-	Not Calculated	-		
28	28-	29			-	Not Calculated	-		
29	29-	30			-	Not Calculated	-		
30	30-	31			-	Not Calculated	-		
31	31-	32			-	Not Calculated	-		
32	32-	33			-	Not Calculated	-		
33	33-	34			-	Not Calculated	-		
34	34-	35			-	Not Calculated	-		
35	35-	36			-	Not Calculated	-		
36	36-	37			-	Not Calculated	-		
37	37-	38			-	Not Calculated	-		
38	38-	39			-	Not Calculated	-		
39	39-	40			-	Not Calculated	-		
40	40-	41			-	Not Calculated	-		
41	41-	42			-	Not Calculated	-		
42	42-	43			-	Not Calculated	-		
43	43-	44			-	Not Calculated	-		

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LIGO BEAM TUBE -- BAKEOUT + SELF WT
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

No	Nodes		Maximum	0	Member Quarter Points			L	Shear
	I	J			1/4	1/2	3/4		
44	44-	45			-	Not Calculated	-		
45	45-	46			-	Not Calculated	-		
46	46-	47			-	Not Calculated	-		
47	47-	48			-	Not Calculated	-		
48	48-	49			-	Not Calculated	-		
49	49-	50			-	Not Calculated	-		
50	50-	51			-	Not Calculated	-		
51	51-	52			-	Not Calculated	-		
52	52-	53			-	Not Calculated	-		
53	53-	54			-	Not Calculated	-		
54	54-	55			-	Not Calculated	-		
55	55-	56			-	Not Calculated	-		
56	56-	57			-	Not Calculated	-		
57	57-	58			-	Not Calculated	-		
58	58-	59			-	Not Calculated	-		
59	59-	60			-	Not Calculated	-		
60	60-	61			-	Not Calculated	-		
61	61-	62			-	Not Calculated	-		
62	62-	63			-	Not Calculated	-		
63	63-	64			-	Not Calculated	-		
64	64-	65			-	Not Calculated	-		
65	65-	66			-	Not Calculated	-		
66	66-	67			-	Not Calculated	-		
67	67-	68			-	Not Calculated	-		
68	68-	69			-	Not Calculated	-		
69	69-	70			-	Not Calculated	-		
70	70-	71			-	Not Calculated	-		
71	71-	72			-	Not Calculated	-		
72	72-	73			-	Not Calculated	-		
73	73-	74			-	Not Calculated	-		
74	74-	75			-	Not Calculated	-		
75	75-	76			-	Not Calculated	-		
76	76-	77			-	Not Calculated	-		
77	77-	78			-	Not Calculated	-		
78	78-	79			-	Not Calculated	-		

**RISA-2D MODEL OF LIGO BEAM TUBE MODULE: CASE #1A NODE NUMBERS
CASE #1A: SIMPLE SUPPORTS AT BOTH ENDS, TOTAL LENGTH = 780'
1" SUPPORT SETTLEMENT @ INTERIOR FIXED (I.E., UNGUIDED) SUPPORT**

NODE	X(FT)	DESCRIPTION
1	0.000	FIXED SUPP'T, ROTATION FREE
2	15.458	
3	30.917	
4	46.375	
5	61.833	GUIDED SUPP'T
6	63.500	END TUBE / START EJ STUB
7	64.083	END EJ STUB / START BELLOWS
8	65.583	END BELLOWS / START EJ STUB
9	66.167	END EJ STUB / START TUBE
10	67.833	GUIDED SUPP'T
11	83.375	
12	98.917	
13	114.458	
14	130.000	FIXED SUPP'T, ROTATION FREE
15	145.458	
16	160.917	
17	176.375	
18	191.833	GUIDED SUPP'T
19	193.500	END TUBE / START EJ STUB
20	194.083	END EJ STUB / START BELLOWS
21	195.583	END BELLOWS / START EJ STUB
22	196.167	END EJ STUB / START TUBE
23	197.833	GUIDED SUPP'T
24	213.375	
25	228.917	
26	244.458	
27	260.000	FIXED SUPP'T, ROTATION FREE
28	275.458	
29	290.917	
30	306.375	
31	321.833	GUIDED SUPP'T
32	323.500	END TUBE / START EJ STUB
33	324.083	END EJ STUB / START BELLOWS
34	325.583	END BELLOWS / START EJ STUB
35	326.167	END EJ STUB / START TUBE
36	327.833	GUIDED SUPP'T
37	343.375	
38	358.917	
39	374.458	
40	390.000	FIXED SUPP'T, ROTATION FREE =====> 1" SUPPORT SETTLEMENT IMPOSED
41	405.458	
42	420.917	
43	436.375	
44	451.833	GUIDED SUPP'T
45	453.500	END TUBE / START EJ STUB
46	454.083	END EJ STUB / START BELLOWS
47	455.583	END BELLOWS / START EJ STUB
48	456.167	END EJ STUB / START TUBE
49	457.833	GUIDED SUPP'T

SUBJECT RISA-2D MODEL OF BEAM TUBE MODULE, CASE #1A NODE NUMBERS, COORDINATES & DESCRIPTION	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD Rwj	MADE BY	CHKD BY	SHT 1 OF 2
	DATE 2/28/94	DATE 12 MAR 94	DATE	DATE	5.26

**RISA-2D MODEL OF LIGO BEAM TUBE MODULE: CASE #1A NODE NUMBERS
CASE #1A: SIMPLE SUPPORTS AT BOTH ENDS, TOTAL LENGTH = 780'
1" SUPPORT SETTLEMENT @ INTERIOR FIXED (I.E., UNGUIDED) SUPPORT**

NODE	X (FT)	DESCRIPTION
50	473.375	
51	488.917	
52	504.458	
53	520.000	FIXED SUPPT, ROTATION FREE
54	535.458	
55	550.917	
56	566.375	
57	581.833	GUIDED SUPPT
58	583.500	END TUBE / START EJ STUB
59	584.083	END EJ STUB / START BELLOWS
60	585.583	END BELLOWS / START EJ STUB
61	586.167	END EJ STUB / START TUBE
62	587.833	GUIDED SUPPT
63	603.375	
64	618.917	
65	634.458	
66	650.000	FIXED SUPPT, ROTATION FREE
67	665.458	
68	680.917	
69	696.375	
70	711.833	GUIDED SUPPT
71	713.500	END TUBE / START EJ STUB
72	714.083	END EJ STUB / START BELLOWS
73	715.583	END BELLOWS / START EJ STUB
74	716.167	END EJ STUB / START TUBE
75	717.833	GUIDED SUPPT
76	733.375	
77	748.917	
78	764.458	
79	780.000	FIXED SUPPT, ROTATION FREE

SUBJECT RISA-2D MODEL OF BEAM TUBE MODULE, CASE #1A NODE NUMBERS, COORDINATES & DESCRIPTION	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RW	MADE BY	CHKD BY	SHT 2 OF 2
	DATE 2/28/94	DATE 1/21/94	DATE	DATE	5.27

3/09/94

LIGO TUBE - 1" DIFF'L SETTLMT FIXED SUP
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Units Option : US Standard
 AISC Code Checks : 9th Edition ASD
 Shear Deformation: No
 P-Delta Effects : No
 Redesign : No
 Edge Forces : No
 A.S.I.F. : 1.333

SUBJECT	OFFICE CBI NIEC		REVISION		REFERENCE NO.
	MADE BY WJC	CHKD BY RW	MADE BY	CHKD BY	SHT ___ OF ___
	DATE 3-18-94	DATE 10 MAR 94	DATE	DATE	

3/09/94

LIGO TUBE - 1" DIFF'L SETTLMT FIXED SUP
780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Node No	Boundary Conditions						Temp. (F)
	X-Coord (ft)	Y-Coord (ft)	X-dof (in,K/in)	Y-dof (in,K/in)	Rotation (r,K-ft/r)		
1	0.00	0.00	R	R			0.00
2	15.46	0.00					0.00
3	30.92	0.00					0.00
4	46.37	0.00					0.00
5	61.83	0.00			R		0.00
6	62.92	0.00					0.00
7	63.50	0.00					0.00
8	66.17	0.00					0.00
9	66.75	0.00					0.00
10	67.83	0.00			R		0.00
11	83.37	0.00					0.00
12	98.92	0.00					0.00
13	114.46	0.00					0.00
14	130.00	0.00	R	R			0.00
15	145.46	0.00					0.00
16	160.92	0.00					0.00
17	176.37	0.00					0.00
18	191.83	0.00			R		0.00
19	192.92	0.00					0.00
20	193.50	0.00					0.00
21	196.17	0.00					0.00
22	196.75	0.00					0.00
23	197.83	0.00			R		0.00
24	213.37	0.00					0.00
25	228.92	0.00					0.00
26	244.46	0.00					0.00
27	260.00	0.00	R	R			0.00
28	275.46	0.00					0.00
29	290.92	0.00					0.00
30	306.37	0.00					0.00
31	321.83	0.00			R		0.00
32	322.92	0.00					0.00
33	323.50	0.00					0.00
34	326.17	0.00					0.00
35	326.75	0.00					0.00
36	327.83	0.00			R		0.00
37	343.37	0.00					0.00
38	358.92	0.00					0.00
39	374.46	0.00					0.00
40	390.00	0.00	R	D -1.00000			0.00
41	405.46	0.00					0.00
42	420.92	0.00					0.00
43	436.37	0.00					0.00
44	451.83	0.00			R		0.00
45	452.92	0.00					0.00

3/09/94

LIGO TUBE - 1" DIFF'L SETTLMT FIXED SUP
780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Node No	X-Coord (ft)	Y-Coord (ft)	Boundary Conditions			Temp. (F)
			X-dof (in,K/in)	Y-dof (in,K/in)	Rotation (r,K-ft/r)	
46	453.50	0.00				0.00
47	456.17	0.00				0.00
48	456.75	0.00				0.00
49	457.83	0.00			R	0.00
50	473.37	0.00				0.00
51	488.92	0.00				0.00
52	504.46	0.00				0.00
53	520.00	0.00	R	R		0.00
54	535.46	0.00				0.00
55	550.92	0.00				0.00
56	566.37	0.00				0.00
57	581.83	0.00			R	0.00
58	582.92	0.00				0.00
59	583.50	0.00				0.00
60	586.17	0.00				0.00
61	586.75	0.00				0.00
62	587.83	0.00			R	0.00
63	603.37	0.00				0.00
64	618.92	0.00				0.00
65	634.46	0.00				0.00
66	650.00	0.00	R	R		0.00
67	665.46	0.00				0.00
68	680.92	0.00				0.00
69	696.37	0.00				0.00
70	711.83	0.00			R	0.00
71	712.92	0.00				0.00
72	713.50	0.00				0.00
73	716.17	0.00				0.00
74	716.75	0.00				0.00
75	717.83	0.00			R	0.00
76	733.37	0.00				0.00
77	748.92	0.00				0.00
78	764.46	0.00				0.00
79	780.00	0.00	R	R		0.00

LIGO TUBE - 1" DIFF'L SETTLMT FIXED SUP
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Material Label	Elastic Modulus (Ksi)	Poisson's Ratio	Thermal Coefficient (F)	Weight Density (K/ft3)	Yield Stress (Fy) (Ksi)
1	27000.00	0.30000	0.65000	0.000	19.200

3/09/94

LIGO TUBE - 1" DIFF'L SETTLMT FIXED SUP
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Section Label	Database Shape	Matl. Set	Area (in ²)	Moment of Inertia (in ⁴)	As Coef	y/y
BEAMTUBE		1	19.19	5731.030	1.20	
STUBEND		1	16.13	4820.000	1.20	
EXPJOINT		1	0.01	1.803	1.20	

3/09/94

LIGO TUBE - 1" DIFF'L SETTLMT FIXED SUP
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

I No	I Node	J Node	Section	I Releases			J			End Offsets		Length (ft)
				x	y	z	x	y	z	Sec	Sway	
1	1	-	2	BEAMTUBE								15.46
2	2	-	3	BEAMTUBE								15.46
3	3	-	4	BEAMTUBE								15.45
4	4	-	5	BEAMTUBE								15.46
5	5	-	6	BEAMTUBE								1.09
6	6	-	7	STUBEND								0.58
7	7	-	8	EXPJOINT								2.67
8	8	-	9	STUBEND								0.58
9	9	-	10	BEAMTUBE								1.08
10	10	-	11	BEAMTUBE								15.54
11	11	-	12	BEAMTUBE								15.55
12	12	-	13	BEAMTUBE								15.54
13	13	-	14	BEAMTUBE								15.54
14	14	-	15	BEAMTUBE								15.46
15	15	-	16	BEAMTUBE								15.46
16	16	-	17	BEAMTUBE								15.45
17	17	-	18	BEAMTUBE								15.46
18	18	-	19	BEAMTUBE								1.09
19	19	-	20	STUBEND								0.58
20	20	-	21	EXPJOINT								2.67
21	21	-	22	STUBEND								0.58
22	22	-	23	BEAMTUBE								1.08
23	23	-	24	BEAMTUBE								15.54
24	24	-	25	BEAMTUBE								15.55
25	25	-	26	BEAMTUBE								15.54
26	26	-	27	BEAMTUBE								15.54
27	27	-	28	BEAMTUBE								15.46
28	28	-	29	BEAMTUBE								15.46
29	29	-	30	BEAMTUBE								15.45
30	30	-	31	BEAMTUBE								15.46
31	31	-	32	BEAMTUBE								1.09
32	32	-	33	STUBEND								0.58
33	33	-	34	EXPJOINT								2.67
34	34	-	35	STUBEND								0.58
35	35	-	36	BEAMTUBE								1.08
36	36	-	37	BEAMTUBE								15.54
37	37	-	38	BEAMTUBE								15.55
38	38	-	39	BEAMTUBE								15.54
39	39	-	40	BEAMTUBE								15.54
40	40	-	41	BEAMTUBE								15.46
41	41	-	42	BEAMTUBE								15.46
42	42	-	43	BEAMTUBE								15.45
43	43	-	44	BEAMTUBE								15.46
44	44	-	45	BEAMTUBE								1.09
45	45	-	46	STUBEND								0.58

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LIGO TUBE - 1" DIFF'L SETTLMT FIXED SUP
780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

I No	Node	J Node	Section	I Releases			J			End Offsets		Length (ft)
				x	y	z	x	y	z	Sec	Sway	
46	46	-	47	EXPJOINT								2.67
47	47	-	48	STUBEND								0.58
48	48	-	49	BEAMTUBE								1.08
49	49	-	50	BEAMTUBE								15.54
50	50	-	51	BEAMTUBE								15.55
51	51	-	52	BEAMTUBE								15.54
52	52	-	53	BEAMTUBE								15.54
53	53	-	54	BEAMTUBE								15.46
54	54	-	55	BEAMTUBE								15.46
55	55	-	56	BEAMTUBE								15.45
56	56	-	57	BEAMTUBE								15.46
57	57	-	58	BEAMTUBE								1.09
58	58	-	59	STUBEND								0.58
59	59	-	60	EXPJOINT								2.67
60	60	-	61	STUBEND								0.58
61	61	-	62	BEAMTUBE								1.08
62	62	-	63	BEAMTUBE								15.54
63	63	-	64	BEAMTUBE								15.55
64	64	-	65	BEAMTUBE								15.54
65	65	-	66	BEAMTUBE								15.54
66	66	-	67	BEAMTUBE								15.46
67	67	-	68	BEAMTUBE								15.46
68	68	-	69	BEAMTUBE								15.45
69	69	-	70	BEAMTUBE								15.46
70	70	-	71	BEAMTUBE								1.09
71	71	-	72	STUBEND								0.58
72	72	-	73	EXPJOINT								2.67
73	73	-	74	STUBEND								0.58
74	74	-	75	BEAMTUBE								1.08
75	75	-	76	BEAMTUBE								15.54
76	76	-	77	BEAMTUBE								15.55
77	77	-	78	BEAMTUBE								15.54
78	78	-	79	BEAMTUBE								15.54

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LIGO TUBE - 1" DIFF'L SETTLMT FIXED SUP
780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

No	Nodes		Maximum	0	Member Quarter Points			L	Shear
	I	J			1/4	1/2	3/4		
44	44-	45			-	Not	Calculated	-	
45	45-	46			-	Not	Calculated	-	
46	46-	47			-	Not	Calculated	-	
47	47-	48			-	Not	Calculated	-	
48	48-	49			-	Not	Calculated	-	
49	49-	50			-	Not	Calculated	-	
50	50-	51			-	Not	Calculated	-	
51	51-	52			-	Not	Calculated	-	
52	52-	53			-	Not	Calculated	-	
53	53-	54			-	Not	Calculated	-	
54	54-	55			-	Not	Calculated	-	
55	55-	56			-	Not	Calculated	-	
56	56-	57			-	Not	Calculated	-	
57	57-	58			-	Not	Calculated	-	
58	58-	59			-	Not	Calculated	-	
59	59-	60			-	Not	Calculated	-	
60	60-	61			-	Not	Calculated	-	
61	61-	62			-	Not	Calculated	-	
62	62-	63			-	Not	Calculated	-	
63	63-	64			-	Not	Calculated	-	
64	64-	65			-	Not	Calculated	-	
65	65-	66			-	Not	Calculated	-	
66	66-	67			-	Not	Calculated	-	
67	67-	68			-	Not	Calculated	-	
68	68-	69			-	Not	Calculated	-	
69	69-	70			-	Not	Calculated	-	
70	70-	71			-	Not	Calculated	-	
71	71-	72			-	Not	Calculated	-	
72	72-	73			-	Not	Calculated	-	
73	73-	74			-	Not	Calculated	-	
74	74-	75			-	Not	Calculated	-	
75	75-	76			-	Not	Calculated	-	
76	76-	77			-	Not	Calculated	-	
77	77-	78			-	Not	Calculated	-	
78	78-	79			-	Not	Calculated	-	

**RISA-2D MODEL OF LIGO BEAM TUBE MODULE: CASE #1B NODE NUMBERS
CASE #1B: SIMPLE SUPPORTS AT BOTH ENDS, TOTAL LENGTH = 780'
1" SUPPORT SETTLEMENT @ INTERIOR GUIDED SUPPORT**

NODE	X (FT)	DESCRIPTION	
1	0.000	FIXED SUPPT, ROTATION FREE	
2	15.458		
3	30.917		
4	46.375		
5	61.833	GUIDED SUPPT	
6	63.500	END TUBE / START EJ STUB	
7	64.083	END EJ STUB / START BELLOWS	
8	65.583	END BELLOWS / START EJ STUB	
9	66.167	END EJ STUB / START TUBE	
10	67.833	GUIDED SUPPT	
11	83.375		
12	98.917		
13	114.458		
14	130.000	FIXED SUPPT, ROTATION FREE	
15	145.458		
16	160.917		
17	176.375		
18	191.833	GUIDED SUPPT	
19	193.500	END TUBE / START EJ STUB	
20	194.083	END EJ STUB / START BELLOWS	
21	195.583	END BELLOWS / START EJ STUB	
22	196.167	END EJ STUB / START TUBE	
23	197.833	GUIDED SUPPT	
24	213.375		
25	228.917		
26	244.458		
27	260.000	FIXED SUPPT, ROTATION FREE	
28	275.458		
29	290.917		
30	306.375		
31	321.833	GUIDED SUPPT	====> 1" SUPPORT SETTLEMENT IMPOSED
32	323.500	END TUBE / START EJ STUB	
33	324.083	END EJ STUB / START BELLOWS	
34	325.583	END BELLOWS / START EJ STUB	
35	326.167	END EJ STUB / START TUBE	
36	327.833	GUIDED SUPPT	====> 1" SUPPORT SETTLEMENT IMPOSED
37	343.375		
38	358.917		
39	374.458		
40	390.000	FIXED SUPPT, ROTATION FREE	
41	405.458		
42	420.917		
43	436.375		
44	451.833	GUIDED SUPPT	
45	453.500	END TUBE / START EJ STUB	
46	454.083	END EJ STUB / START BELLOWS	
47	455.583	END BELLOWS / START EJ STUB	
48	456.167	END EJ STUB / START TUBE	
49	457.833	GUIDED SUPPT	

SUBJECT RISA-2D MODEL OF BEAM TUBE MODULE, CASE #1B NODE NUMBERS, COORDINATES & DESCRIPTION	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD Rjw	MADE BY	CHKD BY	SHT 1 OF 2
	DATE 2/28/94	DATE 18 MAR 94	DATE	DATE	5.48.

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LIGO TUBE - 1" DIFF'L SETTLMT FIXED SUP
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

I No	Node	J Node	Unbraced Lengths			K Factors		Bending Coefs	
			Lb-in (ft)	Lb-out (ft)	Lc (ft)	In	Out	Cm	Cb
1	1	-	2						
2	2	-	3						
3	3	-	4						
4	4	-	5						
5	5	-	6						
6	6	-	7						
7	7	-	8						
8	8	-	9						
9	9	-	10						
10	10	-	11						
11	11	-	12						
12	12	-	13						
13	13	-	14						
14	14	-	15						
15	15	-	16						
16	16	-	17						
17	17	-	18						
18	18	-	19						
19	19	-	20						
20	20	-	21						
21	21	-	22						
22	22	-	23						
23	23	-	24						
24	24	-	25						
25	25	-	26						
26	26	-	27						
27	27	-	28						
28	28	-	29						
29	29	-	30						
30	30	-	31						
31	31	-	32						
32	32	-	33						
33	33	-	34						
34	34	-	35						
35	35	-	36						
36	36	-	37						
37	37	-	38						
38	38	-	39						
39	39	-	40						
40	40	-	41						
41	41	-	42						
42	42	-	43						
43	43	-	44						
44	44	-	45						
45	45	-	46						

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LIGO TUBE - 1" DIFF'L SETTLMNT FIXED SUP
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

I No	Node	J Node	Unbraced Lengths			K Factors		Bending Coefs	
			Lb-in (ft)	Lb-out (ft)	Lc (ft)	In	Out	Cm	Cb
46	46	-	47						
47	47	-	48						
48	48	-	49						
49	49	-	50						
50	50	-	51						
51	51	-	52						
52	52	-	53						
53	53	-	54						
54	54	-	55						
55	55	-	56						
56	56	-	57						
57	57	-	58						
58	58	-	59						
59	59	-	60						
60	60	-	61						
61	61	-	62						
62	62	-	63						
63	63	-	64						
64	64	-	65						
65	65	-	66						
66	66	-	67						
67	67	-	68						
68	68	-	69						
69	69	-	70						
70	70	-	71						
71	71	-	72						
72	72	-	73						
73	73	-	74						
74	74	-	75						
75	75	-	76						
76	76	-	77						
77	77	-	78						
78	78	-	79						

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LIGO TUBE - 1" DIFF'L SETTLMT FIXED SUP
780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

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BLC No.	Basic Load Case Description	Load Totals		
		Nodal	Point	Dist.
1	1" FIXED SUPPORT SETTLEMENT			78

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LIGO TUBE - 1" DIFF'L SETTLMT FIXED SUP
780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Member Distributed Loads,BLC 1: 1" FIXED SUPPORT SETTLEMENT

Membr No	I Node	J Node	Dir	Start Magnitude (K/ft,F)	End Magnitude (K/ft,F)	Start Location (ft)	End Location (ft)
1	1	2	Y	-0.001	-0.001	0.000	15.460
2	2	3	Y	-0.001	-0.001	0.000	15.460
3	3	4	Y	-0.001	-0.001	0.000	15.449
4	4	5	Y	-0.001	-0.001	0.000	15.460
5	5	6	Y	-0.001	-0.001	0.000	1.089
6	6	7	Y	-0.001	-0.001	0.000	0.580
7	7	8	Y	-0.001	-0.001	0.000	2.669
8	8	9	Y	-0.001	-0.001	0.000	0.580
9	9	10	Y	-0.001	-0.001	0.000	1.080
10	10	11	Y	-0.001	-0.001	0.000	15.540
11	11	12	Y	-0.001	-0.001	0.000	15.550
12	12	13	Y	-0.001	-0.001	0.000	15.539
13	13	14	Y	-0.001	-0.001	0.000	15.540
14	14	15	Y	-0.001	-0.001	0.000	15.460
15	15	16	Y	-0.001	-0.001	0.000	15.459
16	16	17	Y	-0.001	-0.001	0.000	15.449
17	17	18	Y	-0.001	-0.001	0.000	15.460
18	18	19	Y	-0.001	-0.001	0.000	1.089
19	19	20	Y	-0.001	-0.001	0.000	0.580
20	20	21	Y	-0.001	-0.001	0.000	2.669
21	21	22	Y	-0.001	-0.001	0.000	0.580
22	22	23	Y	-0.001	-0.001	0.000	1.080
23	23	24	Y	-0.001	-0.001	0.000	15.539
24	24	25	Y	-0.001	-0.001	0.000	15.550
25	25	26	Y	-0.001	-0.001	0.000	15.540
26	26	27	Y	-0.001	-0.001	0.000	15.539
27	27	28	Y	-0.001	-0.001	0.000	15.459
28	28	29	Y	-0.001	-0.001	0.000	15.460
29	29	30	Y	-0.001	-0.001	0.000	15.449
30	30	31	Y	-0.001	-0.001	0.000	15.459
31	31	32	Y	-0.001	-0.001	0.000	1.090
32	32	33	Y	-0.001	-0.001	0.000	0.579
33	33	34	Y	-0.001	-0.001	0.000	2.670
34	34	35	Y	-0.001	-0.001	0.000	0.579
35	35	36	Y	-0.001	-0.001	0.000	1.079
36	36	37	Y	-0.001	-0.001	0.000	15.540
37	37	38	Y	-0.001	-0.001	0.000	15.550
38	38	39	Y	-0.001	-0.001	0.000	15.539
39	39	40	Y	-0.001	-0.001	0.000	15.540
40	40	41	Y	-0.001	-0.001	0.000	15.459
41	41	42	Y	-0.001	-0.001	0.000	15.460
42	42	43	Y	-0.001	-0.001	0.000	15.449
43	43	44	Y	-0.001	-0.001	0.000	15.459
44	44	45	Y	-0.001	-0.001	0.000	1.090

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LIGO TUBE - 1" DIFF'L SETTLMNT FIXED SUP
780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Memb No	I Node	J Node	Dir	Start Magnitude (K/ft,F)	End Magnitude (K/ft,F)	Start Location (ft)	End Location (ft)
45	45	46	Y	-0.001	-0.001	0.000	0.579
46	46	47	Y	-0.001	-0.001	0.000	2.670
47	47	48	Y	-0.001	-0.001	0.000	0.579
48	48	49	Y	-0.001	-0.001	0.000	1.079
49	49	50	Y	-0.001	-0.001	0.000	15.540
50	50	51	Y	-0.001	-0.001	0.000	15.550
51	51	52	Y	-0.001	-0.001	0.000	15.539
52	52	53	Y	-0.001	-0.001	0.000	15.540
53	53	54	Y	-0.001	-0.001	0.000	15.460
54	54	55	Y	-0.001	-0.001	0.000	15.459
55	55	56	Y	-0.001	-0.001	0.000	15.450
56	56	57	Y	-0.001	-0.001	0.000	15.460
57	57	58	Y	-0.001	-0.001	0.000	1.089
58	58	59	Y	-0.001	-0.001	0.000	0.580
59	59	60	Y	-0.001	-0.001	0.000	2.669
60	60	61	Y	-0.001	-0.001	0.000	0.580
61	61	62	Y	-0.001	-0.001	0.000	1.080
62	62	63	Y	-0.001	-0.001	0.000	15.539
63	63	64	Y	-0.001	-0.001	0.000	15.549
64	64	65	Y	-0.001	-0.001	0.000	15.540
65	65	66	Y	-0.001	-0.001	0.000	15.539
66	66	67	Y	-0.001	-0.001	0.000	15.460
67	67	68	Y	-0.001	-0.001	0.000	15.459
68	68	69	Y	-0.001	-0.001	0.000	15.449
69	69	70	Y	-0.001	-0.001	0.000	15.460
70	70	71	Y	-0.001	-0.001	0.000	1.089
71	71	72	Y	-0.001	-0.001	0.000	0.580
72	72	73	Y	-0.001	-0.001	0.000	2.669
73	73	74	Y	-0.001	-0.001	0.000	0.580
74	74	75	Y	-0.001	-0.001	0.000	1.080
75	75	76	Y	-0.001	-0.001	0.000	15.539
76	76	77	Y	-0.001	-0.001	0.000	15.550
77	77	78	Y	-0.001	-0.001	0.000	15.540
78	78	79	Y	-0.001	-0.001	0.000	15.539

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LIGO TUBE - 1" DIFF'L SETTLMT FIXED SUP
780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

=====

Load Combination No.	Description	Self Wt Dir	Wt Fac	BLC Fac	BLC Fac	BLC Fac	BLC Fac	BLC Fac	DYNA	W S	E V
1	1" SUPPORT DISPL			1	1						

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LIGO TUBE - 1" DIFF'L SETTLMT FIXED SUP
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Dynamic Analysis Data

Number of modes (frequencies) : 15
 Basic Load Case for masses : 1
 BLC mass direction of action : Y only
 Acceleration of Gravity : 32.20 ft/sec**2

Load Combination is 1 : 1" SUPPORT DISPL
 Nodal Displacements

Node	Global X (in)	Global Y (in)	Rotation (rad)
1	0.00000	-0.00000	-0.00001
2	0.00000	-0.00149	-0.00001
3	0.00000	-0.00209	0.00000
4	0.00000	-0.00148	0.00001
5	0.00000	-0.00000	0.00001
6	0.00000	0.00012	0.00001
7	0.00000	0.00018	0.00001
8	0.00000	0.00009	-0.00000
9	0.00000	0.00006	-0.00000
10	0.00000	-0.00000	-0.00000
11	0.00000	-0.00075	-0.00000
12	0.00000	-0.00089	0.00000
13	0.00000	-0.00043	0.00000
14	0.00000	-0.00000	0.00000
15	0.00000	-0.00037	-0.00000
16	0.00000	-0.00080	-0.00000
17	0.00000	-0.00068	0.00000
18	0.00000	-0.00000	0.00000
19	0.00000	0.00006	0.00000
20	0.00000	0.00008	0.00000
21	0.00000	-0.00006	0.00000
22	0.00000	-0.00004	0.00000
23	0.00000	-0.00000	0.00000
24	0.00000	0.00066	0.00000
25	0.00000	0.00137	0.00000
26	0.00000	0.00156	-0.00000
27	0.00000	-0.00000	-0.00002
28	0.00000	-0.00405	-0.00002
29	0.00000	-0.00760	-0.00001
30	0.00000	-0.00719	0.00002
31	0.00000	0.00000	0.00006
32	0.00000	0.00082	0.00006
33	0.00000	0.00127	0.00007
34	0.00000	0.03905	-0.00196
35	0.00000	0.02542	-0.00196
36	0.00000	-0.00000	-0.00196

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LIGO TUBE - 1" DIFF'L SETTLMT FIXED SUP
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Node	Global X (in)	Global Y (in)	Rotation (rad)
37	0.00000	-0.36263	-0.00188
38	0.00000	-0.68428	-0.00152
39	0.00000	-0.91346	-0.00089
40	0.00000	-1.00000	0.00001
41	0.00000	-0.91155	0.00090
42	0.00000	-0.68198	0.00153
43	0.00000	-0.36143	0.00188
44	0.00000	-0.00000	0.00197
45	0.00000	0.02569	0.00196
46	0.00000	0.03935	0.00196
47	0.00000	0.00127	-0.00007
48	0.00000	0.00082	-0.00006
49	0.00000	0.00000	-0.00006
50	0.00000	-0.00728	-0.00002
51	0.00000	-0.00770	0.00001
52	0.00000	-0.00411	0.00002
53	0.00000	-0.00000	0.00002
54	0.00000	0.00157	0.00000
55	0.00000	0.00140	-0.00000
56	0.00000	0.00068	-0.00000
57	0.00000	-0.00000	-0.00000
58	0.00000	-0.00004	-0.00000
59	0.00000	-0.00007	-0.00000
60	0.00000	0.00009	-0.00000
61	0.00000	0.00006	-0.00000
62	0.00000	-0.00000	-0.00000
63	0.00000	-0.00070	-0.00000
64	0.00000	-0.00084	0.00000
65	0.00000	-0.00039	0.00000
66	0.00000	-0.00000	-0.00000
67	0.00000	-0.00041	-0.00000
68	0.00000	-0.00086	-0.00000
69	0.00000	-0.00073	0.00000
70	0.00000	-0.00000	0.00000
71	0.00000	0.00006	0.00000
72	0.00000	0.00009	0.00000
73	0.00000	0.00018	-0.00001
74	0.00000	0.00012	-0.00001
75	0.00000	-0.00000	-0.00001
76	0.00000	-0.00151	-0.00001
77	0.00000	-0.00213	-0.00000
78	0.00000	-0.00152	0.00001
79	0.00000	-0.00000	0.00001

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LIGO TUBE - 1" DIFF'L SETTLMT FIXED SUP
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Load Combination is 1 : 1" SUPPORT DISPL
 Spring Reactions

Node	Global X (K)	Global Y (K)	Moment (K-ft)
1	0.00000	0.03069	0.00000
5	0.00000	0.03679	0.00000
10	0.00000	0.02374	0.00000
14	0.00000	0.07710	0.00000
18	0.00000	0.03130	0.00000
23	0.00000	0.00784	0.00000
27	0.00000	0.15946	0.00000
31	0.00000	-1.25177	0.00000
36	0.00000	2.45398	0.00000
40	0.00000	-2.36558	0.00000
44	0.00000	2.46705	0.00000
49	0.00000	-1.25746	0.00000
53	0.00000	0.15946	0.00000
57	0.00000	0.00729	0.00000
62	0.00000	0.03166	0.00000
66	0.00000	0.07710	0.00000
70	0.00000	0.02339	0.00000
75	0.00000	0.03710	0.00000
79	0.00000	0.03086	0.00000
Totals	0.00000	0.78000	0.00000

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LIGO TUBE - 1" DIFF'L SETTLMT FIXED SUP
780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Load Combination is 1 : 1" SUPPORT DISPL
Member End Forces

No	Nodes		I-End			J-End		
	I	J	Axial (K)	Shear (K)	Moment (K-ft)	Axial (K)	Shear (K)	Moment (K-ft)
1	1-	2	0.00	0.03	-0.00	0.00	-0.02	0.35
2	2-	3	0.00	0.02	-0.35	0.00	0.00	0.47
3	3-	4	0.00	-0.00	-0.47	0.00	0.02	0.35
4	4-	5	0.00	-0.02	-0.35	0.00	0.03	-0.01
5	5-	6	0.00	0.01	0.01	0.00	-0.00	-0.01
6	6-	7	0.00	0.00	0.01	0.00	-0.00	-0.01
7	7-	8	0.00	0.00	0.01	0.00	-0.00	0.00
8	8-	9	0.00	0.00	-0.00	0.00	-0.00	0.00
9	9-	10	0.00	0.00	-0.00	0.00	0.00	0.00
10	10-	11	0.00	0.02	-0.00	0.00	-0.01	0.24
11	11-	12	0.00	0.01	-0.24	0.00	0.01	0.25
12	12-	13	0.00	-0.01	-0.25	0.00	0.02	0.01
13	13-	14	0.00	-0.02	-0.01	0.00	0.04	-0.48
14	14-	15	0.00	0.04	0.48	0.00	-0.02	-0.00
15	15-	16	0.00	0.02	0.00	0.00	-0.01	0.23
16	16-	17	0.00	0.01	-0.23	0.00	0.01	0.23
17	17-	18	0.00	-0.01	-0.23	0.00	0.02	-0.02
18	18-	19	0.00	0.01	0.02	0.00	-0.01	-0.01
19	19-	20	0.00	0.01	0.01	0.00	-0.01	-0.01
20	20-	21	0.00	0.01	0.01	0.00	-0.00	0.01
21	21-	22	0.00	0.00	-0.01	0.00	-0.00	0.01
22	22-	23	0.00	0.00	-0.01	0.00	-0.00	0.01
23	23-	24	0.00	0.01	-0.01	0.00	0.01	0.04
24	24-	25	0.00	-0.01	-0.04	0.00	0.02	-0.17
25	25-	26	0.00	-0.02	0.17	0.00	0.04	-0.63
26	26-	27	0.00	-0.04	0.63	0.00	0.05	-1.32
27	27-	28	0.00	0.11	1.32	0.00	-0.09	0.21
28	28-	29	0.00	0.09	-0.21	0.00	-0.08	1.50
29	29-	30	0.00	0.08	-1.50	0.00	-0.06	2.56
30	30-	31	0.00	0.06	-2.56	0.00	-0.05	3.37
31	31-	32	0.00	-1.21	-3.37	0.00	1.21	2.06
32	32-	33	0.00	-1.21	-2.06	0.00	1.21	1.36
33	33-	34	0.00	-1.21	-1.36	0.00	1.21	-1.87
34	34-	35	0.00	-1.21	1.87	0.00	1.21	-2.57
35	35-	36	0.00	-1.21	2.57	0.00	1.21	-3.88
36	36-	37	0.00	1.24	3.88	0.00	-1.23	15.29
37	37-	38	0.00	1.23	-15.29	0.00	-1.21	34.23
38	38-	39	0.00	1.21	-34.23	0.00	-1.19	52.91
39	39-	40	0.00	1.19	-52.91	0.00	-1.18	71.36
40	40-	41	0.00	-1.19	-71.36	0.00	1.20	52.89
41	41-	42	0.00	-1.20	-52.89	0.00	1.22	34.19
42	42-	43	0.00	-1.22	-34.19	0.00	1.23	15.26
43	43-	44	0.00	-1.23	-15.26	0.00	1.25	-3.91

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LIGO TUBE - 1" DIFF'L SETTLMT FIXED SUP
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

No	Nodes		I-End			J-End		
	I	J	Axial (K)	Shear (K)	Moment (K-ft)	Axial (K)	Shear (K)	Moment (K-ft)
44	44-	45	0.00	1.22	3.91	0.00	-1.22	-2.59
45	45-	46	0.00	1.22	2.59	0.00	-1.22	-1.88
46	46-	47	0.00	1.22	1.88	0.00	-1.21	1.37
47	47-	48	0.00	1.21	-1.37	0.00	-1.21	2.07
48	48-	49	0.00	1.21	-2.07	0.00	-1.21	3.38
49	49-	50	0.00	-0.04	-3.38	0.00	0.06	2.57
50	50-	51	0.00	-0.06	-2.57	0.00	0.08	1.51
51	51-	52	0.00	-0.08	-1.51	0.00	0.09	0.21
52	52-	53	0.00	-0.09	-0.21	0.00	0.11	-1.33
53	53-	54	0.00	0.05	1.33	0.00	-0.04	-0.64
54	54-	55	0.00	0.04	0.64	0.00	-0.02	-0.18
55	55-	56	0.00	0.02	0.18	0.00	-0.01	0.03
56	56-	57	0.00	0.01	-0.03	0.00	0.01	0.01
57	57-	58	0.00	-0.00	-0.01	0.00	0.00	0.01
58	58-	59	0.00	-0.00	-0.01	0.00	0.00	0.01
59	59-	60	0.00	-0.00	-0.01	0.00	0.01	-0.01
60	60-	61	0.00	-0.01	0.01	0.00	0.01	-0.01
61	61-	62	0.00	-0.01	0.01	0.00	0.01	-0.02
62	62-	63	0.00	0.02	0.02	0.00	-0.01	0.23
63	63-	64	0.00	0.01	-0.23	0.00	0.01	0.24
64	64-	65	0.00	-0.01	-0.24	0.00	0.02	0.00
65	65-	66	0.00	-0.02	-0.00	0.00	0.04	-0.48
66	66-	67	0.00	0.04	0.48	0.00	-0.02	0.00
67	67-	68	0.00	0.02	-0.00	0.00	-0.01	0.24
68	68-	69	0.00	0.01	-0.24	0.00	0.01	0.24
69	69-	70	0.00	-0.01	-0.24	0.00	0.02	0.00
70	70-	71	0.00	0.00	-0.00	0.00	0.00	0.00
71	71-	72	0.00	-0.00	-0.00	0.00	0.00	0.00
72	72-	73	0.00	-0.00	-0.00	0.00	0.00	-0.01
73	73-	74	0.00	-0.00	0.01	0.00	0.00	-0.01
74	74-	75	0.00	-0.00	0.01	0.00	0.01	-0.01
75	75-	76	0.00	0.03	0.01	0.00	-0.02	0.35
76	76-	77	0.00	0.02	-0.35	0.00	-0.00	0.48
77	77-	78	0.00	0.00	-0.48	0.00	0.02	0.36
78	78-	79	0.00	-0.02	-0.36	0.00	0.03	-0.00

LIGO TUBE - 1" DIFF'L SETTLMT FIXED SUP
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Load Combination is 1 : 1" SUPPORT DISPL
 AISC Code Checks

No	Nodes		Maximum	0	Member Quarter Points			L	Shear
	I	J			1/4	1/2	3/4		
1	1-	2			-	Not Calculated	-		
2	2-	3			-	Not Calculated	-		
3	3-	4			-	Not Calculated	-		
4	4-	5			-	Not Calculated	-		
5	5-	6			-	Not Calculated	-		
6	6-	7			-	Not Calculated	-		
7	7-	8			-	Not Calculated	-		
8	8-	9			-	Not Calculated	-		
9	9-	10			-	Not Calculated	-		
10	10-	11			-	Not Calculated	-		
11	11-	12			-	Not Calculated	-		
12	12-	13			-	Not Calculated	-		
13	13-	14			-	Not Calculated	-		
14	14-	15			-	Not Calculated	-		
15	15-	16			-	Not Calculated	-		
16	16-	17			-	Not Calculated	-		
17	17-	18			-	Not Calculated	-		
18	18-	19			-	Not Calculated	-		
19	19-	20			-	Not Calculated	-		
20	20-	21			-	Not Calculated	-		
21	21-	22			-	Not Calculated	-		
22	22-	23			-	Not Calculated	-		
23	23-	24			-	Not Calculated	-		
24	24-	25			-	Not Calculated	-		
25	25-	26			-	Not Calculated	-		
26	26-	27			-	Not Calculated	-		
27	27-	28			-	Not Calculated	-		
28	28-	29			-	Not Calculated	-		
29	29-	30			-	Not Calculated	-		
30	30-	31			-	Not Calculated	-		
31	31-	32			-	Not Calculated	-		
32	32-	33			-	Not Calculated	-		
33	33-	34			-	Not Calculated	-		
34	34-	35			-	Not Calculated	-		
35	35-	36			-	Not Calculated	-		
36	36-	37			-	Not Calculated	-		
37	37-	38			-	Not Calculated	-		
38	38-	39			-	Not Calculated	-		
39	39-	40			-	Not Calculated	-		
40	40-	41			-	Not Calculated	-		
41	41-	42			-	Not Calculated	-		
42	42-	43			-	Not Calculated	-		
43	43-	44			-	Not Calculated	-		

**RISA-2D MODEL OF LIGO BEAM TUBE MODULE: CASE #1B NODE NUMBERS
CASE #1B: SIMPLE SUPPORTS AT BOTH ENDS, TOTAL LENGTH = 780'
1" SUPPORT SETTLEMENT @ INTERIOR GUIDED SUPPORT**

NODE	X(FT)	DESCRIPTION
50	473.375	
51	488.917	
52	504.458	
53	520.000	FIXED SUPP'T, ROTATION FREE
54	535.458	
55	550.917	
56	566.375	
57	581.833	GUIDED SUPP'T
58	583.500	END TUBE / START EJ STUB
59	584.083	END EJ STUB / START BELLOWS
60	585.583	END BELLOWS / START EJ STUB
61	586.167	END EJ STUB / START TUBE
62	587.833	GUIDED SUPP'T
63	603.375	
64	618.917	
65	634.458	
66	650.000	FIXED SUPP'T, ROTATION FREE
67	665.458	
68	680.917	
69	696.375	
70	711.833	GUIDED SUPP'T
71	713.500	END TUBE / START EJ STUB
72	714.083	END EJ STUB / START BELLOWS
73	715.583	END BELLOWS / START EJ STUB
74	716.167	END EJ STUB / START TUBE
75	717.833	GUIDED SUPP'T
76	733.375	
77	748.917	
78	764.458	
79	780.000	FIXED SUPP'T, ROTATION FREE

SUBJECT RISA-2D MODEL OF BEAM TUBE MODULE, CASE #1B NODE NUMBERS, COORDINATES & DESCRIPTION	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RJW	MADE BY	CHKD BY	SHT 2 OF 2
	DATE 2/28/94	DATE 10/11/94	DATE	DATE	5.49

LIGO TUBE - 1" DIFF'L SETTLMT GUIDE SUP
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Units Option : US Standard

AISC Code Checks : 9th Edition ASD

Shear Deformation: No

P-Delta Effects : No

Redesign : No

Edge Forces : No

A.S.I.F. : 1.333

SUBJECT	OFFICE		REVISION		REFERENCE NO.
	CBI	NOE C			
	MADE BY	CHKD BY	MADE BY	CHKD BY	SHT. OF
	WJC	RSW			___ OF ___
	DATE	DATE	DATE	DATE	
	3-8-94	10/11/94			

LIGO TUBE - 1" DIFF'L SETTLMT GUIDE SUP
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Node No	Boundary Conditions					
	X-Coord (ft)	Y-Coord (ft)	X-dof (in,K/in)	Y-dof (in,K/in)	Rotation (r,K-ft/r)	Temp. (F)
1	0.00	0.00	R	R		0.00
2	15.46	0.00				0.00
3	30.92	0.00				0.00
4	46.37	0.00				0.00
5	61.83	0.00		R		0.00
6	62.92	0.00				0.00
7	63.50	0.00				0.00
8	66.17	0.00				0.00
9	66.75	0.00				0.00
10	67.83	0.00		R		0.00
11	83.37	0.00				0.00
12	98.92	0.00				0.00
13	114.46	0.00				0.00
14	130.00	0.00	R	R		0.00
15	145.46	0.00				0.00
16	160.92	0.00				0.00
17	176.37	0.00				0.00
18	191.83	0.00		R		0.00
19	192.92	0.00				0.00
20	193.50	0.00				0.00
21	196.17	0.00				0.00
22	196.75	0.00				0.00
23	197.83	0.00		R		0.00
24	213.37	0.00				0.00
25	228.92	0.00				0.00
26	244.46	0.00				0.00
27	260.00	0.00	R	R		0.00
28	275.46	0.00				0.00
29	290.92	0.00				0.00
30	306.37	0.00				0.00
31	321.83	0.00		D -1.00000		0.00
32	322.92	0.00				0.00
33	323.50	0.00				0.00
34	326.17	0.00				0.00
35	326.75	0.00				0.00
36	327.83	0.00		D -1.00000		0.00
37	343.37	0.00				0.00
38	358.92	0.00				0.00
39	374.46	0.00				0.00
40	390.00	0.00	R	R		0.00
41	405.46	0.00				0.00
42	420.92	0.00				0.00
43	436.37	0.00				0.00
44	451.83	0.00		R		0.00
45	452.92	0.00				0.00

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LIGO TUBE - 1" DIFF'L SETTLMT GUIDE SUP
780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Node No	X-Coord (ft)	Y-Coord (ft)	Boundary Conditions			Temp. (F)
			X-dof (in,K/in)	Y-dof (in,K/in)	Rotation (r,K-ft/r)	
46	453.50	0.00				0.00
47	456.17	0.00				0.00
48	456.75	0.00				0.00
49	457.83	0.00			R	0.00
50	473.37	0.00				0.00
51	488.92	0.00				0.00
52	504.46	0.00				0.00
53	520.00	0.00	R	R		0.00
54	535.46	0.00				0.00
55	550.92	0.00				0.00
56	566.37	0.00				0.00
57	581.83	0.00			R	0.00
58	582.92	0.00				0.00
59	583.50	0.00				0.00
60	586.17	0.00				0.00
61	586.75	0.00				0.00
62	587.83	0.00			R	0.00
63	603.37	0.00				0.00
64	618.92	0.00				0.00
65	634.46	0.00				0.00
66	650.00	0.00	R	R		0.00
67	665.46	0.00				0.00
68	680.92	0.00				0.00
69	696.37	0.00				0.00
70	711.83	0.00			R	0.00
71	712.92	0.00				0.00
72	713.50	0.00				0.00
73	716.17	0.00				0.00
74	716.75	0.00				0.00
75	717.83	0.00			R	0.00
76	733.37	0.00				0.00
77	748.92	0.00				0.00
78	764.46	0.00				0.00
79	780.00	0.00	R	R		0.00

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LIGO TUBE - 1" DIFF'L SETTLMT GUIDE SUP
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Material Label	Elastic Modulus (Ksi)	Poisson's Ratio	Thermal Coefficient (F)	Weight Density (K/ft3)	Yield Stress (Fy) (Ksi)
1	27000.00	0.30000	0.65000	0.000	19.200

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LIGO TUBE - 1" DIFF'L SETTLMT GUIDE SUP
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Section Label	Database Shape	Matl. Set	Area (in ²)	Moment of Inertia (in ⁴)	As Coef	y/y
BEAMTUBE		1	19.19	5731.030	1.20	
STUBEND		1	16.13	4820.000	1.20	
EXPJOINT		1	0.01	1.803	1.20	

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LIGO TUBE - 1" DIFF'L SETTLMNT GUIDE SUP
780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

I		J		Section	I Releases J			Sec	Sway	End Offsets		Length
No	Node	Node	x		y	z	x			y	z	
1	1	-	2	BEAMTUBE								15.46
2	2	-	3	BEAMTUBE								15.46
3	3	-	4	BEAMTUBE								15.45
4	4	-	5	BEAMTUBE								15.46
5	5	-	6	BEAMTUBE								1.09
6	6	-	7	STUBEND								0.58
7	7	-	8	EXPJOINT								2.67
8	8	-	9	STUBEND								0.58
9	9	-	10	BEAMTUBE								1.08
10	10	-	11	BEAMTUBE								15.54
11	11	-	12	BEAMTUBE								15.55
12	12	-	13	BEAMTUBE								15.54
13	13	-	14	BEAMTUBE								15.54
14	14	-	15	BEAMTUBE								15.46
15	15	-	16	BEAMTUBE								15.46
16	16	-	17	BEAMTUBE								15.45
17	17	-	18	BEAMTUBE								15.46
18	18	-	19	BEAMTUBE								1.09
19	19	-	20	STUBEND								0.58
20	20	-	21	EXPJOINT								2.67
21	21	-	22	STUBEND								0.58
22	22	-	23	BEAMTUBE								1.08
23	23	-	24	BEAMTUBE								15.54
24	24	-	25	BEAMTUBE								15.55
25	25	-	26	BEAMTUBE								15.54
26	26	-	27	BEAMTUBE								15.54
27	27	-	28	BEAMTUBE								15.46
28	28	-	29	BEAMTUBE								15.46
29	29	-	30	BEAMTUBE								15.45
30	30	-	31	BEAMTUBE								15.46
31	31	-	32	BEAMTUBE								1.09
32	32	-	33	STUBEND								0.58
33	33	-	34	EXPJOINT								2.67
34	34	-	35	STUBEND								0.58
35	35	-	36	BEAMTUBE								1.08
36	36	-	37	BEAMTUBE								15.54
37	37	-	38	BEAMTUBE								15.55
38	38	-	39	BEAMTUBE								15.54
39	39	-	40	BEAMTUBE								15.54
40	40	-	41	BEAMTUBE								15.46
41	41	-	42	BEAMTUBE								15.46
42	42	-	43	BEAMTUBE								15.45
43	43	-	44	BEAMTUBE								15.46
44	44	-	45	BEAMTUBE								1.09
45	45	-	46	STUBEND								0.58

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LIGO TUBE - 1" DIFF'L SETTLMT GUIDE SUP
780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

I No	Node	J Node	Section	I Releases			J			End Offsets		Length (ft)
				x	y	z	x	y	z	Sec	Sway	
46	46	-	47	EXPJOINT								2.67
47	47	-	48	STUBEND								0.58
48	48	-	49	BEAMTUBE								1.08
49	49	-	50	BEAMTUBE								15.54
50	50	-	51	BEAMTUBE								15.55
51	51	-	52	BEAMTUBE								15.54
52	52	-	53	BEAMTUBE								15.54
53	53	-	54	BEAMTUBE								15.46
54	54	-	55	BEAMTUBE								15.46
55	55	-	56	BEAMTUBE								15.45
56	56	-	57	BEAMTUBE								15.46
57	57	-	58	BEAMTUBE								1.09
58	58	-	59	STUBEND								0.58
59	59	-	60	EXPJOINT								2.67
60	60	-	61	STUBEND								0.58
61	61	-	62	BEAMTUBE								1.08
62	62	-	63	BEAMTUBE								15.54
63	63	-	64	BEAMTUBE								15.55
64	64	-	65	BEAMTUBE								15.54
65	65	-	66	BEAMTUBE								15.54
66	66	-	67	BEAMTUBE								15.46
67	67	-	68	BEAMTUBE								15.46
68	68	-	69	BEAMTUBE								15.45
69	69	-	70	BEAMTUBE								15.46
70	70	-	71	BEAMTUBE								1.09
71	71	-	72	STUBEND								0.58
72	72	-	73	EXPJOINT								2.67
73	73	-	74	STUBEND								0.58
74	74	-	75	BEAMTUBE								1.08
75	75	-	76	BEAMTUBE								15.54
76	76	-	77	BEAMTUBE								15.55
77	77	-	78	BEAMTUBE								15.54
78	78	-	79	BEAMTUBE								15.54

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LIGO TUBE - 1" DIFF'L SETTLMT GUIDE SUP
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

I No	Node	J Node	Unbraced Lengths			K Factors		Bending Coefs	
			Lb-in (ft)	Lb-out (ft)	Lc (ft)	In	Out	Cm	Cb
1	1	-	2						
2	2	-	3						
3	3	-	4						
4	4	-	5						
5	5	-	6						
6	6	-	7						
7	7	-	8						
8	8	-	9						
9	9	-	10						
10	10	-	11						
11	11	-	12						
12	12	-	13						
13	13	-	14						
14	14	-	15						
15	15	-	16						
16	16	-	17						
17	17	-	18						
18	18	-	19						
19	19	-	20						
20	20	-	21						
21	21	-	22						
22	22	-	23						
23	23	-	24						
24	24	-	25						
25	25	-	26						
26	26	-	27						
27	27	-	28						
28	28	-	29						
29	29	-	30						
30	30	-	31						
31	31	-	32						
32	32	-	33						
33	33	-	34						
34	34	-	35						
35	35	-	36						
36	36	-	37						
37	37	-	38						
38	38	-	39						
39	39	-	40						
40	40	-	41						
41	41	-	42						
42	42	-	43						
43	43	-	44						
44	44	-	45						
45	45	-	46						

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LIGO TUBE - 1" DIFF'L SETTLMT GUIDE SUP
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

I No	Node	J Node	Unbraced Lengths			K Factors		Bending Coefs	
			Lb-in (ft)	Lb-out (ft)	Lc (ft)	In	Out	Cm	Cb
46	46	-	47						
47	47	-	48						
48	48	-	49						
49	49	-	50						
50	50	-	51						
51	51	-	52						
52	52	-	53						
53	53	-	54						
54	54	-	55						
55	55	-	56						
56	56	-	57						
57	57	-	58						
58	58	-	59						
59	59	-	60						
60	60	-	61						
61	61	-	62						
62	62	-	63						
63	63	-	64						
64	64	-	65						
65	65	-	66						
66	66	-	67						
67	67	-	68						
68	68	-	69						
69	69	-	70						
70	70	-	71						
71	71	-	72						
72	72	-	73						
73	73	-	74						
74	74	-	75						
75	75	-	76						
76	76	-	77						
77	77	-	78						
78	78	-	79						

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LIGO TUBE - 1" DIFF'L SETTLMT GUIDE SUP
780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

BLC No.	Basic Load Case Description	Load Totals		
		Nodal	Point	Dist.
1	1" SETTLEMENT AT GUIDED SUPPORT			78

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LIGO TUBE - 1" DIFF'L SETTLMT GUIDE SUP
780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Member Distributed Loads, BLC 1: 1" SETTLEMENT AT GUIDED SUPPORT

Membr No	I Node	J Node	Dir	Start Magnitude (K/ft, F)	End Magnitude (K/ft, F)	Start Location (ft)	End Location (ft)
1	1	2	Y	-0.001	-0.001	0.000	15.460
2	2	3	Y	-0.001	-0.001	0.000	15.460
3	3	4	Y	-0.001	-0.001	0.000	15.449
4	4	5	Y	-0.001	-0.001	0.000	15.460
5	5	6	Y	-0.001	-0.001	0.000	1.089
6	6	7	Y	-0.001	-0.001	0.000	0.580
7	7	8	Y	-0.001	-0.001	0.000	2.669
8	8	9	Y	-0.001	-0.001	0.000	0.580
9	9	10	Y	-0.001	-0.001	0.000	1.080
10	10	11	Y	-0.001	-0.001	0.000	15.540
11	11	12	Y	-0.001	-0.001	0.000	15.550
12	12	13	Y	-0.001	-0.001	0.000	15.539
13	13	14	Y	-0.001	-0.001	0.000	15.540
14	14	15	Y	-0.001	-0.001	0.000	15.460
15	15	16	Y	-0.001	-0.001	0.000	15.459
16	16	17	Y	-0.001	-0.001	0.000	15.449
17	17	18	Y	-0.001	-0.001	0.000	15.460
18	18	19	Y	-0.001	-0.001	0.000	1.089
19	19	20	Y	-0.001	-0.001	0.000	0.580
20	20	21	Y	-0.001	-0.001	0.000	2.669
21	21	22	Y	-0.001	-0.001	0.000	0.580
22	22	23	Y	-0.001	-0.001	0.000	1.080
23	23	24	Y	-0.001	-0.001	0.000	15.539
24	24	25	Y	-0.001	-0.001	0.000	15.550
25	25	26	Y	-0.001	-0.001	0.000	15.540
26	26	27	Y	-0.001	-0.001	0.000	15.539
27	27	28	Y	-0.001	-0.001	0.000	15.459
28	28	29	Y	-0.001	-0.001	0.000	15.460
29	29	30	Y	-0.001	-0.001	0.000	15.449
30	30	31	Y	-0.001	-0.001	0.000	15.459
31	31	32	Y	-0.001	-0.001	0.000	1.090
32	32	33	Y	-0.001	-0.001	0.000	0.579
33	33	34	Y	-0.001	-0.001	0.000	2.670
34	34	35	Y	-0.001	-0.001	0.000	0.579
35	35	36	Y	-0.001	-0.001	0.000	1.079
36	36	37	Y	-0.001	-0.001	0.000	15.540
37	37	38	Y	-0.001	-0.001	0.000	15.550
38	38	39	Y	-0.001	-0.001	0.000	15.539
39	39	40	Y	-0.001	-0.001	0.000	15.540
40	40	41	Y	-0.001	-0.001	0.000	15.459
41	41	42	Y	-0.001	-0.001	0.000	15.460
42	42	43	Y	-0.001	-0.001	0.000	15.449
43	43	44	Y	-0.001	-0.001	0.000	15.459
44	44	45	Y	-0.001	-0.001	0.000	1.090

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LIGO TUBE - 1" DIFF'L SETTLMNT GUIDE SUP
780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Membr No	I Node	J Node	Dir	Start Magnitude (K/ft,F)	End Magnitude (K/ft,F)	Start Location (ft)	End Location (ft)
45	45	46	Y	-0.001	-0.001	0.000	0.579
46	46	47	Y	-0.001	-0.001	0.000	2.670
47	47	48	Y	-0.001	-0.001	0.000	0.579
48	48	49	Y	-0.001	-0.001	0.000	1.079
49	49	50	Y	-0.001	-0.001	0.000	15.540
50	50	51	Y	-0.001	-0.001	0.000	15.550
51	51	52	Y	-0.001	-0.001	0.000	15.539
52	52	53	Y	-0.001	-0.001	0.000	15.540
53	53	54	Y	-0.001	-0.001	0.000	15.460
54	54	55	Y	-0.001	-0.001	0.000	15.459
55	55	56	Y	-0.001	-0.001	0.000	15.450
56	56	57	Y	-0.001	-0.001	0.000	15.460
57	57	58	Y	-0.001	-0.001	0.000	1.089
58	58	59	Y	-0.001	-0.001	0.000	0.580
59	59	60	Y	-0.001	-0.001	0.000	2.669
60	60	61	Y	-0.001	-0.001	0.000	0.580
61	61	62	Y	-0.001	-0.001	0.000	1.080
62	62	63	Y	-0.001	-0.001	0.000	15.539
63	63	64	Y	-0.001	-0.001	0.000	15.549
64	64	65	Y	-0.001	-0.001	0.000	15.540
65	65	66	Y	-0.001	-0.001	0.000	15.539
66	66	67	Y	-0.001	-0.001	0.000	15.460
67	67	68	Y	-0.001	-0.001	0.000	15.459
68	68	69	Y	-0.001	-0.001	0.000	15.449
69	69	70	Y	-0.001	-0.001	0.000	15.460
70	70	71	Y	-0.001	-0.001	0.000	1.089
71	71	72	Y	-0.001	-0.001	0.000	0.580
72	72	73	Y	-0.001	-0.001	0.000	2.669
73	73	74	Y	-0.001	-0.001	0.000	0.580
74	74	75	Y	-0.001	-0.001	0.000	1.080
75	75	76	Y	-0.001	-0.001	0.000	15.539
76	76	77	Y	-0.001	-0.001	0.000	15.550
77	77	78	Y	-0.001	-0.001	0.000	15.540
78	78	79	Y	-0.001	-0.001	0.000	15.539

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LIGO TUBE - 1" DIFF'L SETTLMT GUIDE SUP
780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Load Combination No.	Description	Self Wt Dir	Wt Fac	BLC Fac	BLC Fac	BLC Fac	BLC Fac	BLC Fac	DYNA	W S	E V
1	1" SUPPORT DISPL			1	1						

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LIGO TUBE - 1" DIFF'L SETTLMT GUIDE SUP
780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Dynamic Analysis Data

Number of modes (frequencies) : 15
Basic Load Case for masses : 1
BLC mass direction of action : Y only
Acceleration of Gravity : 32.20 ft/sec**2

Load Combination is 1 : 1" SUPPORT DISPL
Nodal Displacements

Node	Global X (in)	Global Y (in)	Rotation (rad)
1	0.00000	-0.00000	-0.00001
2	0.00000	-0.00149	-0.00001
3	0.00000	-0.00209	0.00000
4	0.00000	-0.00149	0.00001
5	0.00000	-0.00000	0.00001
6	0.00000	0.00012	0.00001
7	0.00000	0.00018	0.00001
8	0.00000	0.00012	-0.00001
9	0.00000	0.00008	-0.00001
10	0.00000	-0.00000	-0.00001
11	0.00000	-0.00098	-0.00000
12	0.00000	-0.00126	0.00000
13	0.00000	-0.00075	0.00000
14	0.00000	-0.00000	0.00000
15	0.00000	0.00023	0.00000
16	0.00000	0.00030	0.00000
17	0.00000	0.00038	-0.00000
18	0.00000	-0.00000	-0.00001
19	0.00000	-0.00007	-0.00001
20	0.00000	-0.00011	-0.00001
21	0.00000	-0.00643	0.00032
22	0.00000	-0.00419	0.00032
23	0.00000	0.00000	0.00032
24	0.00000	0.05727	0.00027
25	0.00000	0.09265	0.00009
26	0.00000	0.08184	-0.00023
27	0.00000	-0.00000	-0.00067
28	0.00000	-0.16872	-0.00112
29	0.00000	-0.40789	-0.00143
30	0.00000	-0.69281	-0.00162
31	0.00000	-1.00000	-0.00167
32	0.00000	-1.02188	-0.00167
33	0.00000	-1.03353	-0.00167
34	0.00000	-1.03319	0.00167
35	0.00000	-1.02160	0.00167
36	0.00000	-1.00000	0.00167

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LIGO TUBE - 1" DIFF'L SETTLMNT GUIDE SUP
780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Node	Global X (in)	Global Y (in)	Rotation (rad)
37	0.00000	-0.69253	0.00161
38	0.00000	-0.40715	0.00143
39	0.00000	-0.16818	0.00111
40	0.00000	-0.00000	0.00067
41	0.00000	0.08051	0.00022
42	0.00000	0.09111	-0.00009
43	0.00000	0.05634	-0.00027
44	0.00000	0.00000	-0.00032
45	0.00000	-0.00418	-0.00032
46	0.00000	-0.00640	-0.00032
47	0.00000	-0.00010	0.00001
48	0.00000	-0.00007	0.00001
49	0.00000	-0.00000	0.00000
50	0.00000	0.00035	-0.00000
51	0.00000	0.00026	-0.00000
52	0.00000	0.00020	-0.00000
53	0.00000	-0.00000	-0.00000
54	0.00000	-0.00071	-0.00000
55	0.00000	-0.00121	-0.00000
56	0.00000	-0.00093	0.00000
57	0.00000	-0.00000	0.00001
58	0.00000	0.00008	0.00001
59	0.00000	0.00012	0.00001
60	0.00000	0.00009	-0.00000
61	0.00000	0.00006	-0.00000
62	0.00000	-0.00000	-0.00000
63	0.00000	-0.00073	-0.00000
64	0.00000	-0.00087	0.00000
65	0.00000	-0.00041	0.00000
66	0.00000	-0.00000	0.00000
67	0.00000	-0.00040	-0.00000
68	0.00000	-0.00085	-0.00000
69	0.00000	-0.00072	0.00000
70	0.00000	-0.00000	0.00000
71	0.00000	0.00006	0.00000
72	0.00000	0.00009	0.00000
73	0.00000	0.00018	-0.00001
74	0.00000	0.00012	-0.00001
75	0.00000	-0.00000	-0.00001
76	0.00000	-0.00151	-0.00001
77	0.00000	-0.00213	-0.00000
78	0.00000	-0.00152	0.00001
79	0.00000	-0.00000	0.00001

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LIGO TUBE - 1" DIFF'L SETTLMT GUIDE SUP
780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Load Combination is 1 : 1" SUPPORT DISPL
Spring Reactions

Node	Global X (K)	Global Y (K)	Moment (K-ft)
1	0.00000	0.03073	0.00000
5	0.00000	0.03597	0.00000
10	0.00000	0.02679	0.00000
14	0.00000	0.06366	0.00000
18	0.00000	0.24021	0.00000
23	0.00000	-0.75450	0.00000
27	0.00000	1.23407	0.00000
31	0.00000	-0.55994	0.00000
36	0.00000	-0.53684	0.00000
40	0.00000	1.22669	0.00000
44	0.00000	-0.75307	0.00000
49	0.00000	0.23882	0.00000
53	0.00000	0.06365	0.00000
57	0.00000	0.02935	0.00000
62	0.00000	0.02565	0.00000
66	0.00000	0.07748	0.00000
70	0.00000	0.02330	0.00000
75	0.00000	0.03712	0.00000
79	0.00000	0.03085	0.00000
Totals	0.00000	0.78000	0.00000

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LIGO TUBE - 1" DIFF'L SETTLMT GUIDE SUP
780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Load Combination is 1 : 1" SUPPORT DISPL
Member End Forces

No	Nodes		I-End			J-End		
	I	J	Axial (K)	Shear (K)	Moment (K-ft)	Axial (K)	Shear (K)	Moment (K-ft)
1	1-	2	0.00	0.03	-0.00	0.00	-0.02	0.36
2	2-	3	0.00	0.02	-0.36	0.00	0.00	0.47
3	3-	4	0.00	-0.00	-0.47	0.00	0.02	0.35
4	4-	5	0.00	-0.02	-0.35	0.00	0.03	-0.01
5	5-	6	0.00	0.00	0.01	0.00	-0.00	-0.01
6	6-	7	0.00	0.00	0.01	0.00	-0.00	-0.00
7	7-	8	0.00	0.00	0.00	0.00	-0.00	-0.00
8	8-	9	0.00	0.00	0.00	0.00	0.00	0.00
9	9-	10	0.00	-0.00	-0.00	0.00	0.00	-0.00
10	10-	11	0.00	0.03	0.00	0.00	-0.01	0.28
11	11-	12	0.00	0.01	-0.28	0.00	0.01	0.31
12	12-	13	0.00	-0.01	-0.31	0.00	0.02	0.11
13	13-	14	0.00	-0.02	-0.11	0.00	0.04	-0.34
14	14-	15	0.00	0.03	0.34	0.00	-0.01	-0.04
15	15-	16	0.00	0.01	0.04	0.00	0.00	0.02
16	16-	17	0.00	-0.00	-0.02	0.00	0.02	-0.15
17	17-	18	0.00	-0.02	0.15	0.00	0.03	-0.57
18	18-	19	0.00	0.21	0.57	0.00	-0.20	-0.35
19	19-	20	0.00	0.20	0.35	0.00	-0.20	-0.23
20	20-	21	0.00	0.20	0.23	0.00	-0.20	0.31
21	21-	22	0.00	0.20	-0.31	0.00	-0.20	0.43
22	22-	23	0.00	0.20	-0.43	0.00	-0.20	0.64
23	23-	24	0.00	-0.55	-0.64	0.00	0.57	-8.10
24	24-	25	0.00	-0.57	8.10	0.00	0.59	-17.09
25	25-	26	0.00	-0.59	17.09	0.00	0.60	-26.32
26	26-	27	0.00	-0.60	26.32	0.00	0.62	-35.79
27	27-	28	0.00	0.62	35.79	0.00	-0.60	-26.37
28	28-	29	0.00	0.60	26.37	0.00	-0.59	-17.19
29	29-	30	0.00	0.59	17.19	0.00	-0.57	-8.26
30	30-	31	0.00	0.57	8.26	0.00	-0.56	0.44
31	31-	32	0.00	-0.00	-0.44	0.00	0.01	0.44
32	32-	33	0.00	-0.01	-0.44	0.00	0.01	0.43
33	33-	34	0.00	-0.01	-0.43	0.00	0.01	0.41
34	34-	35	0.00	-0.01	-0.41	0.00	0.01	0.41
35	35-	36	0.00	-0.01	-0.41	0.00	0.01	0.40
36	36-	37	0.00	-0.55	-0.40	0.00	0.56	-8.24
37	37-	38	0.00	-0.56	8.24	0.00	0.58	-17.12
38	38-	39	0.00	-0.58	17.12	0.00	0.59	-26.23
39	39-	40	0.00	-0.59	26.23	0.00	0.61	-35.59
40	40-	41	0.00	0.62	35.59	0.00	-0.60	-26.17
41	41-	42	0.00	0.60	26.17	0.00	-0.59	-16.99
42	42-	43	0.00	0.59	16.99	0.00	-0.57	-8.06
43	43-	44	0.00	0.57	8.06	0.00	-0.56	0.64

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LIGO TUBE - 1" DIFF'L SETTLMNT GUIDE SUP
780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

No	Nodes		I-End			J-End		
	I	J	Axial (K)	Shear (K)	Moment (K-ft)	Axial (K)	Shear (K)	Moment (K-ft)
44	44-	45	0.00	-0.20	-0.64	0.00	0.20	0.42
45	45-	46	0.00	-0.20	-0.42	0.00	0.20	0.31
46	46-	47	0.00	-0.20	-0.31	0.00	0.20	-0.23
47	47-	48	0.00	-0.20	0.23	0.00	0.20	-0.35
48	48-	49	0.00	-0.20	0.35	0.00	0.20	-0.57
49	49-	50	0.00	0.03	0.57	0.00	-0.02	-0.15
50	50-	51	0.00	0.02	0.15	0.00	-0.00	0.03
51	51-	52	0.00	0.00	-0.03	0.00	0.01	-0.03
52	52-	53	0.00	-0.01	0.03	0.00	0.03	-0.34
53	53-	54	0.00	0.04	0.34	0.00	-0.02	0.10
54	54-	55	0.00	0.02	-0.10	0.00	-0.01	0.31
55	55-	56	0.00	0.01	-0.31	0.00	0.01	0.27
56	56-	57	0.00	-0.01	-0.27	0.00	0.03	-0.01
57	57-	58	0.00	0.00	0.01	0.00	-0.00	-0.00
58	58-	59	0.00	0.00	0.00	0.00	-0.00	-0.00
59	59-	60	0.00	0.00	0.00	0.00	0.00	-0.00
60	60-	61	0.00	-0.00	0.00	0.00	0.00	-0.00
61	61-	62	0.00	-0.00	0.00	0.00	0.00	-0.00
62	62-	63	0.00	0.02	0.00	0.00	-0.01	0.24
63	63-	64	0.00	0.01	-0.24	0.00	0.01	0.24
64	64-	65	0.00	-0.01	-0.24	0.00	0.02	0.00
65	65-	66	0.00	-0.02	-0.00	0.00	0.04	-0.48
66	66-	67	0.00	0.04	0.48	0.00	-0.02	-0.00
67	67-	68	0.00	0.02	0.00	0.00	-0.01	0.24
68	68-	69	0.00	0.01	-0.24	0.00	0.01	0.24
69	69-	70	0.00	-0.01	-0.24	0.00	0.02	0.00
70	70-	71	0.00	0.00	-0.00	0.00	0.00	0.00
71	71-	72	0.00	-0.00	-0.00	0.00	0.00	0.00
72	72-	73	0.00	-0.00	-0.00	0.00	0.00	-0.01
73	73-	74	0.00	-0.00	0.01	0.00	0.00	-0.01
74	74-	75	0.00	-0.00	0.01	0.00	0.01	-0.01
75	75-	76	0.00	0.03	0.01	0.00	-0.02	0.35
76	76-	77	0.00	0.02	-0.35	0.00	-0.00	0.48
77	77-	78	0.00	0.00	-0.48	0.00	0.02	0.36
78	78-	79	0.00	-0.02	-0.36	0.00	0.03	-0.00

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LIGO TUBE - 1" DIFF'L SETTLMNT GUIDE SUP
 780' LENGTH, SIMPLE SUPP'TS @ BOTH ENDS

Load Combination is 1 : 1" SUPPORT DISPL
 AISC Code Checks

No	Nodes		Maximum	0	Member Quarter Points			L	Shear
	I	J			1/4	1/2	3/4		
1	1-	2			-	Not Calculated	-		
2	2-	3			-	Not Calculated	-		
3	3-	4			-	Not Calculated	-		
4	4-	5			-	Not Calculated	-		
5	5-	6			-	Not Calculated	-		
6	6-	7			-	Not Calculated	-		
7	7-	8			-	Not Calculated	-		
8	8-	9			-	Not Calculated	-		
9	9-	10			-	Not Calculated	-		
10	10-	11			-	Not Calculated	-		
11	11-	12			-	Not Calculated	-		
12	12-	13			-	Not Calculated	-		
13	13-	14			-	Not Calculated	-		
14	14-	15			-	Not Calculated	-		
15	15-	16			-	Not Calculated	-		
16	16-	17			-	Not Calculated	-		
17	17-	18			-	Not Calculated	-		
18	18-	19			-	Not Calculated	-		
19	19-	20			-	Not Calculated	-		
20	20-	21			-	Not Calculated	-		
21	21-	22			-	Not Calculated	-		
22	22-	23			-	Not Calculated	-		
23	23-	24			-	Not Calculated	-		
24	24-	25			-	Not Calculated	-		
25	25-	26			-	Not Calculated	-		
26	26-	27			-	Not Calculated	-		
27	27-	28			-	Not Calculated	-		
28	28-	29			-	Not Calculated	-		
29	29-	30			-	Not Calculated	-		
30	30-	31			-	Not Calculated	-		
31	31-	32			-	Not Calculated	-		
32	32-	33			-	Not Calculated	-		
33	33-	34			-	Not Calculated	-		
34	34-	35			-	Not Calculated	-		
35	35-	36			-	Not Calculated	-		
36	36-	37			-	Not Calculated	-		
37	37-	38			-	Not Calculated	-		
38	38-	39			-	Not Calculated	-		
39	39-	40			-	Not Calculated	-		
40	40-	41			-	Not Calculated	-		
41	41-	42			-	Not Calculated	-		
42	42-	43			-	Not Calculated	-		
43	43-	44			-	Not Calculated	-		

**RISA-2D MODEL OF LIGO BEAM TUBE MODULE @ HANFORD SITE: CASE #2 NODE NUMBERS
CASE #2: SIMPLE SUPPORTS AT ONE END, RIGID SUPPORT AT OTHER, TOTAL LENGTH = 858.8'**

NODE	X(FT)	DESCRIPTION
1	0.000	FIXED SUPPT, ROTATION FREE
2	15.458	
3	30.917	
4	46.375	
5	61.833	GUIDED SUPPT
6	63.500	END TUBE / START EJ STUB
7	64.083	END EJ STUB / START BELLOWS
8	65.583	END BELLOWS / START EJ STUB
9	66.167	END EJ STUB / START TUBE
10	67.833	GUIDED SUPPT
11	83.375	
12	98.917	
13	114.458	
14	130.000	FIXED SUPPT, ROTATION FREE
15	145.458	
16	160.917	
17	176.375	
18	191.833	GUIDED SUPPT
19	193.500	END TUBE / START EJ STUB
20	194.083	END EJ STUB / START BELLOWS
21	195.583	END BELLOWS / START EJ STUB
22	196.167	END EJ STUB / START TUBE
23	197.833	GUIDED SUPPT
24	213.375	
25	228.917	
26	244.458	
27	260.000	FIXED SUPPT, ROTATION FREE
28	275.458	
29	290.917	
30	306.375	
31	321.833	GUIDED SUPPT
32	323.500	END TUBE / START EJ STUB
33	324.083	END EJ STUB / START BELLOWS
34	325.583	END BELLOWS / START EJ STUB
35	326.167	END EJ STUB / START TUBE
36	327.833	GUIDED SUPPT
37	343.375	
38	358.917	
39	374.458	
40	390.000	FIXED SUPPT, ROTATION FREE
41	405.458	
42	420.917	
43	436.375	
44	451.833	GUIDED SUPPT
45	453.500	END TUBE / START EJ STUB
46	454.083	END EJ STUB / START BELLOWS
47	455.583	END BELLOWS / START EJ STUB
48	456.167	END EJ STUB / START TUBE
49	457.833	GUIDED SUPPT

SUBJECT RISA-2D MODEL OF BEAM TUBE MODULE, CASE #2 NODE NUMBERS, COORDINATES & DESCRIPTION	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RW	MADE BY	CHKD BY	SHT 1 OF 2
	DATE 2/28/94	DATE 10/11/94	DATE	DATE	5.7°

**RISA-2D MODEL OF LIGO BEAM TUBE MODULE @ HANFORD SITE: CASE #2 NODE NUMBERS
CASE #2: SIMPLE SUPPORTS AT ONE END, RIGID SUPPORT AT OTHER, TOTAL LENGTH = 858.8'**

NODE	X(FT)	DESCRIPTION
50	473.375	
51	488.917	
52	504.458	
53	520.000	FIXED SUPP'T, ROTATION FREE
54	535.458	
55	550.917	
56	566.375	
57	581.833	GUIDED SUPP'T
58	583.500	END TUBE / START EJ STUB
59	584.083	END EJ STUB / START BELLOWS
60	585.583	END BELLOWS / START EJ STUB
61	586.167	END EJ STUB / START TUBE
62	587.833	GUIDED SUPP'T
63	603.375	
64	618.917	
65	634.458	
66	650.000	FIXED SUPP'T, ROTATION FREE
67	665.458	
68	680.917	
69	696.375	
70	711.833	GUIDED SUPP'T
71	713.500	END TUBE / START EJ STUB
72	714.083	END EJ STUB / START BELLOWS
73	715.583	END BELLOWS / START EJ STUB
74	716.167	END EJ STUB / START TUBE
75	717.833	GUIDED SUPP'T
76	733.375	
77	748.917	
78	764.458	
79	780.000	FIXED SUPP'T, ROTATION FREE
80	795.458	
81	810.917	
82	826.375	
83	841.833	GUIDED SUPP'T
84	843.500	END TUBE / START EJ STUB
85	844.083	END EJ STUB / START BELLOWS
86	845.583	END BELLOWS / START EJ STUB
87	846.167	END EJ STUB / START TUBE
88	847.833	GUIDED SUPP'T
89	850.581	
90	853.328	
91	856.075	
92	858.823	RIGID SUPP'T, ROTATION FIXED

SUBJECT RISA-2D MODEL OF BEAM TUBE MODULE, CASE #2 NODE NUMBERS, COORDINATES & DESCRIPTION	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RJW	MADE BY	CHKD BY	SHT 2 OF 2
	DATE 2/28/94	DATE 10/28/94	DATE	DATE	5.71

3/09/94

LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

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Units Option : US Standard

AISC Code Checks : 9th Edition ASD

Shear Deformation: No

P-Delta Effects : No

Redesign : No

Edge Forces : No

A.S.I.F. : 1.333

SUBJECT	OFFICE		REVISION		REFERENCE NO.
	CBI	NOE C			
	MADE BY	CHKD BY	MADE BY	CHKD BY	SHT ___ OF ___
	DATE	DATE	DATE	DATE	
	3-18-94	18 MAR 94			

LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

Node No	X-Coord (ft)	Y-Coord (ft)	Boundary Conditions			Temp. (F)
			X-dof (in,K/in)	Y-dof (in,K/in)	Rotation (r,K-ft/r)	
1	0.00	0.00	R	R		0.00
2	15.46	0.00				0.00
3	30.92	0.00				0.00
4	46.37	0.00				0.00
5	61.83	0.00		R		0.00
6	62.92	0.00				0.00
7	63.50	0.00				0.00
8	66.17	0.00				0.00
9	66.75	0.00				0.00
10	67.83	0.00		R		0.00
11	83.37	0.00				0.00
12	98.92	0.00				0.00
13	114.46	0.00				0.00
14	130.00	0.00	R	R		0.00
15	145.46	0.00				0.00
16	160.92	0.00				0.00
17	176.37	0.00				0.00
18	191.83	0.00		R		0.00
19	192.92	0.00				0.00
20	193.50	0.00				0.00
21	196.17	0.00				0.00
22	196.75	0.00				0.00
23	197.83	0.00		R		0.00
24	213.37	0.00				0.00
25	228.92	0.00				0.00
26	244.46	0.00				0.00
27	260.00	0.00	R	R		0.00
28	275.46	0.00				0.00
29	290.92	0.00				0.00
30	306.37	0.00				0.00
31	321.83	0.00		R		0.00
32	322.92	0.00				0.00
33	323.50	0.00				0.00
34	326.17	0.00				0.00
35	326.75	0.00				0.00
36	327.83	0.00		R		0.00
37	343.37	0.00				0.00
38	358.92	0.00				0.00
39	374.46	0.00				0.00
40	390.00	0.00	R	R		0.00
41	405.46	0.00				0.00
42	420.92	0.00				0.00
43	436.37	0.00				0.00
44	451.83	0.00		R		0.00
45	452.92	0.00				0.00

3/09/94

LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

Node No	Boundary Conditions					
	X-Coord (ft)	Y-Coord (ft)	X-dof (in,K/in)	Y-dof (in,K/in)	Rotation (r,K-ft/r)	Temp. (F)
46	453.50	0.00				0.00
47	456.17	0.00				0.00
48	456.75	0.00				0.00
49	457.83	0.00			R	0.00
50	473.37	0.00				0.00
51	488.92	0.00				0.00
52	504.46	0.00				0.00
53	520.00	0.00	R	R		0.00
54	535.46	0.00				0.00
55	550.92	0.00				0.00
56	566.37	0.00				0.00
57	581.83	0.00			R	0.00
58	582.92	0.00				0.00
59	583.50	0.00				0.00
60	586.17	0.00				0.00
61	586.75	0.00				0.00
62	587.83	0.00			R	0.00
63	603.37	0.00				0.00
64	618.92	0.00				0.00
65	634.46	0.00				0.00
66	650.00	0.00	R	R		0.00
67	665.46	0.00				0.00
68	680.92	0.00				0.00
69	696.37	0.00				0.00
70	711.83	0.00			R	0.00
71	712.92	0.00				0.00
72	713.50	0.00				0.00
73	716.17	0.00				0.00
74	716.75	0.00				0.00
75	717.83	0.00			R	0.00
76	733.37	0.00				0.00
77	748.92	0.00				0.00
78	764.46	0.00				0.00
79	780.00	0.00	R	R		0.00
80	795.46	0.00				0.00
81	810.92	0.00				0.00
82	826.37	0.00				0.00
83	841.83	0.00			R	0.00
84	843.50	0.00				0.00
85	844.08	0.00				0.00
86	845.58	0.00				0.00
87	846.17	0.00				0.00
88	847.83	0.00			R	0.00
89	850.58	0.00				0.00
90	853.33	0.00				0.00

3/09/94

LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

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Node		Boundary Conditions					Temp.
No	X-Coord	Y-Coord	X-dof	Y-dof	Rotation		
	(ft)	(ft)	(in,K/in)	(in,K/in)	(r,K-ft/r)	(F)	
91	856.08	0.00				0.00	
92	858.82	0.00	R	R	R	0.00	

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LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

Material Label	Elastic Modulus (Ksi)	Poisson's Ratio	Thermal Coefficient (F)	Weight Density (K/ft3)	Yield Stress (Fy) (Ksi)
1	27000.00	0.30000	0.65000	0.000	19.200

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LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

Section Label	Database Shape	Matl. Set	Area (in ²)	Moment of Inertia (in ⁴)	As Coef	y/y
BEAMTUBE		1	19.19	5731.030	1.20	
STUBEND		1	16.13	4820.000	1.20	
EXPJOINT		1	0.01	1.803	1.20	

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LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

I No	I Node	J Node	Section	I Releases			J			End Offsets		Length (ft)
				x	y	z	x	y	z	Sec	Sway	
1	1	2	BEAMTUBE									15.46
2	2	3	BEAMTUBE									15.46
3	3	4	BEAMTUBE									15.45
4	4	5	BEAMTUBE									15.46
5	5	6	BEAMTUBE									1.09
6	6	7	STUBEND									0.58
7	7	8	EXPJOINT									2.67
8	8	9	STUBEND									0.58
9	9	10	BEAMTUBE									1.08
10	10	11	BEAMTUBE									15.54
11	11	12	BEAMTUBE									15.55
12	12	13	BEAMTUBE									15.54
13	13	14	BEAMTUBE									15.54
14	14	15	BEAMTUBE									15.46
15	15	16	BEAMTUBE									15.46
16	16	17	BEAMTUBE									15.45
17	17	18	BEAMTUBE									15.46
18	18	19	BEAMTUBE									1.09
19	19	20	STUBEND									0.58
20	20	21	EXPJOINT									2.67
21	21	22	STUBEND									0.58
22	22	23	BEAMTUBE									1.08
23	23	24	BEAMTUBE									15.54
24	24	25	BEAMTUBE									15.55
25	25	26	BEAMTUBE									15.54
26	26	27	BEAMTUBE									15.54
27	27	28	BEAMTUBE									15.46
28	28	29	BEAMTUBE									15.46
29	29	30	BEAMTUBE									15.45
30	30	31	BEAMTUBE									15.46
31	31	32	BEAMTUBE									1.09
32	32	33	STUBEND									0.58
33	33	34	EXPJOINT									2.67
34	34	35	STUBEND									0.58
35	35	36	BEAMTUBE									1.08
36	36	37	BEAMTUBE									15.54
37	37	38	BEAMTUBE									15.55
38	38	39	BEAMTUBE									15.54
39	39	40	BEAMTUBE									15.54
40	40	41	BEAMTUBE									15.46
41	41	42	BEAMTUBE									15.46
42	42	43	BEAMTUBE									15.45
43	43	44	BEAMTUBE									15.46
44	44	45	BEAMTUBE									1.09
45	45	46	STUBEND									0.58

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LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

I No	Node	J Node	Section	I Releases			J Sec	Sway	End Offsets		Length (ft)
				x	y	z			I (in)	J (in)	
46	46	- 47	EXPJOINT								2.67
47	47	- 48	STUBEND								0.58
48	48	- 49	BEAMTUBE								1.08
49	49	- 50	BEAMTUBE								15.54
50	50	- 51	BEAMTUBE								15.55
51	51	- 52	BEAMTUBE								15.54
52	52	- 53	BEAMTUBE								15.54
53	53	- 54	BEAMTUBE								15.46
54	54	- 55	BEAMTUBE								15.46
55	55	- 56	BEAMTUBE								15.45
56	56	- 57	BEAMTUBE								15.46
57	57	- 58	BEAMTUBE								1.09
58	58	- 59	STUBEND								0.58
59	59	- 60	EXPJOINT								2.67
60	60	- 61	STUBEND								0.58
61	61	- 62	BEAMTUBE								1.08
62	62	- 63	BEAMTUBE								15.54
63	63	- 64	BEAMTUBE								15.55
64	64	- 65	BEAMTUBE								15.54
65	65	- 66	BEAMTUBE								15.54
66	66	- 67	BEAMTUBE								15.46
67	67	- 68	BEAMTUBE								15.46
68	68	- 69	BEAMTUBE								15.45
69	69	- 70	BEAMTUBE								15.46
70	70	- 71	BEAMTUBE								1.09
71	71	- 72	STUBEND								0.58
72	72	- 73	EXPJOINT								2.67
73	73	- 74	STUBEND								0.58
74	74	- 75	BEAMTUBE								1.08
75	75	- 76	BEAMTUBE								15.54
76	76	- 77	BEAMTUBE								15.55
77	77	- 78	BEAMTUBE								15.54
78	78	- 79	BEAMTUBE								15.54
79	79	- 80	BEAMTUBE								15.46
80	80	- 81	BEAMTUBE								15.46
81	81	- 82	BEAMTUBE								15.45
82	82	- 83	BEAMTUBE								15.46
83	83	- 84	BEAMTUBE								1.67
84	84	- 85	STUBEND								0.58
85	85	- 86	EXPJOINT								1.50
86	86	- 87	STUBEND								0.59
87	87	- 88	BEAMTUBE								1.66
88	88	- 89	BEAMTUBE								2.75
89	89	- 90	BEAMTUBE								2.75
90	90	- 91	BEAMTUBE								2.75

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LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

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I No	J Node	Section	I Releases J						End Offsets		Length		
			x	y	z	x	y	z	Sec	Sway		I (in)	J (in)
91	91 - 92	BEAMTUBE											2.74

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LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

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No	I Node	J Node	Unbraced Lengths			K Factors		Bending Coefs	
			Lb-in (ft)	Lb-out (ft)	Lc (ft)	In	Out	Cm	Cb
1	1	2							
2	2	3							
3	3	4							
4	4	5							
5	5	6							
6	6	7							
7	7	8							
8	8	9							
9	9	10							
10	10	11							
11	11	12							
12	12	13							
13	13	14							
14	14	15							
15	15	16							
16	16	17							
17	17	18							
18	18	19							
19	19	20							
20	20	21							
21	21	22							
22	22	23							
23	23	24							
24	24	25							
25	25	26							
26	26	27							
27	27	28							
28	28	29							
29	29	30							
30	30	31							
31	31	32							
32	32	33							
33	33	34							
34	34	35							
35	35	36							
36	36	37							
37	37	38							
38	38	39							
39	39	40							
40	40	41							
41	41	42							
42	42	43							
43	43	44							
44	44	45							
45	45	46							

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LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

I No	Node	J Node	Unbraced Lengths			K Factors		Bending Coefs	
			Lb-in (ft)	Lb-out (ft)	Lc (ft)	In	Out	Cm	Cb
46	46	-	47						
47	47	-	48						
48	48	-	49						
49	49	-	50						
50	50	-	51						
51	51	-	52						
52	52	-	53						
53	53	-	54						
54	54	-	55						
55	55	-	56						
56	56	-	57						
57	57	-	58						
58	58	-	59						
59	59	-	60						
60	60	-	61						
61	61	-	62						
62	62	-	63						
63	63	-	64						
64	64	-	65						
65	65	-	66						
66	66	-	67						
67	67	-	68						
68	68	-	69						
69	69	-	70						
70	70	-	71						
71	71	-	72						
72	72	-	73						
73	73	-	74						
74	74	-	75						
75	75	-	76						
76	76	-	77						
77	77	-	78						
78	78	-	79						
79	79	-	80						
80	80	-	81						
81	81	-	82						
82	82	-	83						
83	83	-	84						
84	84	-	85						
85	85	-	86						
86	86	-	87						
87	87	-	88						
88	88	-	89						
89	89	-	90						
90	90	-	91						

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LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

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I No	J Node	Unbraced Lengths			K Factors		Bending Coefs	
		Lb-in (ft)	Lb-out (ft)	Lc (ft)	In	Out	Cm	Cb
91	91 - 92							

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LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

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BLC No.	Basic Load Case Description	Load Totals		
		Nodal	Point	Dist.
1	SELF WEIGHT + INSULATION			91

3/09/94

LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

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 Member Distributed Loads,BLC 1: SELF WEIGHT + INSULATION

Memb No	I Node	J Node	Dir	Start Magnitude (K/ft,F)	End Magnitude (K/ft,F)	Start Location (ft)	End Location (ft)
1	1	2	Y	-0.091	-0.091	0.000	15.460
2	2	3	Y	-0.091	-0.091	0.000	15.460
3	3	4	Y	-0.091	-0.091	0.000	15.449
4	4	5	Y	-0.091	-0.091	0.000	15.460
5	5	6	Y	-0.091	-0.091	0.000	1.089
6	6	7	Y	-0.091	-0.091	0.000	0.580
7	7	8	Y	-0.091	-0.091	0.000	2.669
8	8	9	Y	-0.091	-0.091	0.000	0.580
9	9	10	Y	-0.091	-0.091	0.000	1.080
10	10	11	Y	-0.091	-0.091	0.000	15.540
11	11	12	Y	-0.091	-0.091	0.000	15.550
12	12	13	Y	-0.091	-0.091	0.000	15.539
13	13	14	Y	-0.091	-0.091	0.000	15.540
14	14	15	Y	-0.091	-0.091	0.000	15.460
15	15	16	Y	-0.091	-0.091	0.000	15.459
16	16	17	Y	-0.091	-0.091	0.000	15.449
17	17	18	Y	-0.091	-0.091	0.000	15.460
18	18	19	Y	-0.091	-0.091	0.000	1.089
19	19	20	Y	-0.091	-0.091	0.000	0.580
20	20	21	Y	-0.091	-0.091	0.000	2.669
21	21	22	Y	-0.091	-0.091	0.000	0.580
22	22	23	Y	-0.091	-0.091	0.000	1.080
23	23	24	Y	-0.091	-0.091	0.000	15.539
24	24	25	Y	-0.091	-0.091	0.000	15.550
25	25	26	Y	-0.091	-0.091	0.000	15.540
26	26	27	Y	-0.091	-0.091	0.000	15.539
27	27	28	Y	-0.091	-0.091	0.000	15.459
28	28	29	Y	-0.091	-0.091	0.000	15.460
29	29	30	Y	-0.091	-0.091	0.000	15.449
30	30	31	Y	-0.091	-0.091	0.000	15.459
31	31	32	Y	-0.091	-0.091	0.000	1.090
32	32	33	Y	-0.091	-0.091	0.000	0.579
33	33	34	Y	-0.091	-0.091	0.000	2.670
34	34	35	Y	-0.091	-0.091	0.000	0.579
35	35	36	Y	-0.091	-0.091	0.000	1.079
36	36	37	Y	-0.091	-0.091	0.000	15.540
37	37	38	Y	-0.091	-0.091	0.000	15.550
38	38	39	Y	-0.091	-0.091	0.000	15.539
39	39	40	Y	-0.091	-0.091	0.000	15.540
40	40	41	Y	-0.091	-0.091	0.000	15.459
41	41	42	Y	-0.091	-0.091	0.000	15.460
42	42	43	Y	-0.091	-0.091	0.000	15.449
43	43	44	Y	-0.091	-0.091	0.000	15.459
44	44	45	Y	-0.091	-0.091	0.000	1.090

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LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

Memb No	I Node	J Node	Dir	Start Magnitude (K/ft,F)	End Magnitude (K/ft,F)	Start Location (ft)	End Location (ft)
45	45	46	Y	-0.091	-0.091	0.000	0.579
46	46	47	Y	-0.091	-0.091	0.000	2.670
47	47	48	Y	-0.091	-0.091	0.000	0.579
48	48	49	Y	-0.091	-0.091	0.000	1.079
49	49	50	Y	-0.091	-0.091	0.000	15.540
50	50	51	Y	-0.091	-0.091	0.000	15.550
51	51	52	Y	-0.091	-0.091	0.000	15.539
52	52	53	Y	-0.091	-0.091	0.000	15.540
53	53	54	Y	-0.091	-0.091	0.000	15.460
54	54	55	Y	-0.091	-0.091	0.000	15.459
55	55	56	Y	-0.091	-0.091	0.000	15.450
56	56	57	Y	-0.091	-0.091	0.000	15.460
57	57	58	Y	-0.091	-0.091	0.000	1.089
58	58	59	Y	-0.091	-0.091	0.000	0.580
59	59	60	Y	-0.091	-0.091	0.000	2.669
60	60	61	Y	-0.091	-0.091	0.000	0.580
61	61	62	Y	-0.091	-0.091	0.000	1.080
62	62	63	Y	-0.091	-0.091	0.000	15.539
63	63	64	Y	-0.091	-0.091	0.000	15.549
64	64	65	Y	-0.091	-0.091	0.000	15.540
65	65	66	Y	-0.091	-0.091	0.000	15.539
66	66	67	Y	-0.091	-0.091	0.000	15.460
67	67	68	Y	-0.091	-0.091	0.000	15.459
68	68	69	Y	-0.091	-0.091	0.000	15.449
69	69	70	Y	-0.091	-0.091	0.000	15.460
70	70	71	Y	-0.091	-0.091	0.000	1.089
71	71	72	Y	-0.091	-0.091	0.000	0.580
72	72	73	Y	-0.091	-0.091	0.000	2.669
73	73	74	Y	-0.091	-0.091	0.000	0.580
74	74	75	Y	-0.091	-0.091	0.000	1.080
75	75	76	Y	-0.091	-0.091	0.000	15.539
76	76	77	Y	-0.091	-0.091	0.000	15.550
77	77	78	Y	-0.091	-0.091	0.000	15.540
78	78	79	Y	-0.091	-0.091	0.000	15.539
79	79	80	Y	-0.091	-0.091	0.000	15.460
80	80	81	Y	-0.091	-0.091	0.000	15.459
81	81	82	Y	-0.091	-0.091	0.000	15.449
82	82	83	Y	-0.091	-0.091	0.000	15.460
83	83	84	Y	-0.091	-0.091	0.000	1.669
84	84	85	Y	-0.091	-0.091	0.000	0.580
85	85	86	Y	-0.091	-0.091	0.000	1.500
86	86	87	Y	-0.091	-0.091	0.000	0.589
87	87	88	Y	-0.091	-0.091	0.000	1.660
88	88	89	Y	-0.091	-0.091	0.000	2.750
89	89	90	Y	-0.091	-0.091	0.000	2.750

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LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

Memb No	I Node	J Node	Dir	Start Magnitude (K/ft,F)	End Magnitude (K/ft,F)	Start Location (ft)	End Location (ft)
90	90	91	Y	-0.091	-0.091	0.000	2.750
91	91	92	Y	-0.091	-0.091	0.000	2.739

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LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

Load Combination No.	Description	Self Wt Dir	Fac	BLC Fac	BLC Fac	BLC Fac	BLC Fac	BLC Fac	DYNA	W S	E V
1	DEAD LD + INSUL'N			1	1						

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LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

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Dynamic Analysis Data

Number of modes (frequencies) : 15
 Basic Load Case for masses : 1
 BLC mass direction of action : Y only
 Acceleration of Gravity : 32.20 ft/sec**2

Load Combination is 1 : DEAD LD + INSUL'N
 Nodal Displacements

Node	Global X (in)	Global Y (in)	Rotation (rad)
1	0.00000	-0.00000	-0.00082
2	0.00000	-0.13569	-0.00056
3	0.00000	-0.19000	0.00000
4	0.00000	-0.13484	0.00056
5	0.00000	-0.00000	0.00081
6	0.00000	0.01058	0.00081
7	0.00000	0.01621	0.00081
8	0.00000	0.00858	-0.00043
9	0.00000	0.00558	-0.00043
10	0.00000	-0.00000	-0.00043
11	0.00000	-0.06767	-0.00024
12	0.00000	-0.08041	0.00011
13	0.00000	-0.03810	0.00029
14	0.00000	-0.00000	0.00001
15	0.00000	-0.03509	-0.00028
16	0.00000	-0.07566	-0.00010
17	0.00000	-0.06390	0.00023
18	0.00000	-0.00000	0.00041
19	0.00000	0.00531	0.00041
20	0.00000	0.00814	0.00041
21	0.00000	0.00834	-0.00042
22	0.00000	0.00543	-0.00042
23	0.00000	-0.00000	-0.00042
24	0.00000	-0.06633	-0.00024
25	0.00000	-0.07900	0.00010
26	0.00000	-0.03734	0.00029
27	0.00000	-0.00000	0.00000
28	0.00000	-0.03549	-0.00028
29	0.00000	-0.07612	-0.00010
30	0.00000	-0.06418	0.00023
31	0.00000	-0.00000	0.00041
32	0.00000	0.00533	0.00041
33	0.00000	0.00817	0.00041
34	0.00000	0.00834	-0.00042
35	0.00000	0.00543	-0.00042
36	0.00000	-0.00000	-0.00042

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LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

Node	Global X (in)	Global Y (in)	Rotation (rad)
37	0.00000	-0.06633	-0.00024
38	0.00000	-0.07901	0.00010
39	0.00000	-0.03735	0.00029
40	0.00000	-0.00000	0.00000
41	0.00000	-0.03549	-0.00028
42	0.00000	-0.07611	-0.00010
43	0.00000	-0.06418	0.00023
44	0.00000	-0.00000	0.00041
45	0.00000	0.00533	0.00041
46	0.00000	0.00817	0.00041
47	0.00000	0.00834	-0.00042
48	0.00000	0.00543	-0.00042
49	0.00000	-0.00000	-0.00042
50	0.00000	-0.06633	-0.00024
51	0.00000	-0.07901	0.00010
52	0.00000	-0.03735	0.00029
53	0.00000	-0.00000	0.00000
54	0.00000	-0.03549	-0.00028
55	0.00000	-0.07611	-0.00010
56	0.00000	-0.06418	0.00023
57	0.00000	-0.00000	0.00041
58	0.00000	0.00533	0.00041
59	0.00000	0.00817	0.00041
60	0.00000	0.00834	-0.00042
61	0.00000	0.00543	-0.00042
62	0.00000	-0.00000	-0.00042
63	0.00000	-0.06633	-0.00024
64	0.00000	-0.07900	0.00010
65	0.00000	-0.03734	0.00029
66	0.00000	-0.00000	0.00000
67	0.00000	-0.03551	-0.00028
68	0.00000	-0.07614	-0.00010
69	0.00000	-0.06421	0.00023
70	0.00000	-0.00000	0.00041
71	0.00000	0.00534	0.00041
72	0.00000	0.00818	0.00041
73	0.00000	0.00850	-0.00043
74	0.00000	0.00553	-0.00043
75	0.00000	-0.00000	-0.00043
76	0.00000	-0.06781	-0.00025
77	0.00000	-0.08139	0.00010
78	0.00000	-0.03944	0.00030
79	0.00000	-0.00000	0.00002
80	0.00000	-0.03160	-0.00026
81	0.00000	-0.06893	-0.00009

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LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

Node	Global X (in)	Global Y (in)	Rotation (rad)
82	0.00000	-0.05730	0.00021
83	0.00000	-0.00000	0.00035
84	0.00000	0.00692	0.00034
85	0.00000	0.00931	0.00034
86	0.00000	0.00034	-0.00001
87	0.00000	0.00024	-0.00001
88	0.00000	0.00000	-0.00001
89	0.00000	-0.00021	-0.00000
90	0.00000	-0.00021	0.00000
91	0.00000	-0.00008	0.00000
92	0.00000	-0.00000	0.00000

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LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

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 Load Combination is 1 : DEAD LD + INSUL'N
 Spring Reactions

Node	Global X (K)	Global Y (K)	Moment (K-ft)
1	0.00000	2.79271	0.00000
5	0.00000	3.35017	0.00000
10	0.00000	2.15310	0.00000
14	0.00000	7.04581	0.00000
18	0.00000	2.38115	0.00000
23	0.00000	2.41705	0.00000
27	0.00000	7.02891	0.00000
31	0.00000	2.38504	0.00000
36	0.00000	2.41599	0.00000
40	0.00000	7.02898	0.00000
44	0.00000	2.38502	0.00000
49	0.00000	2.41600	0.00000
53	0.00000	7.02898	0.00000
57	0.00000	2.38505	0.00000
62	0.00000	2.41592	0.00000
66	0.00000	7.02933	0.00000
70	0.00000	2.37957	0.00000
75	0.00000	2.43589	0.00000
79	0.00000	6.94163	0.00000
83	0.00000	3.65638	0.00000
88	0.00000	-0.96797	0.00000
92	0.00000	1.04790	-2.92290
Totals	0.00000	78.15262	-2.92290

LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

Load Combination is 1 : DEAD LD + INSUL'N
 Member End Forces

No	Nodes		I-End			J-End		
	I	J	Axial (K)	Shear (K)	Moment (K-ft)	Axial (K)	Shear (K)	Moment (K-ft)
1	1-	2	0.00	2.79	-0.00	0.00	-1.39	32.30
2	2-	3	0.00	1.39	-32.30	0.00	0.02	42.85
3	3-	4	0.00	-0.02	-42.85	0.00	1.43	31.67
4	4-	5	0.00	-1.43	-31.67	0.00	2.83	-1.27
5	5-	6	0.00	0.52	1.27	0.00	-0.42	-0.76
6	6-	7	0.00	0.42	0.76	0.00	-0.36	-0.54
7	7-	8	0.00	0.36	0.54	0.00	-0.12	0.11
8	8-	9	0.00	0.12	-0.11	0.00	-0.07	0.17
9	9-	10	0.00	0.07	-0.17	0.00	0.03	0.19
10	10-	11	0.00	2.12	-0.19	0.00	-0.71	22.20
11	11-	12	0.00	0.71	-22.20	0.00	0.71	22.23
12	12-	13	0.00	-0.71	-22.23	0.00	2.12	0.27
13	13-	14	0.00	-2.12	-0.27	0.00	3.53	-43.66
14	14-	15	0.00	3.51	43.66	0.00	-2.10	-0.24
15	15-	16	0.00	2.10	0.24	0.00	-0.70	21.43
16	16-	17	0.00	0.70	-21.43	0.00	0.71	21.35
17	17-	18	0.00	-0.71	-21.35	0.00	2.11	-0.47
18	18-	19	0.00	0.27	0.47	0.00	-0.17	-0.23
19	19-	20	0.00	0.17	0.23	0.00	-0.11	-0.15
20	20-	21	0.00	0.11	0.15	0.00	0.13	-0.17
21	21-	22	0.00	-0.13	0.17	0.00	0.18	-0.26
22	22-	23	0.00	-0.18	0.26	0.00	0.28	-0.51
23	23-	24	0.00	2.14	0.51	0.00	-0.72	21.72
24	24-	25	0.00	0.72	-21.72	0.00	0.69	21.97
25	25-	26	0.00	-0.69	-21.97	0.00	2.11	0.23
26	26-	27	0.00	-2.11	-0.23	0.00	3.52	-43.48
27	27-	28	0.00	3.51	43.48	0.00	-2.10	-0.11
28	28-	29	0.00	2.10	0.11	0.00	-0.70	21.51
29	29-	30	0.00	0.70	-21.51	0.00	0.71	21.39
30	30-	31	0.00	-0.71	-21.39	0.00	2.12	-0.47
31	31-	32	0.00	0.27	0.47	0.00	-0.17	-0.23
32	32-	33	0.00	0.17	0.23	0.00	-0.12	-0.15
33	33-	34	0.00	0.12	0.15	0.00	0.13	-0.17
34	34-	35	0.00	-0.13	0.17	0.00	0.18	-0.26
35	35-	36	0.00	-0.18	0.26	0.00	0.28	-0.50
36	36-	37	0.00	2.14	0.50	0.00	-0.72	21.72
37	37-	38	0.00	0.72	-21.72	0.00	0.69	21.97
38	38-	39	0.00	-0.69	-21.97	0.00	2.11	0.23
39	39-	40	0.00	-2.11	-0.23	0.00	3.52	-43.48
40	40-	41	0.00	3.51	43.48	0.00	-2.10	-0.11
41	41-	42	0.00	2.10	0.11	0.00	-0.70	21.51
42	42-	43	0.00	0.70	-21.51	0.00	0.71	21.39
43	43-	44	0.00	-0.71	-21.39	0.00	2.12	-0.47

LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

No	Nodes		I-End			J-End		
	I	J	Axial (K)	Shear (K)	Moment (K-ft)	Axial (K)	Shear (K)	Moment (K-ft)
44	44-	45	0.00	0.27	0.47	0.00	-0.17	-0.23
45	45-	46	0.00	0.17	0.23	0.00	-0.12	-0.15
46	46-	47	0.00	0.12	0.15	0.00	0.13	-0.17
47	47-	48	0.00	-0.13	0.17	0.00	0.18	-0.26
48	48-	49	0.00	-0.18	0.26	0.00	0.28	-0.50
49	49-	50	0.00	2.14	0.50	0.00	-0.72	21.72
50	50-	51	0.00	0.72	-21.72	0.00	0.69	21.97
51	51-	52	0.00	-0.69	-21.97	0.00	2.11	0.23
52	52-	53	0.00	-2.11	-0.23	0.00	3.52	-43.48
53	53-	54	0.00	3.51	43.48	0.00	-2.10	-0.11
54	54-	55	0.00	2.10	0.11	0.00	-0.70	21.51
55	55-	56	0.00	0.70	-21.51	0.00	0.71	21.39
56	56-	57	0.00	-0.71	-21.39	0.00	2.12	-0.47
57	57-	58	0.00	0.27	0.47	0.00	-0.17	-0.23
58	58-	59	0.00	0.17	0.23	0.00	-0.12	-0.15
59	59-	60	0.00	0.12	0.15	0.00	0.13	-0.17
60	60-	61	0.00	-0.13	0.17	0.00	0.18	-0.26
61	61-	62	0.00	-0.18	0.26	0.00	0.28	-0.50
62	62-	63	0.00	2.14	0.50	0.00	-0.72	21.72
63	63-	64	0.00	0.72	-21.72	0.00	0.69	21.97
64	64-	65	0.00	-0.69	-21.97	0.00	2.11	0.23
65	65-	66	0.00	-2.11	-0.23	0.00	3.52	-43.49
66	66-	67	0.00	3.51	43.49	0.00	-2.10	-0.11
67	67-	68	0.00	2.10	0.11	0.00	-0.70	21.52
68	68-	69	0.00	0.70	-21.52	0.00	0.71	21.40
69	69-	70	0.00	-0.71	-21.40	0.00	2.12	-0.46
70	70-	71	0.00	0.26	0.46	0.00	-0.16	-0.22
71	71-	72	0.00	0.16	0.22	0.00	-0.11	-0.14
72	72-	73	0.00	0.11	0.14	0.00	0.13	-0.17
73	73-	74	0.00	-0.13	0.17	0.00	0.19	-0.27
74	74-	75	0.00	-0.19	0.27	0.00	0.28	-0.52
75	75-	76	0.00	2.15	0.52	0.00	-0.74	21.94
76	76-	77	0.00	0.74	-21.94	0.00	0.68	22.41
77	77-	78	0.00	-0.68	-22.41	0.00	2.09	0.90
78	78-	79	0.00	-2.09	-0.90	0.00	3.51	-42.58
79	79-	80	0.00	3.44	42.58	0.00	-2.03	-0.33
80	80-	81	0.00	2.03	0.33	0.00	-0.62	20.17
81	81-	82	0.00	0.62	-20.17	0.00	0.78	18.92
82	82-	83	0.00	-0.78	-18.92	0.00	2.19	-4.06
83	83-	84	0.00	1.47	4.06	0.00	-1.31	-1.74
84	84-	85	0.00	1.31	1.74	0.00	-1.26	-0.99
85	85-	86	0.00	1.26	0.99	0.00	-1.12	0.80
86	86-	87	0.00	1.12	-0.80	0.00	-1.07	1.45
87	87-	88	0.00	1.07	-1.45	0.00	-0.92	3.10
88	88-	89	0.00	-0.05	-3.10	0.00	0.30	2.62

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LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

No	Nodes		I-End			J-End		
	I	J	Axial (K)	Shear (K)	Moment (K-ft)	Axial (K)	Shear (K)	Moment (K-ft)
89	89-	90	0.00	-0.30	-2.62	0.00	0.55	1.46
90	90-	91	0.00	-0.55	-1.46	0.00	0.80	-0.39
91	91-	92	0.00	-0.80	0.39	0.00	1.05	-2.92

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LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

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 Load Combination is 1 : DEAD LD + INSUL'N
 AISC Code Checks

No	Nodes		Maximum	0	Member Quarter Points			L	Shear
	I	J			1/4	1/2	3/4		
1	1-	2			-	Not Calculated	-		
2	2-	3			-	Not Calculated	-		
3	3-	4			-	Not Calculated	-		
4	4-	5			-	Not Calculated	-		
5	5-	6			-	Not Calculated	-		
6	6-	7			-	Not Calculated	-		
7	7-	8			-	Not Calculated	-		
8	8-	9			-	Not Calculated	-		
9	9-	10			-	Not Calculated	-		
10	10-	11			-	Not Calculated	-		
11	11-	12			-	Not Calculated	-		
12	12-	13			-	Not Calculated	-		
13	13-	14			-	Not Calculated	-		
14	14-	15			-	Not Calculated	-		
15	15-	16			-	Not Calculated	-		
16	16-	17			-	Not Calculated	-		
17	17-	18			-	Not Calculated	-		
18	18-	19			-	Not Calculated	-		
19	19-	20			-	Not Calculated	-		
20	20-	21			-	Not Calculated	-		
21	21-	22			-	Not Calculated	-		
22	22-	23			-	Not Calculated	-		
23	23-	24			-	Not Calculated	-		
24	24-	25			-	Not Calculated	-		
25	25-	26			-	Not Calculated	-		
26	26-	27			-	Not Calculated	-		
27	27-	28			-	Not Calculated	-		
28	28-	29			-	Not Calculated	-		
29	29-	30			-	Not Calculated	-		
30	30-	31			-	Not Calculated	-		
31	31-	32			-	Not Calculated	-		
32	32-	33			-	Not Calculated	-		
33	33-	34			-	Not Calculated	-		
34	34-	35			-	Not Calculated	-		
35	35-	36			-	Not Calculated	-		
36	36-	37			-	Not Calculated	-		
37	37-	38			-	Not Calculated	-		
38	38-	39			-	Not Calculated	-		
39	39-	40			-	Not Calculated	-		
40	40-	41			-	Not Calculated	-		
41	41-	42			-	Not Calculated	-		
42	42-	43			-	Not Calculated	-		
43	43-	44			-	Not Calculated	-		

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LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

No	Nodes		Maximum	0	Member Quarter Points			L	Shear
	I	J			1/4	1/2	3/4		
44	44-	45			-	Not Calculated	-		
45	45-	46			-	Not Calculated	-		
46	46-	47			-	Not Calculated	-		
47	47-	48			-	Not Calculated	-		
48	48-	49			-	Not Calculated	-		
49	49-	50			-	Not Calculated	-		
50	50-	51			-	Not Calculated	-		
51	51-	52			-	Not Calculated	-		
52	52-	53			-	Not Calculated	-		
53	53-	54			-	Not Calculated	-		
54	54-	55			-	Not Calculated	-		
55	55-	56			-	Not Calculated	-		
56	56-	57			-	Not Calculated	-		
57	57-	58			-	Not Calculated	-		
58	58-	59			-	Not Calculated	-		
59	59-	60			-	Not Calculated	-		
60	60-	61			-	Not Calculated	-		
61	61-	62			-	Not Calculated	-		
62	62-	63			-	Not Calculated	-		
63	63-	64			-	Not Calculated	-		
64	64-	65			-	Not Calculated	-		
65	65-	66			-	Not Calculated	-		
66	66-	67			-	Not Calculated	-		
67	67-	68			-	Not Calculated	-		
68	68-	69			-	Not Calculated	-		
69	69-	70			-	Not Calculated	-		
70	70-	71			-	Not Calculated	-		
71	71-	72			-	Not Calculated	-		
72	72-	73			-	Not Calculated	-		
73	73-	74			-	Not Calculated	-		
74	74-	75			-	Not Calculated	-		
75	75-	76			-	Not Calculated	-		
76	76-	77			-	Not Calculated	-		
77	77-	78			-	Not Calculated	-		
78	78-	79			-	Not Calculated	-		
79	79-	80			-	Not Calculated	-		
80	80-	81			-	Not Calculated	-		
81	81-	82			-	Not Calculated	-		
82	82-	83			-	Not Calculated	-		
83	83-	84			-	Not Calculated	-		
84	84-	85			-	Not Calculated	-		
85	85-	86			-	Not Calculated	-		
86	86-	87			-	Not Calculated	-		
87	87-	88			-	Not Calculated	-		
88	88-	89			-	Not Calculated	-		

LIGO BEAM TUBE - BAKE + SELF WT, HANFORD
 859' LENGTH, SIMPLE SUPPT - FIXED SUPPT

No	Nodes		Maximum	0	Member Quarter Points			L	Shear
	I	J			1/4	1/2	3/4		
89	89-	90			-	Not Calculated	-		
90	90-	91			-	Not Calculated	-		
91	91-	92			-	Not Calculated	-		

**RISA-2D MODEL OF LIGO BEAM TUBE MODULE @ LIVINGSTON SITE: CASE #3 NODE NUMBERS
CASE #3: SIMPLE SUPPORTS AT ONE END, RIGID SUPPORT AT OTHER, TOTAL LENGTH = 878.5'**

NODE	X (FT)	DESCRIPTION
1	0.000	FIXED SUPPT, ROTATION FREE
2	15.458	
3	30.917	
4	46.375	
5	61.833	GUIDED SUPPT
6	63.500	END TUBE / START EJ STUB
7	64.083	END EJ STUB / START BELLOWS
8	65.583	END BELLOWS / START EJ STUB
9	66.167	END EJ STUB / START TUBE
10	67.833	GUIDED SUPPT
11	83.375	
12	98.917	
13	114.458	
14	130.000	FIXED SUPPT, ROTATION FREE
15	145.458	
16	160.917	
17	176.375	
18	191.833	GUIDED SUPPT
19	193.500	END TUBE / START EJ STUB
20	194.083	END EJ STUB / START BELLOWS
21	195.583	END BELLOWS / START EJ STUB
22	196.167	END EJ STUB / START TUBE
23	197.833	GUIDED SUPPT
24	213.375	
25	228.917	
26	244.458	
27	260.000	FIXED SUPPT, ROTATION FREE
28	275.458	
29	290.917	
30	306.375	
31	321.833	GUIDED SUPPT
32	323.500	END TUBE / START EJ STUB
33	324.083	END EJ STUB / START BELLOWS
34	325.583	END BELLOWS / START EJ STUB
35	326.167	END EJ STUB / START TUBE
36	327.833	GUIDED SUPPT
37	343.375	
38	358.917	
39	374.458	
40	390.000	FIXED SUPPT, ROTATION FREE
41	405.458	
42	420.917	
43	436.375	
44	451.833	GUIDED SUPPT
45	453.500	END TUBE / START EJ STUB
46	454.083	END EJ STUB / START BELLOWS
47	455.583	END BELLOWS / START EJ STUB
48	456.167	END EJ STUB / START TUBE
49	457.833	GUIDED SUPPT

SUBJECT RISA-2D MODEL OF BEAM TUBE MODULE, CASE #3 NODE NUMBERS, COORDINATES & DESCRIPTION	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD P.J.	MADE BY	CHKD BY	SHT 1 OF 2
	DATE 2/28/94	DATE 2/28/94	DATE	DATE	5.99

**RISA-2D MODEL OF LIGO BEAM TUBE MODULE @ LIVINGSTON SITE: CASE #3 NODE NUMBERS
CASE #3: SIMPLE SUPPORTS AT ONE END, RIGID SUPPORT AT OTHER, TOTAL LENGTH = 878.5'**

NODE	X(FT)	DESCRIPTION
50	473.375	
51	488.917	
52	504.458	
53	520.000	FIXED SUPPT, ROTATION FREE
54	535.458	
55	550.917	
56	566.375	
57	581.833	GUIDED SUPPT
58	583.500	END TUBE / START EJ STUB
59	584.083	END EJ STUB / START BELLOWS
60	585.583	END BELLOWS / START EJ STUB
61	586.167	END EJ STUB / START TUBE
62	587.833	GUIDED SUPPT
63	603.375	
64	618.917	
65	634.458	
66	650.000	FIXED SUPPT, ROTATION FREE
67	665.458	
68	680.917	
69	696.375	
70	711.833	GUIDED SUPPT
71	713.500	END TUBE / START EJ STUB
72	714.083	END EJ STUB / START BELLOWS
73	715.583	END BELLOWS / START EJ STUB
74	716.167	END EJ STUB / START TUBE
75	717.833	GUIDED SUPPT
76	733.375	
77	748.917	
78	764.458	
79	780.000	FIXED SUPPT, ROTATION FREE
80	795.458	
81	810.917	
82	826.375	
83	841.833	GUIDED SUPPT
84	843.500	END TUBE / START EJ STUB
85	844.083	END EJ STUB / START BELLOWS
86	845.583	END BELLOWS / START EJ STUB
87	846.167	END EJ STUB / START TUBE
88	847.833	GUIDED SUPPT
89	855.501	
90	863.169	
91	870.837	
92	878.505	RIGID SUPPT, ROTATION FIXED

SUBJECT RISA-2D MODEL OF BEAM TUBE MODULE, CASE #3 NODE NUMBERS, COORDINATES & DESCRIPTION	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD <i>(Signature)</i>	MADE BY	CHKD BY	SHT 2 OF 2
	DATE 2/28/94	DATE 15 APR 94	DATE	DATE	5.100.

LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

Units Option : US Standard

AISC Code Checks : 9th Edition ASD

Shear Deformation: No

P-Delta Effects : No

Redesign : No

Edge Forces : No

A.S.I.F. : 1.333

DESIGNED BY WJC	CHECKED BY Rsu	BY	
DATE 3-18-94	DATE 18 MAR 1994	CHECKED	
		DATE	

LIGO BEAM TUBE -- NATURAL PERIOD MODEL
130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

Units Option : US Standard

AISC Code Checks : 9th Edition ASD

Shear Deformation: No

P-Delta Effects : No

Redesign : No

Edge Forces : No

A.S.I.F. : 1.333

SUBJECT	OFFICE NoE C		REVISION		REFERENCE NO.
	MADE BY NLC	CHKD BY KSW	MADE BY	CHKD BY	SMT ___ CF ___
	DATE 3-18-94	DATE 15 APR 94	DATE	DATE	

LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
 879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

Node No	Boundary Conditions					
	X-Coord (ft)	Y-Coord (ft)	X-dof (in,K/in)	Y-dof (in,K/in)	Rotation (r,K-ft/r)	Temp. (F)
1	0.00	0.00	R	R		0.00
2	15.46	0.00				0.00
3	30.92	0.00				0.00
4	46.37	0.00				0.00
5	61.83	0.00				0.00
6	62.92	0.00			R	0.00
7	63.50	0.00				0.00
8	66.17	0.00				0.00
9	66.75	0.00				0.00
10	67.83	0.00			R	0.00
11	83.37	0.00				0.00
12	98.92	0.00				0.00
13	114.46	0.00				0.00
14	130.00	0.00	R	R		0.00
15	145.46	0.00				0.00
16	160.92	0.00				0.00
17	176.37	0.00				0.00
18	191.83	0.00			R	0.00
19	192.92	0.00				0.00
20	193.50	0.00				0.00
21	196.17	0.00				0.00
22	196.75	0.00				0.00
23	197.83	0.00			R	0.00
24	213.37	0.00				0.00
25	228.92	0.00				0.00
26	244.46	0.00				0.00
27	260.00	0.00	R	R		0.00
28	275.46	0.00				0.00
29	290.92	0.00				0.00
30	306.37	0.00				0.00
31	321.83	0.00			R	0.00
32	322.92	0.00				0.00
33	323.50	0.00				0.00
34	326.17	0.00				0.00
35	326.75	0.00				0.00
36	327.83	0.00			R	0.00
37	343.37	0.00				0.00
38	358.92	0.00				0.00
39	374.46	0.00				0.00
40	390.00	0.00	R	R		0.00
41	405.46	0.00				0.00
42	420.92	0.00				0.00
43	436.37	0.00				0.00
44	451.83	0.00			R	0.00
45	452.92	0.00				0.00

LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
 879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

Node		Boundary Conditions					Temp.
No	X-Coord	Y-Coord	X-dof	Y-dof	Rotation		
	(ft)	(ft)	(in,K/in)	(in,K/in)	(r,K-ft/r)	(F)	
91	870.84	0.00				0.00	
92	878.51	0.00	R	R	R	0.00	

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LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
 879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

Material Label	Elastic Modulus (Ksi)	Poisson's Ratio	Thermal Coefficient (F)	Weight Density (K/ft3)	Yield Stress (Fy) (Ksi)
1	27000.00	0.30000	0.65000	0.000	19.200

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LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
 879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

Section Label	Database Shape	Matl. Set	Area (in ²)	Moment of Inertia (in ⁴)	As Coef	y/y
BEAMTUBE		1	19.19	5731.030	1.20	
STUBEND		1	16.13	4820.000	1.20	
EXPJOINT		1	0.01	1.803	1.20	

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LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
 879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

No	I J		Section	I Releases J			Sec	Sway	End Offsets		Length
	Node	Node		x	y	z			x	y	
									(in)	(in)	(ft)
1	1	-	2								15.46
2	2	-	3								15.46
3	3	-	4								15.45
4	4	-	5								15.46
5	5	-	6								1.09
6	6	-	7								0.58
7	7	-	8								2.67
8	8	-	9								0.58
9	9	-	10								1.08
10	10	-	11								15.54
11	11	-	12								15.55
12	12	-	13								15.54
13	13	-	14								15.54
14	14	-	15								15.46
15	15	-	16								15.46
16	16	-	17								15.45
17	17	-	18								15.46
18	18	-	19								1.09
19	19	-	20								0.58
20	20	-	21								2.67
21	21	-	22								0.58
22	22	-	23								1.08
23	23	-	24								15.54
24	24	-	25								15.55
25	25	-	26								15.54
26	26	-	27								15.54
27	27	-	28								15.46
28	28	-	29								15.46
29	29	-	30								15.45
30	30	-	31								15.46
31	31	-	32								1.09
32	32	-	33								0.58
33	33	-	34								2.67
34	34	-	35								0.58
35	35	-	36								1.08
36	36	-	37								15.54
37	37	-	38								15.54
38	38	-	39								15.54
39	39	-	40								15.54
40	40	-	41								15.46
41	41	-	42								15.46
42	42	-	43								15.46
43	43	-	44								15.46
44	44	-	45								1.09
45	45	-	46								0.58

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LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
 879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

I No	I Node	J Node	Section	I Releases			J			End Offsets		Length (ft)
				x	y	z	x	y	z	Sec	Sway	
46	46	-	47	EXPJOINT								2.67
47	47	-	48	STUBEND								0.58
48	48	-	49	BEAMTUBE								1.08
49	49	-	50	BEAMTUBE								15.54
50	50	-	51	BEAMTUBE								15.55
51	51	-	52	BEAMTUBE								15.54
52	52	-	53	BEAMTUBE								15.54
53	53	-	54	BEAMTUBE								15.46
54	54	-	55	BEAMTUBE								15.46
55	55	-	56	BEAMTUBE								15.45
56	56	-	57	BEAMTUBE								15.46
57	57	-	58	BEAMTUBE								1.09
58	58	-	59	STUBEND								0.58
59	59	-	60	EXPJOINT								2.67
60	60	-	61	STUBEND								0.58
61	61	-	62	BEAMTUBE								1.08
62	62	-	63	BEAMTUBE								15.54
63	63	-	64	BEAMTUBE								15.55
64	64	-	65	BEAMTUBE								15.54
65	65	-	66	BEAMTUBE								15.54
66	66	-	67	BEAMTUBE								15.46
67	67	-	68	BEAMTUBE								15.46
68	68	-	69	BEAMTUBE								15.45
69	69	-	70	BEAMTUBE								15.46
70	70	-	71	BEAMTUBE								1.09
71	71	-	72	STUBEND								0.58
72	72	-	73	EXPJOINT								2.67
73	73	-	74	STUBEND								0.58
74	74	-	75	BEAMTUBE								1.08
75	75	-	76	BEAMTUBE								15.54
76	76	-	77	BEAMTUBE								15.55
77	77	-	78	BEAMTUBE								15.54
78	78	-	79	BEAMTUBE								15.54
79	79	-	80	BEAMTUBE								15.46
80	80	-	81	BEAMTUBE								15.46
81	81	-	82	BEAMTUBE								15.45
82	82	-	83	BEAMTUBE								15.46
83	83	-	84	BEAMTUBE								1.61
84	84	-	85	STUBEND								0.58
85	85	-	86	EXPJOINT								1.50
86	86	-	87	STUBEND								0.58
87	87	-	88	BEAMTUBE								1.61
88	88	-	89	BEAMTUBE								7.61
89	89	-	90	BEAMTUBE								7.61
90	90	-	91	BEAMTUBE								7.61

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LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
 879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

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I No	Node	J Node	Section	I Releases J						End Offsets		Length		
				x	y	z	x	y	z	Sec	Sway		I (in)	J (in)
91	91	92	BEAMTUBE											7.67

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LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
 879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

I No	Node	J Node	Unbraced Lengths			K Factors		Bending Coefs	
			Lb-in (ft)	Lb-out (ft)	Lc (ft)	In	Out	Cm	Cb
1	1	-	2						
2	2	-	3						
3	3	-	4						
4	4	-	5						
5	5	-	6						
6	6	-	7						
7	7	-	8						
8	8	-	9						
9	9	-	10						
10	10	-	11						
11	11	-	12						
12	12	-	13						
13	13	-	14						
14	14	-	15						
15	15	-	16						
16	16	-	17						
17	17	-	18						
18	18	-	19						
19	19	-	20						
20	20	-	21						
21	21	-	22						
22	22	-	23						
23	23	-	24						
24	24	-	25						
25	25	-	26						
26	26	-	27						
27	27	-	28						
28	28	-	29						
29	29	-	30						
30	30	-	31						
31	31	-	32						
32	32	-	33						
33	33	-	34						
34	34	-	35						
35	35	-	36						
36	36	-	37						
37	37	-	38						
38	38	-	39						
39	39	-	40						
40	40	-	41						
41	41	-	42						
42	42	-	43						
43	43	-	44						
44	44	-	45						
45	45	-	46						

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LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
 879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

No	I J		Unbraced Lengths			K Factors		Bending Coefs	
	Node	Node	Lb-in	Lb-out	Lc	In	Out	Cm	Cb
			(ft)	(ft)	(ft)				
46	46	-	47						
47	47	-	48						
48	48	-	49						
49	49	-	50						
50	50	-	51						
51	51	-	52						
52	52	-	53						
53	53	-	54						
54	54	-	55						
55	55	-	56						
56	56	-	57						
57	57	-	58						
58	58	-	59						
59	59	-	60						
60	60	-	61						
61	61	-	62						
62	62	-	63						
63	63	-	64						
64	64	-	65						
65	65	-	66						
66	66	-	67						
67	67	-	68						
68	68	-	69						
69	69	-	70						
70	70	-	71						
71	71	-	72						
72	72	-	73						
73	73	-	74						
74	74	-	75						
75	75	-	76						
76	76	-	77						
77	77	-	78						
78	78	-	79						
79	79	-	80						
80	80	-	81						
81	81	-	82						
82	82	-	83						
83	83	-	84						
84	84	-	85						
85	85	-	86						
86	86	-	87						
87	87	-	88						
88	88	-	89						
89	89	-	90						
90	90	-	91						

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LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
 879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

I No	J Node	Unbraced Lengths			K Factors		Bending Coefs	
		Lb-in (ft)	Lb-out (ft)	Lc (ft)	In	Out	Cm	Cb
91	91 - 92							

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LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

BLC No.	Basic Load Case Description	Load Totals		
		Nodal	Point	Dist.
1	SELF WEIGHT + INSULATION			91

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LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
 879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

Member Distributed Loads, BLC 1: SELF WEIGHT + INSULATION

Membr	I	J	Dir	Start	End	Start	End
No	Node	Node		Magnitude	Magnitude	Location	Location
				(K/ft,F)	(K/ft,F)	(ft)	(ft)
1	1	2	Y	-0.091	-0.091	0.000	15.460
2	2	3	Y	-0.091	-0.091	0.000	15.460
3	3	4	Y	-0.091	-0.091	0.000	15.449
4	4	5	Y	-0.091	-0.091	0.000	15.460
5	5	6	Y	-0.091	-0.091	0.000	1.089
6	6	7	Y	-0.091	-0.091	0.000	0.580
7	7	8	Y	-0.091	-0.091	0.000	2.669
8	8	9	Y	-0.091	-0.091	0.000	0.580
9	9	10	Y	-0.091	-0.091	0.000	1.080
10	10	11	Y	-0.091	-0.091	0.000	15.540
11	11	12	Y	-0.091	-0.091	0.000	15.550
12	12	13	Y	-0.091	-0.091	0.000	15.539
13	13	14	Y	-0.091	-0.091	0.000	15.540
14	14	15	Y	-0.091	-0.091	0.000	15.460
15	15	16	Y	-0.091	-0.091	0.000	15.459
16	16	17	Y	-0.091	-0.091	0.000	15.449
17	17	18	Y	-0.091	-0.091	0.000	15.460
18	18	19	Y	-0.091	-0.091	0.000	1.089
19	19	20	Y	-0.091	-0.091	0.000	0.580
20	20	21	Y	-0.091	-0.091	0.000	2.669
21	21	22	Y	-0.091	-0.091	0.000	0.580
22	22	23	Y	-0.091	-0.091	0.000	1.080
23	23	24	Y	-0.091	-0.091	0.000	15.539
24	24	25	Y	-0.091	-0.091	0.000	15.550
25	25	26	Y	-0.091	-0.091	0.000	15.540
26	26	27	Y	-0.091	-0.091	0.000	15.539
27	27	28	Y	-0.091	-0.091	0.000	15.459
28	28	29	Y	-0.091	-0.091	0.000	15.460
29	29	30	Y	-0.091	-0.091	0.000	15.449
30	30	31	Y	-0.091	-0.091	0.000	15.459
31	31	32	Y	-0.091	-0.091	0.000	1.090
32	32	33	Y	-0.091	-0.091	0.000	0.579
33	33	34	Y	-0.091	-0.091	0.000	2.670
34	34	35	Y	-0.091	-0.091	0.000	0.579
35	35	36	Y	-0.091	-0.091	0.000	1.079
36	36	37	Y	-0.091	-0.091	0.000	15.540
37	37	38	Y	-0.091	-0.091	0.000	15.550
38	38	39	Y	-0.091	-0.091	0.000	15.539
39	39	40	Y	-0.091	-0.091	0.000	15.540
40	40	41	Y	-0.091	-0.091	0.000	15.459
41	41	42	Y	-0.091	-0.091	0.000	15.460
42	42	43	Y	-0.091	-0.091	0.000	15.449
43	43	44	Y	-0.091	-0.091	0.000	15.459
44	44	45	Y	-0.091	-0.091	0.000	1.090

LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
 879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

Memb No	I Node	J Node	Dir	Start Magnitude (K/ft,F)	End Magnitude (K/ft,F)	Start Location (ft)	End Location (ft)
45	45	46	Y	-0.091	-0.091	0.000	0.579
46	46	47	Y	-0.091	-0.091	0.000	2.670
47	47	48	Y	-0.091	-0.091	0.000	0.579
48	48	49	Y	-0.091	-0.091	0.000	1.079
49	49	50	Y	-0.091	-0.091	0.000	15.540
50	50	51	Y	-0.091	-0.091	0.000	15.550
51	51	52	Y	-0.091	-0.091	0.000	15.539
52	52	53	Y	-0.091	-0.091	0.000	15.540
53	53	54	Y	-0.091	-0.091	0.000	15.460
54	54	55	Y	-0.091	-0.091	0.000	15.459
55	55	56	Y	-0.091	-0.091	0.000	15.450
56	56	57	Y	-0.091	-0.091	0.000	15.460
57	57	58	Y	-0.091	-0.091	0.000	1.089
58	58	59	Y	-0.091	-0.091	0.000	0.580
59	59	60	Y	-0.091	-0.091	0.000	2.669
60	60	61	Y	-0.091	-0.091	0.000	0.580
61	61	62	Y	-0.091	-0.091	0.000	1.080
62	62	63	Y	-0.091	-0.091	0.000	15.539
63	63	64	Y	-0.091	-0.091	0.000	15.549
64	64	65	Y	-0.091	-0.091	0.000	15.540
65	65	66	Y	-0.091	-0.091	0.000	15.539
66	66	67	Y	-0.091	-0.091	0.000	15.460
67	67	68	Y	-0.091	-0.091	0.000	15.459
68	68	69	Y	-0.091	-0.091	0.000	15.449
69	69	70	Y	-0.091	-0.091	0.000	15.460
70	70	71	Y	-0.091	-0.091	0.000	1.089
71	71	72	Y	-0.091	-0.091	0.000	0.580
72	72	73	Y	-0.091	-0.091	0.000	2.669
73	73	74	Y	-0.091	-0.091	0.000	0.580
74	74	75	Y	-0.091	-0.091	0.000	1.080
75	75	76	Y	-0.091	-0.091	0.000	15.539
76	76	77	Y	-0.091	-0.091	0.000	15.550
77	77	78	Y	-0.091	-0.091	0.000	15.540
78	78	79	Y	-0.091	-0.091	0.000	15.539
79	79	80	Y	-0.091	-0.091	0.000	15.460
80	80	81	Y	-0.091	-0.091	0.000	15.459
81	81	82	Y	-0.091	-0.091	0.000	15.449
82	82	83	Y	-0.091	-0.091	0.000	15.460
83	83	84	Y	-0.091	-0.091	0.000	1.669
84	84	85	Y	-0.091	-0.091	0.000	0.580
85	85	86	Y	-0.091	-0.091	0.000	1.500
86	86	87	Y	-0.091	-0.091	0.000	0.589
87	87	88	Y	-0.091	-0.091	0.000	1.660
88	88	89	Y	-0.091	-0.091	0.000	7.669
89	89	90	Y	-0.091	-0.091	0.000	7.669

LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
 879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

Memb No	I Node	J Node	Dir	Start Magnitude (K/ft,F)	End Magnitude (K/ft,F)	Start Location (ft)	End Location (ft)
90	90	91	Y	-0.091	-0.091	0.000	7.670
91	91	92	Y	-0.091	-0.091	0.000	7.669

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LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

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Load No.	Combination Description	Self Wt Dir Fac	BLC Fac	BLC Fac	BLC Fac	BLC Fac	BLC Fac	DYNA	W S	E V
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1	DEAD LD + INSUL'N		1	1						
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LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
 879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

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Dynamic Analysis Data

Number of modes (frequencies) : 15
 Basic Load Case for masses : 1
 BLC mass direction of action : Y only
 Acceleration of Gravity : 32.20 ft/sec**2

Load Combination is 1 : DEAD LD + INSUL'N
 Nodal Displacements

Node	Global X (in)	Global Y (in)	Rotation (rad)
1	0.00000	-0.00000	-0.00082
2	0.00000	-0.13569	-0.00056
3	0.00000	-0.19000	0.00000
4	0.00000	-0.13484	0.00056
5	0.00000	-0.00000	0.00081
6	0.00000	0.01058	0.00081
7	0.00000	0.01621	0.00081
8	0.00000	0.00858	-0.00043
9	0.00000	0.00558	-0.00043
10	0.00000	-0.00000	-0.00043
11	0.00000	-0.06767	-0.00024
12	0.00000	-0.08041	0.00011
13	0.00000	-0.03810	0.00029
14	0.00000	-0.00000	0.00001
15	0.00000	-0.03509	-0.00028
16	0.00000	-0.07566	-0.00010
17	0.00000	-0.06390	0.00023
18	0.00000	-0.00000	0.00041
19	0.00000	0.00531	0.00041
20	0.00000	0.00814	0.00041
21	0.00000	0.00834	-0.00042
22	0.00000	0.00543	-0.00042
23	0.00000	-0.00000	-0.00042
24	0.00000	-0.06633	-0.00024
25	0.00000	-0.07900	0.00010
26	0.00000	-0.03734	0.00029
27	0.00000	-0.00000	0.00000
28	0.00000	-0.03549	-0.00028
29	0.00000	-0.07612	-0.00010
30	0.00000	-0.06418	0.00023
31	0.00000	-0.00000	0.00041
32	0.00000	0.00533	0.00041
33	0.00000	0.00817	0.00041
34	0.00000	0.00834	-0.00042
35	0.00000	0.00543	-0.00042
36	0.00000	-0.00000	-0.00042

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LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
 879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

Node	Global X (in)	Global Y (in)	Rotation (rad)
37	0.00000	-0.06633	-0.00024
38	0.00000	-0.07901	0.00010
39	0.00000	-0.03735	0.00029
40	0.00000	-0.00000	0.00000
41	0.00000	-0.03549	-0.00028
42	0.00000	-0.07611	-0.00010
43	0.00000	-0.06418	0.00023
44	0.00000	-0.00000	0.00041
45	0.00000	0.00533	0.00041
46	0.00000	0.00817	0.00041
47	0.00000	0.00834	-0.00042
48	0.00000	0.00543	-0.00042
49	0.00000	-0.00000	-0.00042
50	0.00000	-0.06633	-0.00024
51	0.00000	-0.07901	0.00010
52	0.00000	-0.03735	0.00029
53	0.00000	-0.00000	0.00000
54	0.00000	-0.03549	-0.00028
55	0.00000	-0.07611	-0.00010
56	0.00000	-0.06418	0.00023
57	0.00000	-0.00000	0.00041
58	0.00000	0.00533	0.00041
59	0.00000	0.00817	0.00041
60	0.00000	0.00834	-0.00042
61	0.00000	0.00543	-0.00042
62	0.00000	-0.00000	-0.00042
63	0.00000	-0.06633	-0.00024
64	0.00000	-0.07900	0.00010
65	0.00000	-0.03734	0.00029
66	0.00000	-0.00000	0.00000
67	0.00000	-0.03550	-0.00028
68	0.00000	-0.07614	-0.00010
69	0.00000	-0.06421	0.00023
70	0.00000	-0.00000	0.00041
71	0.00000	0.00534	0.00041
72	0.00000	0.00818	0.00041
73	0.00000	0.00848	-0.00043
74	0.00000	0.00552	-0.00043
75	0.00000	-0.00000	-0.00043
76	0.00000	-0.06760	-0.00024
77	0.00000	-0.08105	0.00010
78	0.00000	-0.03914	0.00030
79	0.00000	-0.00000	0.00002
80	0.00000	-0.03216	-0.00026
81	0.00000	-0.06997	-0.00009

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LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

Node	Global X (in)	Global Y (in)	Rotation (rad)
82	0.00000	-0.05829	0.00022
83	0.00000	-0.00000	0.00036
84	0.00000	0.00710	0.00035
85	0.00000	0.00955	0.00035
86	0.00000	0.00192	-0.00007
87	0.00000	0.00141	-0.00007
88	0.00000	-0.00000	-0.00007
89	0.00000	-0.00490	-0.00003
90	0.00000	-0.00553	0.00002
91	0.00000	-0.00251	0.00004
92	0.00000	-0.00000	0.00000

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LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
 879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

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 Load Combination is 1 : DEAD LD + INSUL'N
 Spring Reactions

Node	Global X (K)	Global Y (K)	Moment (K-ft)
1	0.00000	2.79271	0.00000
5	0.00000	3.35017	0.00000
10	0.00000	2.15310	0.00000
14	0.00000	7.04581	0.00000
18	0.00000	2.38115	0.00000
23	0.00000	2.41705	0.00000
27	0.00000	7.02891	0.00000
31	0.00000	2.38504	0.00000
36	0.00000	2.41599	0.00000
40	0.00000	7.02898	0.00000
44	0.00000	2.38502	0.00000
49	0.00000	2.41600	0.00000
53	0.00000	7.02898	0.00000
57	0.00000	2.38504	0.00000
62	0.00000	2.41593	0.00000
66	0.00000	7.02928	0.00000
70	0.00000	2.38036	0.00000
75	0.00000	2.43302	0.00000
79	0.00000	6.95421	0.00000
83	0.00000	3.46843	0.00000
88	0.00000	0.17960	0.00000
92	0.00000	1.86962	-11.98205
Totals	0.00000	79.94441	-11.98205

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LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

Load Combination is 1 : DEAD LD + INSUL'N
Member End Forces

No	Nodes		I-End			J-End		
	I	J	Axial (K)	Shear (K)	Moment (K-ft)	Axial (K)	Shear (K)	Moment (K-ft)
1	1-	2	0.00	2.79	-0.00	0.00	-1.39	32.30
2	2-	3	0.00	1.39	-32.30	0.00	0.02	42.85
3	3-	4	0.00	-0.02	-42.85	0.00	1.43	31.67
4	4-	5	0.00	-1.43	-31.67	0.00	2.83	-1.27
5	5-	6	0.00	0.52	1.27	0.00	-0.42	-0.76
6	6-	7	0.00	0.42	0.76	0.00	-0.36	-0.54
7	7-	8	0.00	0.36	0.54	0.00	-0.12	0.11
8	8-	9	0.00	0.12	-0.11	0.00	-0.07	0.17
9	9-	10	0.00	0.07	-0.17	0.00	0.03	0.19
10	10-	11	0.00	2.12	-0.19	0.00	-0.71	22.20
11	11-	12	0.00	0.71	-22.20	0.00	0.71	22.23
12	12-	13	0.00	-0.71	-22.23	0.00	2.12	0.27
13	13-	14	0.00	-2.12	-0.27	0.00	3.53	-43.66
14	14-	15	0.00	3.51	43.66	0.00	-2.10	-0.24
15	15-	16	0.00	2.10	0.24	0.00	-0.70	21.43
16	16-	17	0.00	0.70	-21.43	0.00	0.71	21.35
17	17-	18	0.00	-0.71	-21.35	0.00	2.11	-0.47
18	18-	19	0.00	0.27	0.47	0.00	-0.17	-0.23
19	19-	20	0.00	0.17	0.23	0.00	-0.11	-0.15
20	20-	21	0.00	0.11	0.15	0.00	0.13	-0.17
21	21-	22	0.00	-0.13	0.17	0.00	0.18	-0.26
22	22-	23	0.00	-0.18	0.26	0.00	0.28	-0.51
23	23-	24	0.00	2.14	0.51	0.00	-0.72	21.72
24	24-	25	0.00	0.72	-21.72	0.00	0.69	21.97
25	25-	26	0.00	-0.69	-21.97	0.00	2.11	0.23
26	26-	27	0.00	-2.11	-0.23	0.00	3.52	-43.48
27	27-	28	0.00	3.51	43.48	0.00	-2.10	-0.11
28	28-	29	0.00	2.10	0.11	0.00	-0.70	21.51
29	29-	30	0.00	0.70	-21.51	0.00	0.71	21.39
30	30-	31	0.00	-0.71	-21.39	0.00	2.12	-0.47
31	31-	32	0.00	0.27	0.47	0.00	-0.17	-0.23
32	32-	33	0.00	0.17	0.23	0.00	-0.12	-0.15
33	33-	34	0.00	0.12	0.15	0.00	0.13	-0.17
34	34-	35	0.00	-0.13	0.17	0.00	0.18	-0.26
35	35-	36	0.00	-0.18	0.26	0.00	0.28	-0.50
36	36-	37	0.00	2.14	0.50	0.00	-0.72	21.72
37	37-	38	0.00	0.72	-21.72	0.00	0.69	21.97
38	38-	39	0.00	-0.69	-21.97	0.00	2.11	0.23
39	39-	40	0.00	-2.11	-0.23	0.00	3.52	-43.48
40	40-	41	0.00	3.51	43.48	0.00	-2.10	-0.11
41	41-	42	0.00	2.10	0.11	0.00	-0.70	21.51
42	42-	43	0.00	0.70	-21.51	0.00	0.71	21.39
43	43-	44	0.00	-0.71	-21.39	0.00	2.12	-0.47

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LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
 879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

No	Nodes		I-End			J-End		
	I	J	Axial (K)	Shear (K)	Moment (K-ft)	Axial (K)	Shear (K)	Moment (K-ft)
44	44-	45	0.00	0.27	0.47	0.00	-0.17	-0.22
45	45-	46	0.00	0.17	0.23	0.00	-0.12	-0.15
46	46-	47	0.00	0.12	0.15	0.00	0.13	-0.17
47	47-	48	0.00	-0.13	0.17	0.00	0.18	-0.20
48	48-	49	0.00	-0.18	0.26	0.00	0.28	-0.50
49	49-	50	0.00	2.14	0.50	0.00	-0.72	21.72
50	50-	51	0.00	0.72	-21.72	0.00	0.69	21.97
51	51-	52	0.00	-0.69	-21.97	0.00	2.11	0.22
52	52-	53	0.00	-2.11	-0.23	0.00	3.52	-43.48
53	53-	54	0.00	3.51	43.48	0.00	-2.10	-0.15
54	54-	55	0.00	2.10	0.11	0.00	-0.70	21.52
55	55-	56	0.00	0.70	-21.51	0.00	0.71	21.39
56	56-	57	0.00	-0.71	-21.39	0.00	2.12	-0.47
57	57-	58	0.00	0.27	0.47	0.00	-0.17	-0.22
58	58-	59	0.00	0.17	0.23	0.00	-0.12	-0.15
59	59-	60	0.00	0.12	0.15	0.00	0.13	-0.17
60	60-	61	0.00	-0.13	0.17	0.00	0.18	-0.20
61	61-	62	0.00	-0.18	0.26	0.00	0.28	-0.50
62	62-	63	0.00	2.14	0.50	0.00	-0.72	21.72
63	63-	64	0.00	0.72	-21.72	0.00	0.69	21.97
64	64-	65	0.00	-0.69	-21.97	0.00	2.11	0.22
65	65-	66	0.00	-2.11	-0.23	0.00	3.52	-43.48
66	66-	67	0.00	3.51	43.49	0.00	-2.10	-0.15
67	67-	68	0.00	2.10	0.11	0.00	-0.70	21.52
68	68-	69	0.00	0.70	-21.52	0.00	0.71	21.40
69	69-	70	0.00	-0.71	-21.40	0.00	2.12	-0.40
70	70-	71	0.00	0.26	0.46	0.00	-0.16	-0.22
71	71-	72	0.00	0.16	0.23	0.00	-0.11	-0.15
72	72-	73	0.00	0.11	0.15	0.00	0.13	-0.17
73	73-	74	0.00	-0.13	0.17	0.00	0.18	-0.20
74	74-	75	0.00	-0.18	0.27	0.00	0.28	-0.50
75	75-	76	0.00	2.15	0.52	0.00	-0.74	21.97
76	76-	77	0.00	0.74	-21.91	0.00	0.68	22.38
77	77-	78	0.00	-0.68	-22.35	0.00	2.09	0.80
78	78-	79	0.00	-2.09	-0.80	0.00	3.51	-42.72
79	79-	80	0.00	3.45	42.71	0.00	-2.04	-0.30
80	80-	81	0.00	2.04	0.30	0.00	-0.63	20.30
81	81-	82	0.00	0.63	-20.36	0.00	0.77	19.20
82	82-	83	0.00	-0.77	-19.28	0.00	2.18	-3.50
83	83-	84	0.00	1.29	3.54	0.00	-1.14	-1.50
84	84-	85	0.00	1.14	1.52	0.00	-1.08	-0.80
85	85-	86	0.00	1.08	0.87	0.00	-0.95	0.60
36	86-	87	0.00	0.95	-0.65	0.00	-0.89	1.10
87	87-	88	0.00	0.89	-1.19	0.00	-0.74	2.50
88	88-	89	0.00	0.92	-2.55	0.00	-0.22	6.90

LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
 879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

No	Nodes		I-End			J-End		
	I	J	Axial (K)	Shear (K)	Moment (K-ft)	Axial (K)	Shear (K)	Moment (K-ft)
89	89-	90	0.00	0.22	-6.95	0.00	0.47	5.99
90	90-	91	0.00	-0.47	-5.99	0.00	1.17	-0.32
91	91-	92	0.00	-1.17	0.32	0.00	1.87	-11.98

LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
 879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

Load Combination is 1 : DEAD LD + INSUL'N
 AISC Code Checks

No	Nodes		Maximum	0	Member Quarter Points			L	Shear
	I	J			1/4	1/2	3/4		
1	1-	2			-	Not Calculated	-		
2	2-	3			-	Not Calculated	-		
3	3-	4			-	Not Calculated	-		
4	4-	5			-	Not Calculated	-		
5	5-	6			-	Not Calculated	-		
6	6-	7			-	Not Calculated	-		
7	7-	8			-	Not Calculated	-		
8	8-	9			-	Not Calculated	-		
9	9-	10			-	Not Calculated	-		
10	10-	11			-	Not Calculated	-		
11	11-	12			-	Not Calculated	-		
12	12-	13			-	Not Calculated	-		
13	13-	14			-	Not Calculated	-		
14	14-	15			-	Not Calculated	-		
15	15-	16			-	Not Calculated	-		
16	16-	17			-	Not Calculated	-		
17	17-	18			-	Not Calculated	-		
18	18-	19			-	Not Calculated	-		
19	19-	20			-	Not Calculated	-		
20	20-	21			-	Not Calculated	-		
21	21-	22			-	Not Calculated	-		
22	22-	23			-	Not Calculated	-		
23	23-	24			-	Not Calculated	-		
24	24-	25			-	Not Calculated	-		
25	25-	26			-	Not Calculated	-		
26	26-	27			-	Not Calculated	-		
27	27-	28			-	Not Calculated	-		
28	28-	29			-	Not Calculated	-		
29	29-	30			-	Not Calculated	-		
30	30-	31			-	Not Calculated	-		
31	31-	32			-	Not Calculated	-		
32	32-	33			-	Not Calculated	-		
33	33-	34			-	Not Calculated	-		
34	34-	35			-	Not Calculated	-		
35	35-	36			-	Not Calculated	-		
36	36-	37			-	Not Calculated	-		
37	37-	38			-	Not Calculated	-		
38	38-	39			-	Not Calculated	-		
39	39-	40			-	Not Calculated	-		
40	40-	41			-	Not Calculated	-		
41	41-	42			-	Not Calculated	-		
42	42-	43			-	Not Calculated	-		
43	43-	44			-	Not Calculated	-		

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LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
 879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

No	Nodes		Maximum	0	Member Quarter Points			L	Shear
	I	J			1/4	1/2	3/4		
44	44-	45			-	Not Calculated	-		
45	45-	46			-	Not Calculated	-		
46	46-	47			-	Not Calculated	-		
47	47-	48			-	Not Calculated	-		
48	48-	49			-	Not Calculated	-		
49	49-	50			-	Not Calculated	-		
50	50-	51			-	Not Calculated	-		
51	51-	52			-	Not Calculated	-		
52	52-	53			-	Not Calculated	-		
53	53-	54			-	Not Calculated	-		
54	54-	55			-	Not Calculated	-		
55	55-	56			-	Not Calculated	-		
56	56-	57			-	Not Calculated	-		
57	57-	58			-	Not Calculated	-		
58	58-	59			-	Not Calculated	-		
59	59-	60			-	Not Calculated	-		
60	60-	61			-	Not Calculated	-		
61	61-	62			-	Not Calculated	-		
62	62-	63			-	Not Calculated	-		
63	63-	64			-	Not Calculated	-		
64	64-	65			-	Not Calculated	-		
65	65-	66			-	Not Calculated	-		
66	66-	67			-	Not Calculated	-		
67	67-	68			-	Not Calculated	-		
68	68-	69			-	Not Calculated	-		
69	69-	70			-	Not Calculated	-		
70	70-	71			-	Not Calculated	-		
71	71-	72			-	Not Calculated	-		
72	72-	73			-	Not Calculated	-		
73	73-	74			-	Not Calculated	-		
74	74-	75			-	Not Calculated	-		
75	75-	76			-	Not Calculated	-		
76	76-	77			-	Not Calculated	-		
77	77-	78			-	Not Calculated	-		
78	78-	79			-	Not Calculated	-		
79	79-	80			-	Not Calculated	-		
80	80-	81			-	Not Calculated	-		
81	81-	82			-	Not Calculated	-		
82	82-	83			-	Not Calculated	-		
83	83-	84			-	Not Calculated	-		
84	84-	85			-	Not Calculated	-		
85	85-	86			-	Not Calculated	-		
86	86-	87			-	Not Calculated	-		
87	87-	88			-	Not Calculated	-		
88	88-	89			-	Not Calculated	-		

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LIGO BEAM TUBE - BAKE + SELF WT, LIVINGS
 879' LENGTH, SIMPLE SUPPT - FIXED SUPPT

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No	Nodes		Maximum	0	Member Quarter Points			L	Shear
	I	J			1/4	1/2	3/4		
89	89-	90			- Not Calculated	-			
90	90-	91			- Not Calculated	-			
91	91-	92			- Not Calculated	-			

**RISA-2D MODEL OF LIGO BEAM TUBE MODULE: CASE #4 NODE NUMBERS
 MODEL FOR DETERMINATION OF NATURAL PERIOD
 CASE #4: RIGID SUPPORTS AT BOTH ENDS, TOTAL LENGTH = 130'**

NODE	X (FT)	DESCRIPTION
1	0.000	FIXED SUPPT, ROTATION FIXED
2	6.870	
3	13.740	
4	20.610	
5	27.480	
6	34.350	
7	41.220	
8	48.090	
9	54.960	
10	61.830	GUIDED SUPPT
11	62.920	END TUBE / START EJ STUB
12	63.500	END EJ STUB / START BELLOWS
13	66.170	END BELLOWS / START EJ STUB
14	66.750	END EJ STUB / START TUBE
15	67.830	GUIDED SUPPT
16	74.740	
17	81.650	
18	88.550	
19	95.460	
20	102.370	
21	109.280	
22	116.180	
23	123.090	
24	130.000	FIXED SUPPT, ROTATION FIXED

SUBJECT RISA-2D MODEL OF BEAM TUBE MODULE, CASE #4 NODE NUMBERS, COORDINATES & DESCRIPTION	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RJC	MADE BY	CHKD BY	SHT 1 OF 1
	DATE 3/9/94	DATE 3/11/94	DATE	DATE	3.5.128

LIGO BEAM TUBE -- NATURAL PERIOD MODEL
 130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

Node No	Boundary Conditions					
	X-Coord (ft)	Y-Coord (ft)	X-dof (in,K/in)	Y-dof (in,K/in)	Rotation (r,K-ft/r)	Temp (F)
1	0.00	0.00	R	R	R	0.00
2	6.87	0.00				0.00
3	13.74	0.00				0.00
4	20.61	0.00				0.00
5	27.48	0.00				0.00
6	34.35	0.00				0.00
7	41.22	0.00				0.00
8	48.09	0.00				0.00
9	54.96	0.00				0.00
10	61.83	0.00		R		0.00
11	62.92	0.00				0.00
12	63.50	0.00				0.00
13	66.17	0.00				0.00
14	66.75	0.00				0.00
15	67.83	0.00		R		0.00
16	74.74	0.00				0.00
17	81.65	0.00				0.00
18	88.55	0.00				0.00
19	95.46	0.00				0.00
20	102.37	0.00				0.00
21	109.28	0.00				0.00
22	116.18	0.00				0.00
23	123.09	0.00				0.00
24	130.00	0.00	R	R	R	0.00

SUBJECT	OFFICE		REVISION		REFERENCE NO.
	CBI				
	MADE BY	CHKD BY	MADE BY	CHKD BY	SHT ___ OF ___
	DATE	DATE	DATE	DATE	

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LIGO BEAM TUBE -- NATURAL PERIOD MODEL
130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

Material Label	Elastic Modulus (Ksi)	Poisson's Ratio	Thermal Coefficient (F)	Weight Density (K/ft3)	Yield Stress (Fy) (Ksi)
1	27000.00	0.30000	0.65000	0.000	19.200

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LIGO BEAM TUBE -- NATURAL PERIOD MODEL
130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

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Section Label	Database Shape	Matl. Set	Area (in ²)	Moment of Inertia (in ⁴)	As Coef	y/y
BEAMTUBE		1	19.19	5731.030	1.20	
STUBEND		1	16.13	4820.000	1.20	
EXPJOINT		1	0.01	1.803	1.20	

LIGO BEAM TUBE -- NATURAL PERIOD MODEL
 130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

No	I Node	J Node	Section	I Releases J						End Offsets		Length	
				x	y	z	x	y	z	Sec	Sway		I (in)
1	1	-	2										6.87
2	2	-	3										6.87
3	3	-	4										6.87
4	4	-	5										6.87
5	5	-	6										6.87
6	6	-	7										6.87
7	7	-	8										6.87
8	8	-	9										6.87
9	9	-	10										6.87
10	10	-	11										1.09
11	11	-	12										0.58
12	12	-	13										2.67
13	13	-	14										0.58
14	14	-	15										1.08
15	15	-	16										6.91
16	16	-	17										6.91
17	17	-	18										6.90
18	18	-	19										6.91
19	19	-	20										6.91
20	20	-	21										6.91
21	21	-	22										6.90
22	22	-	23										6.91
23	23	-	24										6.91

LIGO BEAM TUBE -- NATURAL PERIOD MODEL
 130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

No	I J		Unbraced Lengths			K Factors		Bending Coefs	
	Node	Node	Lb-in	Lb-out	Lc	In	Out	Cm	Cb
			(ft)	(ft)	(ft)				
1	1	-	2						
2	2	-	3						
3	3	-	4						
4	4	-	5						
5	5	-	6						
6	6	-	7						
7	7	-	8						
8	8	-	9						
9	9	-	10						
10	10	-	11						
11	11	-	12						
12	12	-	13						
13	13	-	14						
14	14	-	15						
15	15	-	16						
16	16	-	17						
17	17	-	18						
18	18	-	19						
19	19	-	20						
20	20	-	21						
21	21	-	22						
22	22	-	23						
23	23	-	24						

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LIGO BEAM TUBE -- NATURAL PERIOD MODEL
130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

BLC No.	Basic Load Case Description	Load Totals		
		Nodal	Point	Dist.
1	SELF WEIGHT + INSULATION			23

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LIGO BEAM TUBE -- NATURAL PERIOD MODEL
 130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

Member Distributed Loads, BLC 1: SELF WEIGHT + INSULATION

Member No	I Node	J Node	Dir	Start Magnitude (K/ft,F)	End Magnitude (K/ft,F)	Start Location (ft)	End Location (ft)
1	1	2	Y	-0.091	-0.091	0.000	6.870
2	2	3	Y	-0.091	-0.091	0.000	6.870
3	3	4	Y	-0.091	-0.091	0.000	6.869
4	4	5	Y	-0.091	-0.091	0.000	6.870
5	5	6	Y	-0.091	-0.091	0.000	6.869
6	6	7	Y	-0.091	-0.091	0.000	6.870
7	7	8	Y	-0.091	-0.091	0.000	6.869
8	8	9	Y	-0.091	-0.091	0.000	6.870
9	9	10	Y	-0.091	-0.091	0.000	6.869
10	10	11	Y	-0.091	-0.091	0.000	1.089
11	11	12	Y	-0.091	-0.091	0.000	0.580
12	12	13	Y	-0.091	-0.091	0.000	2.669
13	13	14	Y	-0.091	-0.091	0.000	0.580
14	14	15	Y	-0.091	-0.091	0.000	1.080
15	15	16	Y	-0.091	-0.091	0.000	6.909
16	16	17	Y	-0.091	-0.091	0.000	6.910
17	17	18	Y	-0.091	-0.091	0.000	6.900
18	18	19	Y	-0.091	-0.091	0.000	6.909
19	19	20	Y	-0.091	-0.091	0.000	6.910
20	20	21	Y	-0.091	-0.091	0.000	6.909
21	21	22	Y	-0.091	-0.091	0.000	6.900
22	22	23	Y	-0.091	-0.091	0.000	6.909
23	23	24	Y	-0.091	-0.091	0.000	6.910

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LIGO BEAM TUBE -- NATURAL PERIOD MODEL
130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

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Load Combination No.	Description	Self Wt Dir Fac	BLC Fac	BLC Fac	BLC Fac	BLC Fac	BLC Fac	DYNA	W S	E V
1	DEAD LD + INSUL'N		1	1						

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LIGO BEAM TUBE -- NATURAL PERIOD MODEL
 130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

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Dynamic Analysis Data

Number of modes (frequencies) : 12
 Basic Load Case for masses : 1
 BLC mass direction of action : Y only
 Acceleration of Gravity : 32.20 ft/sec**2

Load Combination is 1 : DEAD LD + INSUL'N
 Nodal Displacements

Node	Global X (in)	Global Y (in)	Rotation (rad)
1	0.00000	-0.00000	-0.00000
2	0.00000	-0.00938	-0.00020
3	0.00000	-0.03017	-0.00028
4	0.00000	-0.05310	-0.00026
5	0.00000	-0.07112	-0.00017
6	0.00000	-0.07947	-0.00003
7	0.00000	-0.07564	0.00012
8	0.00000	-0.05940	0.00027
9	0.00000	-0.03275	0.00037
10	0.00000	-0.00000	0.00041
11	0.00000	0.00536	0.00041
12	0.00000	0.00821	0.00041
13	0.00000	0.00830	-0.00042
14	0.00000	0.00540	-0.00042
15	0.00000	-0.00000	-0.00042
16	0.00000	-0.03348	-0.00038
17	0.00000	-0.06071	-0.00027
18	0.00000	-0.07730	-0.00013
19	0.00000	-0.08122	0.00003
20	0.00000	-0.07268	0.00017
21	0.00000	-0.05426	0.00026
22	0.00000	-0.03085	0.00029
23	0.00000	-0.00959	0.00021
24	0.00000	-0.00000	0.00000

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LIGO BEAM TUBE -- NATURAL PERIOD MODEL
 130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

Load Combination is 1 : DEAD LD + INSUL'N
 Spring Reactions

Node	Global X (K)	Global Y (K)	Moment (K-ft)
1	0.00000	3.50498	43.24687
10	0.00000	2.39166	0.00000
15	0.00000	2.40941	0.00000
24	0.00000	3.52396	-43.71775
Totals	0.00000	11.83000	-0.47088

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LIGO BEAM TUBE -- NATURAL PERIOD MODEL
 130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

Load Combination is 1 : DEAD LD + INSUL'N
 Member End Forces

No	Nodes		I-End			J-End		
	I	J	Axial (K)	Shear (K)	Moment (K-ft)	Axial (K)	Shear (K)	Moment (K-ft)
1	1-	2	0.00	3.50	43.25	0.00	-2.88	-21.32
2	2-	3	0.00	2.88	21.32	0.00	-2.25	-3.68
3	3-	4	0.00	2.25	3.68	0.00	-1.63	9.66
4	4-	5	0.00	1.63	-9.66	0.00	-1.00	18.71
5	5-	6	0.00	1.00	-18.71	0.00	-0.38	23.46
6	6-	7	0.00	0.38	-23.46	0.00	0.25	23.92
7	7-	8	0.00	-0.25	-23.92	0.00	0.87	20.08
8	8-	9	0.00	-0.87	-20.08	0.00	1.50	11.95
9	9-	10	0.00	-1.50	-11.95	0.00	2.12	-0.48
10	10-	11	0.00	0.27	0.48	0.00	-0.17	-0.24
11	11-	12	0.00	0.17	0.24	0.00	-0.12	-0.15
12	12-	13	0.00	0.12	0.15	0.00	0.12	-0.16
13	13-	14	0.00	-0.12	0.16	0.00	0.18	-0.25
14	14-	15	0.00	-0.18	0.25	0.00	0.28	-0.50
15	15-	16	0.00	2.13	0.50	0.00	-1.50	12.07
16	16-	17	0.00	1.50	-12.07	0.00	-0.88	20.30
17	17-	18	0.00	0.88	-20.30	0.00	-0.25	24.18
18	18-	19	0.00	0.25	-24.18	0.00	0.38	23.72
19	19-	20	0.00	-0.38	-23.72	0.00	1.01	18.91
20	20-	21	0.00	-1.01	-18.91	0.00	1.64	9.76
21	21-	22	0.00	-1.64	-9.76	0.00	2.27	-3.71
22	22-	23	0.00	-2.27	3.71	0.00	2.90	-21.54
23	23-	24	0.00	-2.90	21.54	0.00	3.52	-43.72

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LIGO BEAM TUBE -- NATURAL PERIOD MODEL
 130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

Load Combination is 1 : DEAD LD + INSUL'N
 AISC Code Checks

No	Nodes		Maximum	0	Member Quarter Points			L	Shear
	I	J			1/4	1/2	3/4		
1	1-	2			-	Not Calculated	-		
2	2-	3			-	Not Calculated	-		
3	3-	4			-	Not Calculated	-		
4	4-	5			-	Not Calculated	-		
5	5-	6			-	Not Calculated	-		
6	6-	7			-	Not Calculated	-		
7	7-	8			-	Not Calculated	-		
8	8-	9			-	Not Calculated	-		
9	9-	10			-	Not Calculated	-		
10	10-	11			-	Not Calculated	-		
11	11-	12			-	Not Calculated	-		
12	12-	13			-	Not Calculated	-		
13	13-	14			-	Not Calculated	-		
14	14-	15			-	Not Calculated	-		
15	15-	16			-	Not Calculated	-		
16	16-	17			-	Not Calculated	-		
17	17-	18			-	Not Calculated	-		
18	18-	19			-	Not Calculated	-		
19	19-	20			-	Not Calculated	-		
20	20-	21			-	Not Calculated	-		
21	21-	22			-	Not Calculated	-		
22	22-	23			-	Not Calculated	-		
23	23-	24			-	Not Calculated	-		

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LIGO BEAM TUBE -- NATURAL PERIOD MODEL
130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

Frequencies

Mode No.	Frequency (Hz)	Period (Sec)
1	12.43124	0.08044
2	12.64979	0.07905
3	40.13626	0.02492
4	40.60882	0.02463
5	83.44815	0.01198
6	84.37569	0.01185
7	141.78408	0.00705
8	143.31877	0.00698
9	213.26871	0.00469
10	215.61330	0.00464
11	293.23037	0.00341
12	296.47407	0.00337

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LIGO BEAM TUBE -- NATURAL PERIOD MODEL
130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

Mode Shape 1

Node	Global X	Global Y	Rotation
1	0.00000	0.00000	0.00000
2	0.00000	0.57763	0.01282
3	0.00000	1.92170	0.01864
4	0.00000	3.47250	0.01798
5	0.00000	4.74021	0.01202
6	0.00000	5.35744	0.00254
7	0.00000	5.12237	-0.00827
8	0.00000	4.01954	-0.01814
9	0.00000	2.21144	-0.02509
10	0.00000	0.00000	-0.02777
11	0.00000	-0.36346	-0.02780
12	0.00000	-0.55702	-0.02782
13	0.00000	-1.12756	0.05657
14	0.00000	-0.73376	0.05659
15	0.00000	0.00000	0.05665
16	0.00000	4.57149	0.05191
17	0.00000	8.35349	0.03794
18	0.00000	10.68467	0.01761
19	0.00000	11.20956	-0.00493
20	0.00000	9.94052	-0.02483
21	0.00000	7.29357	-0.03743
22	0.00000	4.04456	-0.03894
23	0.00000	1.21701	-0.02685
24	0.00000	0.00000	-0.00000

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LIGO BEAM TUBE -- NATURAL PERIOD MODEL
 130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

Mode Shape 2

Node	Global X	Global Y	Rotation
1	0.00000	-0.00000	-0.00000
2	0.00000	-1.22509	-0.02718
3	0.00000	-4.06935	-0.03937
4	0.00000	-7.33777	-0.03777
5	0.00000	-9.98857	-0.02494
6	0.00000	-11.24774	-0.00476
7	0.00000	-10.70232	0.01800
8	0.00000	-8.34450	0.03841
9	0.00000	-4.55138	0.05220
10	0.00000	-0.00000	0.05646
11	0.00000	0.73763	0.05633
12	0.00000	1.12949	0.05628
13	0.00000	-0.51982	0.02602
14	0.00000	-0.33858	0.02607
15	0.00000	0.00000	0.02619
16	0.00000	2.14082	0.02458
17	0.00000	3.94734	0.01829
18	0.00000	5.08243	0.00873
19	0.00000	5.35868	-0.00208
20	0.00000	4.76997	-0.01173
21	0.00000	3.50984	-0.01791
22	0.00000	1.95053	-0.01873
23	0.00000	0.58785	-0.01296
24	0.00000	0.00000	-0.00000

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LIGO BEAM TUBE -- NATURAL PERIOD MODEL
 130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

=====
 Mode Shape 3

Node	Global X	Global Y	Rotation
1	0.00000	-0.00000	-0.00000
2	0.00000	-0.65967	-0.01319
3	0.00000	-1.75287	-0.01101
4	0.00000	-2.20314	0.00108
5	0.00000	-1.54714	0.01416
6	0.00000	-0.07368	0.01975
7	0.00000	1.40277	0.01418
8	0.00000	2.04047	0.00046
9	0.00000	1.46824	-0.01363
10	0.00000	0.00000	-0.02017
11	0.00000	-0.26483	-0.02031
12	0.00000	-0.40637	-0.02036
13	0.00000	-2.15388	0.10805
14	0.00000	-1.40165	0.10811
15	0.00000	0.00000	0.10819
16	0.00000	8.10754	0.07669
17	0.00000	11.46377	-0.00019
18	0.00000	8.07413	-0.07737
19	0.00000	-0.13738	-0.11018
20	0.00000	-8.46396	-0.08025
21	0.00000	-12.25883	-0.00736
22	0.00000	-9.82154	0.06078
23	0.00000	-3.70977	0.07364
24	0.00000	-0.00000	0.00000

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LIGO BEAM TUBE -- NATURAL PERIOD MODEL
130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

Mode Shape 4

Node	Global X	Global Y	Rotation
1	0.00000	-0.00000	-0.00000
2	0.00000	-3.71935	-0.07426
3	0.00000	-9.84661	-0.06128
4	0.00000	-12.28895	0.00755
5	0.00000	-8.47481	0.08102
6	0.00000	-0.12191	0.11112
7	0.00000	8.10826	0.07795
8	0.00000	11.49993	-0.00001
9	0.00000	8.12221	-0.07741
10	0.00000	0.00000	-0.10885
11	0.00000	-1.42279	-0.10871
12	0.00000	-2.17910	-0.10862
13	0.00000	0.36679	-0.01830
14	0.00000	0.23919	-0.01837
15	0.00000	-0.00000	-0.01855
16	0.00000	-1.42780	-0.01387
17	0.00000	-2.05788	-0.00043
18	0.00000	-1.48557	0.01354
19	0.00000	-0.02710	0.01976
20	0.00000	1.47737	0.01463
21	0.00000	2.18014	0.00158
22	0.00000	1.75922	-0.01078
23	0.00000	0.66698	-0.01322
24	0.00000	0.00000	-0.00000

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LIGO BEAM TUBE -- NATURAL PERIOD MODEL
 130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

Mode Shape 5

Node	Global X	Global Y	Rotation
1	0.00000	-0.00000	-0.00000
2	0.00000	-0.59273	-0.01011
3	0.00000	-1.12096	-0.00036
4	0.00000	-0.56747	0.01249
5	0.00000	0.56994	0.01195
6	0.00000	1.03629	-0.00194
7	0.00000	0.31436	-0.01350
8	0.00000	-0.76304	-0.00954
9	0.00000	-0.95222	0.00556
10	0.00000	-0.00000	0.01505
11	0.00000	0.19866	0.01530
12	0.00000	0.30547	0.01539
13	0.00000	3.15350	-0.15831
14	0.00000	2.05147	-0.15835
15	0.00000	-0.00000	-0.15817
16	0.00000	-10.53105	-0.06588
17	0.00000	-8.84953	0.10150
18	0.00000	3.03702	0.15148
19	0.00000	11.41685	0.02646
20	0.00000	6.55323	-0.13012
21	0.00000	-6.09813	-0.13966
22	0.00000	-12.41794	0.00214
23	0.00000	-6.61926	0.11178
24	0.00000	-0.00000	0.00000

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LIGO BEAM TUBE -- NATURAL PERIOD MODEL
 130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

Mode Shape 6

Node	Global X	Global Y	Rotation
1	0.00000	0.00000	0.00000
2	0.00000	6.63494	0.11272
3	0.00000	12.45184	0.00224
4	0.00000	6.10865	-0.14083
5	0.00000	-6.57122	-0.13115
6	0.00000	-11.44514	0.02666
7	0.00000	-3.04814	0.15270
8	0.00000	8.88171	0.10222
9	0.00000	10.55691	-0.06651
10	0.00000	0.00000	-0.15939
11	0.00000	-2.08624	-0.15955
12	0.00000	-3.19652	-0.15948
13	0.00000	0.26393	-0.01310
14	0.00000	0.17241	-0.01320
15	0.00000	-0.00000	-0.01342
16	0.00000	-0.93900	-0.00631
17	0.00000	-0.82758	0.00874
18	0.00000	0.23190	0.01380
19	0.00000	1.01589	0.00283
20	0.00000	0.60830	-0.01150
21	0.00000	-0.52864	-0.01270
22	0.00000	-1.11234	0.00003
23	0.00000	-0.59756	0.01005
24	0.00000	-0.00000	0.00000

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LIGO BEAM TUBE -- NATURAL PERIOD MODEL
 130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

Mode Shape 7

Node	Global X	Global Y	Rotation
1	0.00000	0.00000	0.00000
2	0.00000	0.50134	0.00655
3	0.00000	0.54213	-0.00646
4	0.00000	-0.29271	-0.00960
5	0.00000	-0.57620	0.00416
6	0.00000	0.18495	0.01033
7	0.00000	0.61454	-0.00214
8	0.00000	-0.06063	-0.01070
9	0.00000	-0.61264	0.00028
10	0.00000	-0.00000	0.01180
11	0.00000	0.15706	0.01218
12	0.00000	0.24228	0.01230
13	0.00000	4.10133	-0.20628
14	0.00000	2.66569	-0.20619
15	0.00000	-0.00000	-0.20490
16	0.00000	-11.54311	-0.01463
17	0.00000	-1.81479	0.19775
18	0.00000	11.26454	0.04951
19	0.00000	3.87908	-0.18884
20	0.00000	-10.54197	-0.08309
21	0.00000	-5.67308	0.17559
22	0.00000	9.93379	0.12246
23	0.00000	9.36175	-0.12047
24	0.00000	0.00000	-0.00000

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LIGO BEAM TUBE -- NATURAL PERIOD MODEL
 130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

Mode Shape 7

Node	Global X	Global Y	Rotation
1	0.00000	0.00000	0.00000
2	0.00000	0.50134	0.00655
3	0.00000	0.54213	-0.00646
4	0.00000	-0.29271	-0.00960
5	0.00000	-0.57620	0.00416
6	0.00000	0.18495	0.01033
7	0.00000	0.61454	-0.00214
8	0.00000	-0.06063	-0.01070
9	0.00000	-0.61264	0.00028
10	0.00000	-0.00000	0.01180
11	0.00000	0.15706	0.01218
12	0.00000	0.24228	0.01230
13	0.00000	4.10133	-0.20628
14	0.00000	2.66569	-0.20619
15	0.00000	-0.00000	-0.20490
16	0.00000	-11.54311	-0.01463
17	0.00000	-1.81479	0.19775
18	0.00000	11.26454	0.04951
19	0.00000	3.87908	-0.18884
20	0.00000	-10.54197	-0.08309
21	0.00000	-5.67308	0.17559
22	0.00000	9.93379	0.12246
23	0.00000	9.36175	-0.12047
24	0.00000	0.00000	-0.00000

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LIGO BEAM TUBE -- NATURAL PERIOD MODEL
 130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

Mode Shape 8

Node	Global X	Global Y	Rotation
1	0.00000	-0.00000	-0.00000
2	0.00000	-9.38431	-0.12153
3	0.00000	-9.96984	0.12333
4	0.00000	5.69539	0.17696
5	0.00000	10.56776	-0.08380
6	0.00000	-3.88538	-0.19039
7	0.00000	-11.29517	0.04975
8	0.00000	1.82622	0.19922
9	0.00000	11.56402	-0.01480
10	0.00000	0.00000	-0.20650
11	0.00000	-2.71166	-0.20784
12	0.00000	-4.15874	-0.20792
13	0.00000	0.19944	-0.00984
14	0.00000	0.13055	-0.00996
15	0.00000	-0.00000	-0.01020
16	0.00000	-0.62340	-0.00130
17	0.00000	-0.13407	0.01055
18	0.00000	0.59353	0.00317
19	0.00000	0.23093	-0.00999
20	0.00000	-0.55497	-0.00478
21	0.00000	-0.31656	0.00930
22	0.00000	0.52484	0.00672
23	0.00000	0.50367	-0.00643
24	0.00000	0.00000	-0.00000

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LIGO BEAM TUBE -- NATURAL PERIOD MODEL
130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

Mode Shape 9

Node	Global X	Global Y	Rotation
1	0.00000	0.00000	0.00000
2	0.00000	0.37991	0.00293
3	0.00000	0.11234	-0.00763
4	0.00000	-0.38546	-0.00027
5	0.00000	0.08190	0.00759
6	0.00000	0.34716	-0.00343
7	0.00000	-0.24967	-0.00594
8	0.00000	-0.22810	0.00626
9	0.00000	0.35018	0.00271
10	0.00000	0.00000	-0.00881
11	0.00000	-0.11900	-0.00932
12	0.00000	-0.18451	-0.00948
13	0.00000	-4.88557	0.24663
14	0.00000	-3.16986	0.24619
15	0.00000	0.00000	0.24221
16	0.00000	10.99622	-0.06635
17	0.00000	-6.09366	-0.19725
18	0.00000	-8.02079	0.16747
19	0.00000	10.11072	0.11322
20	0.00000	2.92311	-0.22441
21	0.00000	-11.55024	0.00075
22	0.00000	3.10960	0.22937
23	0.00000	11.44308	-0.08547
24	0.00000	0.00000	-0.00000

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LIGO BEAM TUBE -- NATURAL PERIOD MODEL
 130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

Mode Shape 10

Node	Global X	Global Y	Rotation
1	0.00000	0.00000	0.00000
2	0.00000	11.46020	0.08622
3	0.00000	3.12774	-0.23106
4	0.00000	-11.58367	-0.00055
5	0.00000	2.93808	0.22634
6	0.00000	10.14329	-0.11418
7	0.00000	-8.03873	-0.16907
8	0.00000	-6.11189	0.19892
9	0.00000	11.02250	0.06701
10	0.00000	0.00000	-0.24457
11	0.00000	-3.23173	-0.24875
12	0.00000	-4.96537	-0.24921
13	0.00000	0.14096	-0.00689
14	0.00000	0.09255	-0.00702
15	0.00000	-0.00000	-0.00727
16	0.00000	-0.37779	0.00170
17	0.00000	0.17592	0.00685
18	0.00000	0.28214	-0.00520
19	0.00000	-0.32399	-0.00411
20	0.00000	-0.11241	0.00735
21	0.00000	0.38181	0.00022
22	0.00000	-0.09459	-0.00765
23	0.00000	-0.38007	0.00277
24	0.00000	-0.00000	0.00000

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LIGO BEAM TUBE -- NATURAL PERIOD MODEL
 130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

Mode Shape 11

Node	Global X	Global Y	Rotation
1	0.00000	-0.00000	-0.00000
2	0.00000	-0.25078	-0.00022
3	0.00000	0.09889	0.00448
4	0.00000	0.12064	-0.00420
5	0.00000	-0.23419	0.00020
6	0.00000	0.13669	0.00399
7	0.00000	0.08368	-0.00459
8	0.00000	-0.22949	0.00104
9	0.00000	0.16272	0.00330
10	0.00000	-0.00000	-0.00588
11	0.00000	-0.08124	-0.00647
12	0.00000	-0.12698	-0.00665
13	0.00000	-5.23401	0.26578
14	0.00000	-3.38645	0.26471
15	0.00000	0.00000	0.25628
16	0.00000	8.99656	-0.14467
17	0.00000	-10.95015	-0.07377
18	0.00000	3.17646	0.23055
19	0.00000	7.38280	-0.18422
20	0.00000	-11.47176	-0.02364
21	0.00000	5.49324	0.21043
22	0.00000	5.18858	-0.21563
23	0.00000	-12.34592	0.00698
24	0.00000	-0.00000	0.00000

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LIGO BEAM TUBE -- NATURAL PERIOD MODEL
130' LENGTH, FIXED SUPPORTS @ BOTH ENDS

Mode Shape 12

Node	Global X	Global Y	Rotation
1	0.00000	-0.00000	-0.00000
2	0.00000	-12.37033	-0.00724
3	0.00000	5.18207	0.21753
4	0.00000	5.51670	-0.21197
5	0.00000	-11.49780	0.02336
6	0.00000	7.37389	0.18609
7	0.00000	3.21788	-0.23224
8	0.00000	-10.99832	0.07428
9	0.00000	9.02324	0.14636
10	0.00000	0.00000	-0.25951
11	0.00000	-3.46391	-0.26843
12	0.00000	-5.33760	-0.26955
13	0.00000	0.08469	-0.00406
14	0.00000	0.05598	-0.00420
15	0.00000	-0.00000	-0.00445
16	0.00000	-0.19864	0.00254
17	0.00000	0.21137	0.00193
18	0.00000	-0.04489	-0.00471
19	0.00000	-0.16037	0.00347
20	0.00000	0.22861	0.00075
21	0.00000	-0.10133	-0.00431
22	0.00000	-0.11030	0.00425
23	0.00000	0.24794	-0.00007
24	0.00000	0.00000	-0.00000

A-2D

F1:Full Plot
F2:Options
F5:Axis Off
F7:Animate
F9:Unanchor
F10:Anchor/
View
Ctrl:Drift
Esc:Return

Solution:
Mode 1



A-2D

F1:Full Plot
F2:Options
F5:Ann Off
F7:Animate
F9:UnAnchor
F10:Anchor/
View
Ctrl/D:Drift
Esc:Return

Solution:
Mode 2



A-2D

F1:Full Plot

F2:Options

F5:Box Off

F7:Animate

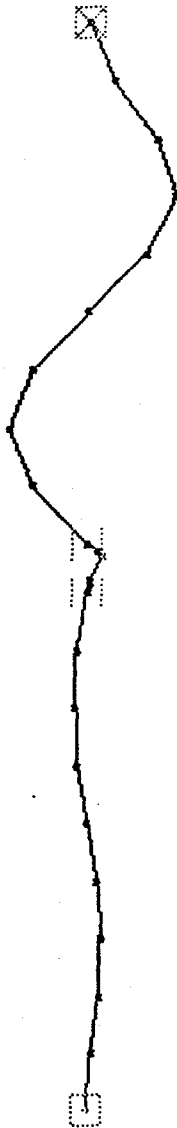
F9:UnAnchor

F10:Anchor/
View

Ctrl/D:Drift

Esc:Return

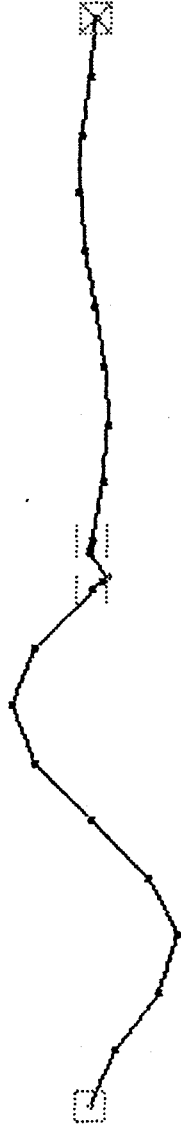
Solution:
Mode 3



A-2D

F1:Full Plot
F2:Options
F5:Ann Off
F7:Animate
F9:Unanchor
F10:Anchor/
View
Ctrl:Drift
Esc:Return

Solution:
Mode 4



A-2B

F1:Full Plot
F2:Options
F5:Bar Off
F7:Animate
F9:Unanchor
F10:Anchor/
View
Ctrl-D:Drift
Esc:Return

Solution:
Mode 5

y
x



A-20

F1:Full Plot
F2:Options
F5:Box Off
F7:Annotate
F9:Unanchor
F10:Anchor/
View
Ctrl/D:Drift
Esc:Return

Solution:
Mode 7



A-2D

F1:Full Plot
F2:Options
F5:Box Off
F7:Animate
F9:UnAnchor
F10:Anchor/
View
Ctrl-D:Drift
Esc:Return

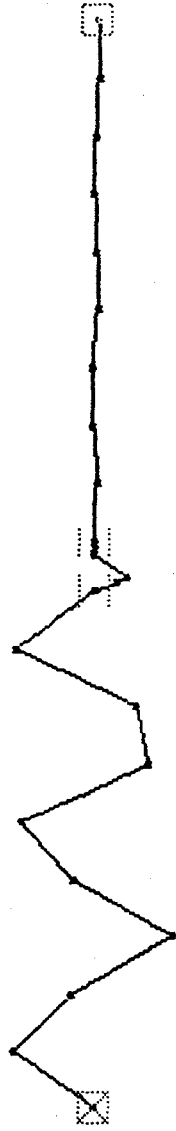
Solution:
Mode 9



A-2D

F1:Full Plot
F2:Options
F5:Axis Off
F7:Animate
F9:Unanchor
F10:Anchor/
View
Ctrl D:Drift
Esc:Return

Solution:
Mode 10



A-2D

F1:Full Plot
F2:Options
F5:Box Off
F7:Animate

F9:In/Out
F10:In/Out
View
Ctrl:Drift
Esc:Return

Solution:
Mode 11



A-2D

F1:Full Plot
F2:Options
F5:Box Off
F7:Animate
F9:Unanchor
F10:Anchor/
View
Ctrl:D:Drift
Esc:Return

Solution:
Mode 12



INTRODUCTION

THE BEAM TUBE SECTIONS ARE DESIGNED PER ASME SECTION VIII DIV 2. RISAD IS USED TO DETERMINE REACTIONS AND MOMENTS. THE REACTIONS AND MOMENTS ARE SUPERIMPOSED WITH THE OTHER LOAD CASES. THE FOLLOWING LOAD CASES WILL BE INVESTIGATED

CASE:

- 1 DEAD LOAD
- 2 DEAD LOAD + INSULATION (WET)
- 3 SNOW
- 4 WIND
- 5 SEISMIC - HORIZONTAL
- 6 SEISMIC - AXIAL
- 7 VACUUM
- 8 DIFFERENTIAL SETTLEMENT OF SUPPORTS
- 9 BAKE OUT @ 302°
- 10 MAXIMUM OPERATING TEMPERATURE OF 100°F
- 11 MINIMUM OPERATING TEMPERATURE OF -15°F

14 COMBINATIONS OF THE CASES WERE INVESTIGATED. THE COMBINATIONS ARE LISTED ON SUMMARY SHEETS WHICH ARE AT THE END OF EACH CALCULATION SET.

SUBJECT BEAM TUBE DESIGN STRESSES & REACTIONS	OFFICE CBI WOR.C		REVISION		REFERENCE NO. 980312
	MADE BY KIH	CHKD BY WJC	MADE BY	CHKD BY	SHT 1 OF 4
	DATE 12/20/93	DATE 4.4.94	DATE	DATE	6.1

CASE 3 LIVINGSTON

(NORMAL) $R_{FIX} = 7.029 \text{ k}$

$R_{AX} = 6.95 \text{ k}$

TYPICAL $R_{SUPPORT} = \begin{matrix} 2.39 \\ 2.42 \end{matrix} \} = 4.81$

BND $R_{SUPPORT} = \begin{matrix} 3.468 \\ 0.180 \end{matrix} \} = 3.648$

$M_{MAX} = 43.66$

$M_{TYP} = 43.44$

$\theta_{55-96} = 0.00035 + .0007 = .00042 \text{ RAD} = 0.024^\circ$

$\theta_{72-73} = 0.00044 - 0.00043 = 0.00001 = 0.048^\circ$

F.P. $\theta_{59-60} = 0.00041 + .00042 = 0.00083 = 0.048^\circ$

$\Delta_{25} = 0.079 \text{ ''}$

$\Delta_{29} = 0.076$

$\Delta_{34} = 0.079$

$\Delta_{42} = 0.076$

SUBJECT RISA2D Results Used for Beam Tube Design	OFFICE CBI 1158-C		REVISION		REFERENCE NO. 930212
	MADE BY RJA	CHKD BY WIC	MADE BY	CHKD BY	SHT 3 OF 4
	DATE 2 MAR 94	DATE 4-4-94	DATE	DATE	6.3

CASES WITH DIFFERENTIAL SETTLEMENT

CASE 1A - SETTLEMENT @ FIXED SUPPORT

RISA 2D RUN WITH 1" DIFFERENTIAL SETTLEMENT DOWN

NOTE: DIFFERENTIAL SETTLEMENT MAY BE UP - SIGNS REVERSE

$$\begin{array}{l}
 \text{NODE 40 FIXED SUPPORT} = -2.37 \text{ k} \\
 \left. \begin{array}{l} 31 \\ 36 \end{array} \right\} \text{GUIDE SUPPORT} \left\{ \begin{array}{l} = -1.25 \\ = 2.45 \end{array} \right\} = 1.2 \text{ k} \\
 \left. \begin{array}{l} 44 \\ 49 \end{array} \right\} \text{GUIDE SUPPORT} \left\{ \begin{array}{l} = 2.47 \\ = -1.26 \end{array} \right\} = 1.21 \text{ k}
 \end{array}$$

$$\left. \begin{array}{l} \theta_{33} = 0.00007 \\ \theta_{34} = -0.00196 \end{array} \right\} = 0.00203 = 0.116^\circ$$

$$M_{\text{MAX}} @ 40 = 71.36 \text{ k}$$

CASE 1B - SETTLEMENT @ GUIDED SUPPORT

$$\begin{array}{l}
 \text{NODE 31 } \left. \begin{array}{l} \\ 36 \end{array} \right\} \text{GUIDED} \left\{ \begin{array}{l} = -0.560 \text{ k} \\ = -0.537 \end{array} \right\} = -1.10 \text{ k} \\
 27 \text{ FIXED} = 1.23 \\
 40 \text{ FIXED} = 1.23
 \end{array}$$

$$\left. \begin{array}{l} \theta_{33} = 0.00167 \\ \theta_{34} = -0.00167 \end{array} \right\} = 0.00334 \text{ RAD} = 0.191^\circ$$

$$M_{\text{MAX}} @ 27 = 35.79 \text{ k-ft}$$

CASE 1A CONTROLS FOR SUPPORT REACTIONS & MAXIMUM MOMENTS

CASE 1B CONTROLS FOR EXCESSIVE SUPPORT REACTIONS

RUN BEAM TUBE DESIGN W/ CASE 1A FOR DIFFERENTIAL SETTLEMENT TO YIELD HIGHEST STRESSES & REACTIONS

SUBJECT RISA 2D RESULTS USED FOR BEAM TUBE DESIGN	OFFICE CBI 100-1		REVISION		REFERENCE NO. 930212
	MADE BY R311	CHKD BY WIK	MADE BY	CHKD BY	SHT 4 OF 4
	DATE 7 MAR 94	DATE 4-4-94	DATE	DATE	6.4

- INPUT VARIABLES

Tube outside diameter, Do = 49.004 in
 Beam Tube section length, L = 19.812 m = 65.000 ft
 Beam Tube Span length, Lsp = 18.9484 m = 62.167 ft
 Tube thickness, t = 0.127 in
 Insulation Density, Deni = 24 kg/m = 16.127 lbs/ft

Vacuum Stiffener thickness, ts = 0.1875 in
 Vacuum Stiffener width, ws = 1.75 in
 Vacuum Stiffener spacing, Ls = 29.84 in
 Support Stiffener thickness, tss = 0.375 in
 Support Stiffener width, wss = 4 in

Mod. of Elast. @ ambient, Ea = 28,300 ksi Table TM-1 @ 70
 Mod. of Elast. @ 302 degrees, Eb = 27,000 ksi Table TM-1, Page 664
 Coefficient of expansion, e = 9E-06 in/in/F Average from 70 to 300 degrees F

Anchor bolt spacing, Abs = 30 in
 C. line height of tube, H = 42.000 in
 Support Collar / Saddle width, b = 20.000 in

Expansion Joint O.D., De = 53.75 in
 Expansion Joint I.D., Dei = 48.75 in
 E.J. Concentricity Error, CE = 0.1875 in
 Expansion joint axial spring rate, Kej = 9147.6 lbs/in = 8316 * 1.1
 Spring rate variation, Eej = 10.0%
 Spring Rate, K = 10062 lbs/in = Kej * (1 + Eej)

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 1 OF 29
	DATE 3/11/94	DATE 4-4-94	DATE	DATE	6.5

- TUBE WEIGHT & PROPERTIES

Inside Diameter, Di = 48.75 in
 Area, A = $19.501 \text{ in}^2 = \text{PI}() * (\text{Do}^2 - \text{Di}^2) / 4$
 Section Modulus, S = $237.7 \text{ in}^3 = \text{PI}() * (\text{Do}^4 - \text{Di}^4) / 32 / \text{Do}$
 Moment of inertia, I = $5823 \text{ in}^4 = \text{PI}() * (\text{Do}^4 - \text{Di}^4) / 64$
 Radius of gyration, rg = $17.281 \text{ in} = (I / A)^{0.5}$

Theoretical # of vacuum stiffeners = 27 = Round(L/Ls*12)
 Number of stiffeners used, Ns = 25
 True spacing = 31.200 in = L/(Ns)*12

Number of support stiffeners, Nss = 1.5 Per section

Shell weight per section = 4357 lbs = $495 * A / 144 * L$

Weight per vacuum stiffener = $14.987 \text{ lbs} = \text{PI}() * ((\text{Do} + 2 * \text{ws})^2 - \text{Do}^2) / 4 * \text{Ts} * 495 / 12^3$
 Weight per support stiffener = $71.550 \text{ lbs} = \text{PI}() * ((\text{Do} + 2 * \text{wss})^2 - \text{Do}^2) / 4 * \text{Tss} * 495 / 12^3$
 Stiffener weight per section = $482.01 \text{ lbs} = \text{Weight Vacuum} * \text{Ns} + \text{Weight support} * \text{Nss}$
 Estimated Baffle wt / section = 27.5 lbs

Total metal weight, DL = 4867 lbs, or
 wd = 74.87 lbs/ft

Insulation weight per section = 1048 lbs = $\text{Deni} * L$

DL + Insulation = 5915 lbs, or
 wdi = 91.00 lbs/ft

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 2 OF 29
	DATE 3/11/94	DATE 4-4-94	DATE	DATE	6.6

- ALLOWABLE STRESS PER ASME SECTION VIII DIV 1 UG 23(b)

- Allowable Stresses @ 300 Degrees F.

Yield Stress, $F_y = \frac{19,200}{}$ psi Table Y1
 Tensile Allowable, $S_h = \frac{13000}{}$ psi Table 1A
 Joint Efficiency, $E_t = \frac{0.7}{}$
 Compression Allowable UG 23(b)
 $A = 0.000648 = 0.125 / (Ro/t)$
 $B = \frac{5900}{}$ psi per Figure HA 3, interpolate to 300 degrees F.
 Where $B = F_a = F_{bx} = F_{by} = F_{bxy}$, local buckling stress
 See below for column buckling

- Allowable Stresses @ Ambient (100 degrees F)

Yield Stress = $\frac{25,000}{}$ psi Table Y1
 Tensile Allowable, $S_a = \frac{16,300}{}$ psi Table 1A
 Compression Allowable UG 23(b)
 $A = 0.000648 = 0.125 / (Ro/t)$
 $B = \frac{7800}{}$ psi per Figure HA 3, 100 degrees F.
 Where $B = F_a = F_{bx} = F_{by} = F_{bxy}$

- Allowable Stress Increase for Wind and Seismic
 Allowable increase for wind or seismic is 1.20

- Allowable Axial Stress, Column Buckling per AISC

$$k = \frac{1}{}$$

$$L = \frac{746}{}$$
 in = $L_{sp} * 12$

$$r = \frac{17.28069}{}$$
 in = r_g

$$kL / r = 43.16958$$

$$C_c = 166.6081 = (2 * \pi)^2 * E_b * 1000 / F_y^{0.5}$$

$$F_a = 10533 \text{ psi} = \frac{(1 - (kL/r)^2 / 2 / C_c^2) * F_y}{(5/3 + 3 * (kL/r) / 8 / C_c - (kL/r)^3 / 8 / C_c^3)}$$

$$F_a > B, \text{ Thus use } B = 5900 \text{ psi}$$

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 3 OF 29
	DATE 3/11/94	DATE 4-4-94	DATE	DATE	6.7

- CALCULATE WIND LOAD PER ASCE 7-88 (Livingston, LA)

$$F = Q_z * G_h * C_f * A_f$$

$$Q_z = 4.1472 = 0.00256 * K_z * (I * V)^2$$

$$K_z = \frac{0.80}{1.00}, \text{ Assume exposure C}$$

$$I = 1.00$$

$$V = 45 \text{ mph}$$

$$G_h = 1.32$$

$$C_f = 0.739 \quad h / D = 0.0853$$

$$h = (D_o / 2 + H) / 12$$

$$D = L$$

$$D' / D_o = w_s / D_o = 0.03571$$

$$A_f = 4.084 \text{ Sqft / ft} = D_o / 12$$

$$F = 16.53 \text{ lbs / ft}$$

$$F = 1074 \text{ lbs / section}$$

Table 4

Eq 3

Table 6

Table 5

Figure 1

Table 8

Table 12

- CALCULATE WIND LOAD PER ASCE 7-88 (Hanford, WA)

$$F = Q_z * G_h * C_f * A_f$$

$$Q_z = 4.1472 = 0.00256 * K_z * (I * V)^2$$

$$K_z = \frac{0.80}{1.00}, \text{ Assume exposure C}$$

$$I = 1.00$$

$$V = 45 \text{ mph}$$

$$G_h = 1.32$$

$$C_f = 0.739 \quad h / D = 0.0853$$

$$h = (D_o / 2 + H) / 12$$

$$D = L$$

$$D' / D_o = w_s / D_o = 0.03571$$

$$A_f = 4.083667 \text{ Sqft / ft} = D_o / 12$$

$$F = 16.53 \text{ lbs / ft} = ww$$

$$F = 1074 \text{ lbs / section}$$

Table 4

Eq 3

Table 6

Table 5

Figure 1

Table 8

Table 12

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJK	MADE BY	CHKD BY	SHT 4 OF 29
	DATE 3/11/94	DATE 4-4-94.	DATE	DATE	6.8

- CALCULATE SEISMIC LOAD PER UBC (Livingston, LA)

$$V = Z * I * C / R_w * W$$

$$Z = \frac{0}{}$$

$$I = \frac{1.0}{}$$

$$C = \frac{2.75}{}$$

$$R_w = \frac{3}{}$$

$$C/R_w = 0.916667 > 0.075 \text{ use: } 0.91667$$

$$W = 91.00 \text{ lbs/ft} = \text{DL} + \text{Insulation}$$

$$V = 0.0500W = 0.05W \text{ minimum per ASCE 7-88, 9.11.2}$$

$$V = 4.55 \text{ Lbs/ft}$$

$$V = 295.8 \text{ Lbs per section}$$

- CALCULATE SEISMIC LOAD PER ASCE 7-88 (Hanford, WA)

$$V = Z * I * C / R_w * W$$

$$Z = \frac{0.2}{}$$

$$I = \frac{1.0}{}$$

$$C = \frac{2.75}{}$$

$$R_w = \frac{3}{}$$

$$C/R_w = 0.916667 > 0.075 \text{ use: } 0.91667$$

$$W = 91.00 \text{ lbs/ft} = \text{DL} + \text{Insulation}$$

$$V = 0.1833W$$

$$V = 16.68 \text{ Lbs/ft} = \text{wseis}$$

$$V = 1084.4 \text{ Lbs per section}$$

- CALCULATE SNOW LOAD PER ASCE 7-88 (Hanford, WA only)

$$P_f = 15.12 = 0.7 * C_e * C_t * I * P_g$$

Eq 5a

$$C_e = \frac{0.9}{}$$

Little shelter assumed

Table 18

$$C_t = \frac{1.2}{}$$

Unheated

Table 19

$$I = \frac{1}{}$$

Category 1

Table 20

$$P_g = \frac{20 \text{ psf}}{}$$

Figure 6

$$P_s = 9.828 = C_s * P_f$$

$$C_s = \frac{0.65}{}$$

Angle is 35 degrees, $90 - (180 - 70) / 2$, Figure 8b

$$\text{Width} = 46.049 \text{ in} = D_o * \sin 70 \text{ Per 7.4.3, slope} > 70 \text{ no load}$$

$$\text{Snow Load per foot, } w_{sn} = 37.71 \text{ lbs/ft} = P_s * \text{Width}$$

$$\text{Snow load per section} = 2451 \text{ lbs}$$

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 5 OF 29
	DATE 3/11/94	DATE 4-4-94	DATE	DATE	6.9

- DETERMINE REACTIONS, STRESSES AND DEFLECTIONS FOR INDIVIDUAL LOAD CASES

- Nomenclature

- Rfx1 = Reaction, fixed support, x direction (horizontal), case 1
- Rgy2 = Reaction, guided support, y direction (vertical), case 2
- Rfz1 = Reaction, fixed support, z direction (axial), case 1
- Mx1 = Moment about the horizontal axis due to vertical loads, Case 1.
- My2 = Moment about the vertical axis due to horizontal loads, Case 2.
- fa9c = Stress, axial, case 9, compression, (if tension t is used instead of c)

- Reactions Based on RISA2D

Fixed support = 7.029 kips, per RISA2D
 1/2 of guided support = 2.41 kips, per RISA2D
 Total, two spans = 11.849 kips, per RISA2D

% Fixed support, Kf = 0.593215 (Percentage of 2 spans)
 % Guided support, Kg = 0.406785

- Moments based on RISA2D

Maximum Moment = 43480 lb-ft, at fixed support
 Based on, 91 lbs/ft, DL + Insulation, used in RISA2D

True DL + Insulation = wdi = 91.00
 Estimated Moment = 43961 lb-ft = wdi * (Lsp ft)^2 / 8

Moment correction factor, Km = 0.989 = RISA2D moment / Estimated moment

- Deflections based on RISA2D

Midspan Deflection = 0.079 in
 Based on, 91 lbs/ft, DL + Insulation, used in RISA2D

True DL + Insulation = wdi = 91.00
 Estimated Deflection = 0.081 in = wdi * Lsp^4 / 185 / Eb / I * 12^3 / 1000

Moment correction factor, Kd = 0.978 = RISA2D deflection / Estimated deflection

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 6 OF 29
	DATE 3/11/94	DATE 4-4-94	DATE	DATE	6.10

- CASE 1: Dead Load Reactions, Stresses and Deflections

FIXED SUPPORT

Rfx1 = 0
 Rfy1 = 5774 lbs = $K_f * 2 * L * wd$
 Rfz1 = 0

GUIDED SUPPORT

Rgx1 = 0
 Rgy1 = 3959 lbs = $K_g * 2 * L * wd$

MOMENTS & BENDING STRESS

Mx1 = 35774 lb-ft = $wd * (L_{sp})^2 / 8 * K_m$
 fbx1 = 1806 psi = $Mx1 * 12 / S$

My1 = 0
 fby1 = 0 psi = $My1 * 12 / S$

MIDSPAN DEFLECTION

Dely1 (amb)= 0.062 in = $wd * L_{sp}^4 / 185 / E_a / I * 12^3 / 1000 * K_d$

- CASE 2: Dead Load plus Insulation Reaction, Stresses and Deflections

FIXED SUPPORT

Rfx2 = 0
 Rfy2 = 7018 lbs = $K_f * 2 * L * wdi$
 Rfz2 = 0

GUIDED SUPPORT

Rgx2 = 0
 Rgy2 = 4812 lbs = $K_g * 2 * L * wdi$

MOMENTS & BENDING STRESS

Mx2 = 43480 lb-ft = $wdi * (L_{sp})^2 / 8 * K_m$
 fbx2 = 2195 psi = $Mx2 * 12 / S$

My2 = 0
 fby2 = 0 psi = $My2 * 12 / S$

MIDSPAN DEFLECTION

Dely2 (302)= 0.079 in = $wdi * L_{sp}^4 / 185 / E_b / I * 12^3 / 1000 * K_d$

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 7 OF 29
	DATE 3/11/94	DATE 4.4.94	DATE	DATE	6.11

- CASE 3: Snow Load Reaction, Stresses and Deflections (Hanford, WA)

FIXED SUPPORT

Rfx3 = 0
 Rfy3 = 2908 lbs = $K_f * 2 * L * w_{sn}$
 Rfz3 = 0

GUIDED SUPPORT

Rgx3 = 0
 Rgy3 = 1994 lbs = $K_g * 2 * L * w_{sn}$

MOMENTS & BENDING STRESS

Mx3 = 18020 lb-ft = $w_{sn} * (L_{sp})^2 / 8 * K_m$
 fbx3 = 910 psi = $M_{x3} * 12 / S$

My3 = 0
 fby3 = 0 psi = $M_{y3} * 12 / S$

MIDSPAN DEFLECTION

Dely3 (amb) = 0.031 in = $w_{sn} * L_{sp}^4 / 185 / E_a / I * 12^3 / 1000 * K_d$

- CASE 4: Wind Load Reaction, Stresses and Deflections (Livingston, LA)

FIXED SUPPORT

Rfx4 = 1275 lbs = $w_w * 2 * L * K_f$
 Rfy4 = 0
 Rfz4 = 0

GUIDED SUPPORT

Rgx4 = 874 lbs = $w_w * 2 * L * K_g$
 Rgy4 = 0

MOMENTS & BENDING STRESS

Mx4 = 0
 fbx4 = 0 psi = $M_{x4} * 12 / S$

My4 = 7896 lb-ft = $w_w * (L_{sp})^2 / 8 * K_m$
 fby4 = 399 psi = $M_{y4} * 12 / S$

MIDSPAN DEFLECTION

Delx4 (amb) = 0.014 in = $w_w * L_{sp}^4 / 185 / E_a / I * 12^3 / 1000 * K_d$

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 9 OF 29
	DATE 3/11/94	DATE 4.4.94	DATE	DATE	6.12

- DETERMINE REACTIONS, STRESSES AND DEFLECTIONS FOR INDIVIDUAL LOAD CASES

- Nomenclature

- Rfx1 = Reaction, fixed support, x direction (horizontal), case 1
- Rgy2 = Reaction, guided support, y direction (vertical), case 2
- Rfz1 = Reaction, fixed support, z direction (axial), case 1
- Mx1 = Moment about the horizontal axis due to vertical loads, Case 1.
- My2 = Moment about the vertical axis due to horizontal loads, Case 2.
- fa9c = Stress, axial, case 9, compression, (if tension t is used instead of c)

- Reactions Based on RISA2D

Fixed support = $\frac{7.029}{1}$ kips, per RISA2D
 1/2 of guided support = $\frac{2.41}{1}$ kips, per RISA2D
 Total, two spans = 11.849 kips, per RISA2D

% Fixed support, Kf = 0.593215 (Percentage of 2 spans)
 % Guided support, Kg = 0.406785

- Moments based on RISA2D

Maximum Moment = $\frac{43480}{1}$ lb-ft, at fixed support
 Based on, $\frac{91}{1}$ lbs/ft, DL + Insulation, used in RISA2D

True DL + Insulation = wdi = 91.00
 Estimated Moment = 43961 lb-ft = wdi * (Lsp ft)^2 / 8

Moment correction factor, Km = 0.989 = RISA2D moment / Estimated moment

- Deflections based on RISA2D

Midspan Deflection = $\frac{0.079}{1}$ in
 Based on, $\frac{91}{1}$ lbs/ft, DL + Insulation, used in RISA2D

True DL + Insulation = wdi = 91.00
 Estimated Deflection = 0.081 in = wdi * Lsp^4 / 185 / Eb / I * 12^3 / 1000

Moment correction factor, Kd = 0.978 = RISA2D deflection / Estimated deflection

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 6 OF 29
	DATE 3/11/94	DATE 4.4.94	DATE	DATE	6.10

- CASE 1: Dead Load Reactions, Stresses and Deflections

FIXED SUPPORT

Rfx1 = 0
 Rfy1 = 5774 lbs = $K_f * 2 * L * wd$
 Rfz1 = 0

GUIDED SUPPORT

Rgx1 = 0
 Rgy1 = 3959 lbs = $K_g * 2 * L * wd$

MOMENTS & BENDING STRESS

Mx1 = 35774 lb-ft = $wd * (Lsp)^2 / 8 * Km$
 fbx1 = 1806 psi = $Mx1 * 12 / S$

My1 = 0
 fby1 = 0 psi = $My1 * 12 / S$

MIDSPAN DEFLECTION

Dely1 (amb) = 0.062 in = $wd * Lsp^4 / 185 / Ea / I * 12^3 / 1000 * Kd$

- CASE 2: Dead Load plus Insulation Reaction, Stresses and Deflections

FIXED SUPPORT

Rfx2 = 0
 Rfy2 = 7018 lbs = $K_f * 2 * L * wdi$
 Rfz2 = 0

GUIDED SUPPORT

Rgx2 = 0
 Rgy2 = 4812 lbs = $K_g * 2 * L * wdi$

MOMENTS & BENDING STRESS

Mx2 = 43480 lb-ft = $wdi * (Lsp)^2 / 8 * Km$
 fbx2 = 2195 psi = $Mx2 * 12 / S$

My2 = 0
 fby2 = 0 psi = $My2 * 12 / S$

MIDSPAN DEFLECTION

Dely2 (302) = 0.079 in = $wdi * Lsp^4 / 185 / Eb / I * 12^3 / 1000 * Kd$

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 7 OF 29
	DATE 3/11/94	DATE 4.4.94	DATE	DATE	6.11

- CASE 3: Snow Load Reaction, Stresses and Deflections (Hanford, WA)

FIXED SUPPORT

Rfx3 = 0
 Rfy3 = 2908 lbs = $K_f * 2 * L * w_{sn}$
 Rfz3 = 0

GUIDED SUPPORT

Rgx3 = 0
 Rgy3 = 1994 lbs = $K_g * 2 * L * w_{sn}$

MOMENTS & BENDING STRESS

Mx3 = 18020 lb-ft = $w_{sn} * (L_{sp})^2 / 8 * K_m$
 fbx3 = 910 psi = $M_{x3} * 12 / S$

My3 = 0
 fby3 = 0 psi = $M_{y3} * 12 / S$

MIDSPAN DEFLECTION

Dely3 (amb)= 0.031 in = $w_{sn} * L_{sp}^4 / 185 / E_a / I * 12^3 / 1000 * K_d$

- CASE 4: Wind Load Reaction, Stresses and Deflections (Livingston, LA)

FIXED SUPPORT

Rfx4 = 1275 lbs = $w_w * 2 * L * K_f$
 Rfy4 = 0
 Rfz4 = 0

GUIDED SUPPORT

Rgx4 = 874 lbs = $w_w * 2 * L * K_g$
 Rgy4 = 0

MOMENTS & BENDING STRESS

Mx4 = 0
 fbx4 = 0 psi = $M_{x4} * 12 / S$

My4 = 7896 lb-ft = $w_w * (L_{sp})^2 / 8 * K_m$
 fby4 = 399 psi = $M_{y4} * 12 / S$

MIDSPAN DEFLECTION

Delx4 (amb)= 0.014 in = $w_w * L_{sp}^4 / 185 / E_a / I * 12^3 / 1000 * K_d$

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 9 OF 29
	DATE 3/11/94	DATE 4-4-94	DATE	DATE	6.12

- CASE 5: Seismic Reaction, Stresses and Deflections (Hanford, WA), X Direction (Horizontal)

FIXED SUPPORT

Rfx5 = 1287 lbs = wseis * 2 * L * Kf
 Rfy5 = 0
 Rfz5 = 0

GUIDED SUPPORT

Rgx5 = 882 lbs = wseis * 2 * L * Kg
 Rgy5 = 0

MOMENTS & BENDING STRESS

Mx5 = 0
 fbx5 = 0 psi = Mx5 * 12 / S

My5 = 7971 lb-ft = wseis * (Lsp)^2 / 8 * Km
 fby5 = 402 psi = My5 * 12 / S

MIDSPAN DEFLECTION

Delx5 (amb)= 0.014 in = wseis * Lsp^4 / 185 / Ea / I * 12^3 / 1000 * Kd

- CASE 6: Seismic Reaction, Stresses and Deflections (Hanford, WA), Z Direction (Axial)

FIXED SUPPORT

Rfx6 = 0
 Rfy6 = 0
 Rfz6 = 2169 lbs = wseis * 2 * L

GUIDED SUPPORT

Rgx6 = 0
 Rgy6 = 0

MOMENTS & BENDING STRESS

Mx6 = 0
 fbx6 = 0 psi = Mx5 * 12 / S

My6 = 0
 fby6 = 0 psi = My6 * 12 / S

MIDSPAN DEFLECTION

Dely6 (amb)= 0.000 in

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 9 OF 29
	DATE 3/11/94	DATE 4.4.94	DATE	DATE	6.13

- CASE 7: Vacuum

Bellows effective area = 2062.9 sq in = $PI() * ((De+Dei)/2)^2 / 4$
 Tube pressure area = 1866.55 sq in = $PI() * (Di)^2 / 4$
 Bellows pressure area = 196.35 sq in = Bellows effective - tube area
 Axial force, Pzp = 2886 lbs = 14.7 * Bellows pressure area, pos. = tension

Axial Stress due to vacuum, fav7 = 148.0 psi = Pzp / A, pos. = tension

FIXED SUPPORT

Rfx7 = 0 lbs
 Rfy7 = 0
 Rfz7 = 0

GUIDED SUPPORT

Rgx7 = 0 lbs
 Rgy7 = 0
 fbx7 = fby7 = 0
 Dely7 = 0

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 10 OF 29
	DATE 3/11/94	DATE 4-4-94	DATE	DATE	6.14

- CASE 8: Differential Settlement of Fixed Support

- Reactions Based on RISA2D with 1" differential settlement

Fixed support = -2.37 kips, per RISA2D
 1/2 of guided support = 2.47 kips, per RISA2D
 Based on, 1 lbs/ft, DL, used in RISA2D

FIXED SUPPORTS

Rfx8 = -2447 lbs = (RISA2D fixed) - DL * 2 * L * Kf

Rfy8 = -2447 lbs = (RISA2D fixed) - DL * 2 * L * Kf

GUIDED SUPPORTS

Rgx8 = 4887 lbs = 2 * (RISA2D guided) - DL * 2 * L * Kg

Rgy8 = 4887 lbs = 2 * (RISA2D guided) - DL * 2 * L * Kg

- Moments based on RISA2D with 1" differential settlement

Maximum Moment = 71360 lb-ft, at fixed support
 Based on, 1 lbs/ft, DL + Insulation, used in RISA2D

Mx8 = My8 = 70882 lb-ft = (RISA2D Moment) - DL * Lsp² / 8 * Km

fbx8 = fby8 = 3579 psi = Mx8 * 12 / S

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT " OF 29
	DATE 3/11/94	DATE 4-4-94	DATE	DATE	6.15

- CASE 9: Expansion Joint Forces at 302 Degrees F.

Maximum Bellows spring rate, K = 10062 lbs/in = $Kej * (1 + Eej)$
 Est. temp. change above 70, Tch = 232 degrees Fahrenheit
 Thermal growth hot, x = 3.257 in = $e * 2 * L * 12 * Tch$
 Longitudinal Comp. force, Pbc = 32776 lbs = $K * x$
 fa9c = -1681 psi = Pbc / A , stress on tube, neg is comp

FIXED SUPPORT

Rfx9 = 0 lbs
 Rfy9 = 0
 Rfz9 = 5959 lbs = $2 * Eej * Kej * x$

GUIDED SUPPORT

Rgx9 = 0
 Rgy9 = 167.8 lbs = $Rfz9 * H / Lsp / 12 / 2$
 fbx9 = fby9 = 0
 Delx9 = Dely9 = 0

- CASE 10: Expansion Joint Forces at 100 Degrees F.

Maximum Bellows spring rate, K = 10062 lbs/in = $Kej * (1 + Eej)$
 Est. temp. change above 70, Tch = 30 degrees Fahrenheit
 Thermal growth hot, xw = 0.421 in = $e * 2 * L * 12 * Tch$
 Longitudinal Comp. force, Pbc = 4238 lbs = $K * xw$
 fa10c = -217 psi = Pbc / A , stress on tube, neg is comp

FIXED SUPPORT

Rfx10 = 0 lbs
 Rfy10 = 0
 Rfz10 = 771 lbs = $2 * Eej * Kej * xw$

GUIDED SUPPORT

Rgx10 = 0
 Rgy10 = 21.7 lbs = $Rfz10 * H / Lsp / 12 / 2$
 fbx10 = fby10 = 0
 Delx10 = Dely10 = 0

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WIK	MADE BY	CHKD BY	SHT 12 OF 29
	DATE 3/11/94	DATE 4-4-94	DATE	DATE	6.16

- CASE 11: Expansion Joint Forces at -16 Degrees F.

Maximum Bellows spring rate, K = 10062 lbs/in = $K_{ej} * (1 + E_{ej})$
 Est. temp. change below 70, Tchc = 86 degrees Fahrenheit
 Thermal shrinkage, xs = $-1.207 \text{ in} = e * 2 * L * 12 * Tchc$
 Longitudinal Tension force, Pbt = 12150 lbs = $K * xs$
 fa11t = 623 psi = $-Pbt / A$ Positive is tension

FIXED SUPPORT

Rfx11 = 0 lbs
 Rfy11 = 0
 Rfz11 = 2209 lbs = $-2 * E_{ej} * K_{ej} * xs$

GUIDED SUPPORT

Rgx11 = 0
 Rgy11 = 62.2 lbs = $Rfz11 * H / L_{sp} / 12 / 2$
 fbx11 = fby11 = 0
 Delx11 = Dely11 = 0

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 13 OF 29
	DATE 3/11/94	DATE 4-4-94	DATE	DATE	6.17

- LOAD COMBINATIONS FOR MAXIMUM REACTIONS, STRESS AND DEFLECTIONS
COMBINATION 1 - DL + Insulation + Vacuum + 302 F (Case 2 + 7 + 9)

FIXED SUPPORTS

Rx = 0 lbs = Rfx2 + Rfx7 + Rfx9 (lateral)
 Ry = 7018 lbs = Rfy2 + Rfy7 + Rfy9 (Vertical)
 Rz = 5959 lbs = Rfz2 + Rfz7 + Rfz9 (axial)
 Rmax per bolt = 3509 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 3509 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 0 lbs = Rgx2 + Rgx7 + Rgx9
 Ry = 4980 lbs = Rgy2 + Rgy7 + Rgy9 (Vertical)
 Rmax per bolt = 2490 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 2490 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.000 in = Delx2 + Delx7 + Delx9
 Delta y (302) = 0.079 in = Dely2 + Dely7 + Dely9
 Max Delta = 0.079 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fac = -1533 psi, fav7 + fa9c , neg = compression
 fbx = 2195 psi = fbx2 + fbx7 + fbx9
 fby = 0 psi = fby2 + fby7 + fby9
 Md = 7966 in-lbs = fac * A * (Max Delta + CE)
 fd = 33.5 psi = Md / S

COMBINED STRESS (compression)

fat/Fa + fbx/Fbx + fby/Fby + fd/Fbxy <= 1.00

Where B=Fa=Fbx=Fby=Fbxy= 5900 psi for compression from page 3

fac / Fa = 0.260
 fbx / Fbx = 0.372
 fby / Fby = 0.000
 fd / Fbxy = 0.006
 SUM = 0.638 < 1.00

Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 14 OF 29
	DATE 3/11/94	DATE 4-4-94	DATE	DATE	6.18

COMBINATION 2 - DL + Insulation + Settlement + Vacuum + 302 F (Case 2+7+8+9)

SETTLEMENT WITH COMBINATION

Delta x, x8 = 0 in Maxset = 0.579 = (x8^2 + y8^2)^0.5
 Delta y, y8 = 0.579 in

FIXED SUPPORTS

Rx = 0 lbs = Rfx2 + Rfx7 + x8 * Rfx8 + Rfx9
 Ry = 5601 lbs = Rfy2 + Rfy7 + y8 * Rfy8 + Rfy9
 Rz = 5959 lbs = Rfz2 + Rfz7 + Rfz9
 Rmax per bolt = 2800 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 2800 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 0 lbs = Rgx2 + Rgx7 + x8 * Rgx8 + Rgx9
 Ry = 7810 lbs = Rgy2 + Rgy7 + y8 * Rgy8 + Rgy9
 Rmax per bolt = 3905 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 3905 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.000 in = 0.5 * x8
 Delta y (302) = 0.369 in = Dely2 + 0.5 * y8
 Max Delta = 0.369 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fac = -1533 psi, fav7 + fa9c , neg = compression
 fbx = 4267 psi = fbx2 + fbx7 + y8 * fbx8 + fbx9
 fby = 0 psi = fby2 + fby7 + x8 * fby8 + fby9
 Md = 22910 in-lbs = fac * A * (Max(Max Delta or maxset) + CE)
 fd = 96.4 psi = Md / S

COMBINED STRESS (compression)

fat/Fa + fbx/Fbx + fby/Fby + fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 5900 psi for compression from page 3

fac / Fa = 0.260
 fbx / Fbx = 0.723
 fby / Fby = 0.000
 fd / Fbxy = 0.016
 SUM = 0.999 < 1.00

Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 15 OF 29
	DATE 3/11/94	DATE 4.4.94	DATE	DATE	6.19

COMBINATION 3 - DL + Insul + Seis x + Dif settle + Vac + 302 F (Case 2+5+7+8+9)

SETTLEMENT WITH COMBINATION

Delta x, x8 = 0 in Maxset = 0.788 = (x8^2 + y8^2)^0.5
 Delta y, y8 = 0.788 in

FIXED SUPPORTS

Rx = 1287 lbs = Rfx2 + Rfx5 + Rfx7 + x8 * Rfx8 + Rfx9
 Ry = 5089 lbs = Rfy2 + Rfy5 + Rfy7 + y8 * Rfy8 + Rfy9
 Rz = 5959 lbs = Rfz2 + Rfz5 + Rfz7 + Rfz9
 Rmax per bolt = 4346 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 743 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 882 lbs = Rgx2 + Rgx5 + Rgx7 + x8 * Rgx8 + Rgx9
 Ry = 8831 lbs = Rgy2 + Rgy5 + Rgy7 + y8 * Rgy8 + Rgy9
 Rmax per bolt = 5651 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 3180 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.014 in = Delx5 + 0.5 * x8
 Delta y (302) = 0.473 in = Dely2 + 0.5 * y8
 Max Delta = 0.473 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fac = -1533 psi, fav7 + fa9c , neg = compression
 fbx = 5015 psi = fbx2 + fbx5 + fbx7 + y8 * fbx8 + fbx9
 fby = 402 psi = fby2 + fby5 + fby7 + x8 * fby8 + fby9
 Md = 29157 in-lbs = fac * A * (Max(Max Delta or maxset) + CE)
 fd = 122.7 psi = Md / S

COMBINED STRESS (compression)

fat/Fa + fbx/Fbx + fby/Fby + fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 7080 psi = 1.2 * B from page 3

fac / Fa = 0.216
 fbx / Fbx = 0.708
 fby / Fby = 0.057
 fd / Fbxy = 0.017
 SUM = 0.999 < 1.00 Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 16 OF 29
	DATE 3/11/94	DATE 4.4.94	DATE	DATE :	6.20

COMBINATION 4 - DL + Insul + Seis z + Dif settle + Vac + 302 F (Case 2+5+7+8+9)

SETTLEMENT WITH COMBINATION

Delta x, x8 = 0 in Maxset = 0.756 = (x8^2 + y8^2)^0.5
 Delta y, y8 = 0.756 in

FIXED SUPPORTS

Rx = 0 lbs = Rfx2 + Rfx6 + Rfx7 + x8 * Rfx8 + Rfx9
 Ry = 5168 lbs = Rfy2 + Rfy6 + Rfy7 + y8 * Rfy8 + Rfy9
 Rz = 8128 lbs = Rfz2 + Rfz6 + Rfz7 + Rfz9
 Rmax per bolt = 2584 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 2584 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 0 lbs = Rgx2 + Rgx6 + Rgx7 + x8 * Rgx8 + Rgx9
 Ry = 8675 lbs = Rgy2 + Rgy6 + Rgy7 + y8 * Rgy8 + Rgy9
 Rmax per bolt = 4337 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 4337 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.000 in = 0.5 * x8
 Delta y (302) = 0.457 in = Dely27 + 0.5 * y8
 Max Delta = 0.457 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fac = -1644 psi, fav7 + fa9c - Rfz6 / A , neg = compression
 fbx = 4901 psi = fbx2 + fbx5 + fbx7 + y8 * fbx8 + fbx9
 fby = 402 psi = fby2 + fby5 + fby7 + x8 * fby8 + fby9
 Md = 30247 in-lbs = fac * A * (Max(Max Delta or maxset) + CE)
 fd = 127.3 psi = Md / S

COMBINED STRESS (compression)

fat/Fa + fbx/Fbx + fby/Fby + fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 7080 psi = 1.2 * B from page 3

fac / Fa = 0.232
 fbx / Fbx = 0.692
 fby / Fby = 0.057
 fd / Fbxy = 0.018
 SUM = 0.999 < 1.00 Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 17 OF 29
	DATE 3/11/94	DATE 4.4.94	DATE	DATE	6.21

COMBINATION 5 - DL + Wind + Dif Set + 100 F (Case 1 + 4 + 8 + 10)

SETTLEMENT WITH COMBINATION

Delta x, x8 = 0 in Maxset = 0.965 = (x8^2 + y8^2)^0.5
 Delta y, y8 = 0.965 in

FIXED SUPPORTS

Rx = 1275 lbs = Rfx1 + Rfx4 + x8 * Rfx8 + Rfx10
 Ry = 3413 lbs = Rfy1 + Rfy4 + y8 * Rfy8 + Rfy10
 Rz = 771 lbs = Rfz1 + Rfz4 + Rfz10
 Rmax per bolt = 3491 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = -78 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 874 lbs = Rgx1 + Rgx4 + x8 * Rgx8 + Rgx10
 Ry = 8697 lbs = Rgy1 + Rgy4 + y8 * Rgy8 + Rgy10
 Rmax per bolt = 5572 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 3125 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.014 in = Delx4 + 0.5 * x8
 Delta y (302) = 0.545 in = Dely1 + 0.5 * y8
 Max Delta = 0.545 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fac = -217 psi, fav4 + fa10c , neg = compression
 fbx = 5260 psi = fbx1 + fbx4 + y8 * fbx8 + fbx10
 fby = 399 psi = fby1 + fby4 + x8 * fby8 + fby10

 Md = 4885 in-lbs = fac * A * (Max(Max Delta or maxset) + CE)
 fd = 20.6 psi = Md / S

COMBINED STRESS (compression)

fat/Fa + fbx/Fbx + fby/Fby + fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 5900 psi for compression from page 3

fac / Fa = 0.037
 fbx / Fbx = 0.891
 fby / Fby = 0.068
 fd / Fbxy = 0.003
 SUM = 0.999 < 1.00 Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 19 OF 29
	DATE 3/11/94	DATE 4-4-94	DATE	DATE	6.22

COMBINATION 6 - DL + Insulation + Vacuum + 100 F (Case 2 + 7 + 10)

FIXED SUPPORTS

Rx = 0 lbs = Rfx2 + Rfx7 + Rfx10 (lateral)
 Ry = 7018 lbs = Rfy2 + Rfy7 + Rfy10 (Vertical)
 Rz = 771 lbs = Rfz2 + Rfz7 + Rfz10 (axial)
 Rmax per bolt = 3509 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 3509 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 0 lbs = Rgx2 + Rgx7 + Rgx10
 Ry = 4834 lbs = Rgy2 + Rgy7 + Rgy10 (Vertical)
 Rmax per bolt = 2417 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 2417 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.000 in = Delx2 + Delx7 + Delx10
 Delta y (302) = 0.079 in = Dely2 + Dely7 + Dely10
 Max Delta = 0.079 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fac = -69 psi, fav7 + fa10c , neg = compression
 fbx = 2195 psi = fbx2 + fbx7 + fbx10
 fby = 0 psi = fby2 + fby7 + fby10

 Md = 360 in-lbs = fac * A * (Max Delta + CE)
 fd = 1.5 psi = Md / S

COMBINED STRESS (compression)

fat/Fa + fbx/Fbx + fby/Fby + fd/Fbxy <= 1.00

Where B=Fa=Fbx=Fby=Fbxy= 5900 psi for compression from page 3

fac / Fa = 0.012
 fbx / Fbx = 0.372
 fby / Fby = 0.000
 fd / Fbxy = 0.000

SUM = 0.384 < 1.00

Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 13 OF 29
	DATE 3/11/94	DATE 4-4-94	DATE	DATE	6.23

COMBINATION 7 - DL + Insulation + Settlement + Vacuum + 100 F (Case 2+7+8+10)

SETTLEMENT WITH COMBINATION

Delta x, x8 = 0 in Maxset = 1.012 = (x8^2 + y8^2)^0.5
 Delta y, y8= 1.012 in

FIXED SUPPORTS

Rx = 0 lbs = Rfx2 + Rfx7 + x8 * Rfx8 + Rfx10
 Ry = 4541 lbs = Rfy2 + Rfy7 + y8 * Rfy8 + Rfy10
 Rz = 771 lbs = Rfz2 + Rfz7 + Rfz10
 Rmax per bolt = 2271 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 2271 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 0 lbs = Rgx2 + Rgx7 + x8 * Rgx8 + Rgx10
 Ry = 9780 lbs = Rgy2 + Rgy7 + y8 * Rgy8 + Rgy10
 Rmax per bolt = 4890 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 4890 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.000 in = 0.5 * x8
 Delta y (302) = 0.585 in = Dely2 + 0.5 * y8
 Max Delta = 0.585 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fac = -69 psi, fav7 + fa10c , neg = compression
 fbx = 5817 psi = fbx2 + fbx7 + y8 * fbx8 + fbx10
 fby = 0 psi = fby2 + fby7 + x8 * fby8 + fby10

 Md = 1622 in-lbs = fac * A * (Max(Max Delta or maxset) + CE)
 fd = 6.8 psi = Md / S

COMBINED STRESS (compression)

fat/Fa + fbx/Fbx + fby/Fby +fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 5900 psi for compression from page 3

fac / Fa = 0.012
 fbx / Fbx = 0.986
 fby / Fby = 0.000
 fd / Fbxy = 0.001
 SUM = 0.999 < 1.00

Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 20 OF 29
	DATE 3/11/94	DATE 4-4-94	DATE	DATE	6.24

COMBINATION 8 - DL + Insul + Seis x + Dif settle + Vac + 100 F (Case 2+5+7+8+10)

SETTLEMENT WITH COMBINATION

Delta x, x8 = 0 in Maxset = 1.229 = (x8^2 + y8^2)^0.5
 Delta y, y8 = 1.229 in

FIXED SUPPORTS

Rx = 1287 lbs = Rfx2 + Rfx5 + Rfx7 + x8 * Rfx8 + Rfx10
 Ry = 4010 lbs = Rfy2 + Rfy5 + Rfy7 + y8 * Rfy8 + Rfy10
 Rz = 771 lbs = Rfz2 + Rfz5 + Rfz7 + Rfz10
 Rmax per bolt = 3806 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 204 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 882 lbs = Rgx2 + Rgx5 + Rgx7 + x8 * Rgx8 + Rgx10
 Ry = 10840 lbs = Rgy2 + Rgy5 + Rgy7 + y8 * Rgy8 + Rgy10
 Rmax per bolt = 6655 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 4185 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.014 in = Delx5 + 0.5 * x8
 Delta y (302) = 0.694 in = Dely2 + 0.5 * y8
 Max Delta = 0.694 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fac = -69 psi, fav7 + fa10c , neg = compression
 fbx = 6594 psi = fbx2 + fbx5 + fbx7 + y8 * fbx8 + fbx10
 fby = 402 psi = fby2 + fby5 + fby7 + x8 * fby8 + fby10

 Md = 1915 in-lbs = fac * A * (Max(Max Delta or maxset) + CE)
 fd = 8.1 psi = Md / S

COMBINED STRESS (compression)

fat/Fa + fbx/Fbx + fby/Fby + fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 7080 psi = 1.2 * B from page 3

fac / Fa = 0.010
 fbx / Fbx = 0.931
 fby / Fby = 0.057
 fd / Fbxy = 0.001
 SUM = 0.999 < 1.00

Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 21 OF 29
	DATE 3/11/94	DATE 4-4-94	DATE	DATE	. 6.25

COMBINATION 9 - DL + Insul + Seis z + Dif settle + Vac + 100 F (Case 2+5+7+8+10)

SETTLEMENT WITH COMBINATION

Delta x, x8 = 0 in Maxset = 1.195 = (x8^2 + y8^2)^0.5
 Delta y, y8= 1.195 in

FIXED SUPPORTS

Rx = 0 lbs = Rfx2 + Rfx6 + Rfx7 + x8 * Rfx8 + Rfx10
 Ry = 4093 lbs = Rfy2 + Rfy6 + Rfy7 + y8 * Rfy8 + Rfy10
 Rz = 2939 lbs = Rfz2 + Rfz6 + Rfz7 + Rfz10
 Rmax per bolt = 2047 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 2047 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 0 lbs = Rgx2 + Rgx6 + Rgx7 + x8*Rgx8 + Rgx10
 Ry = 10674 lbs = Rgy2 + Rgy6 + Rgy7 + y8*Rgy8 + Rgy10
 Rmax per bolt = 5337 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 5337 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.000 in = 0.5 * x8
 Delta y (302) = 0.677 in = Dely27 + 0.5 * y8
 Max Delta = 0.677 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fac = -181 psi, fav7 + fa10c - Rfz6 / A , neg = compression
 fbx = 6472 psi = fbx2 + fbx5 + fbx7 + y8 * fbx8 + fbx10
 fby = 402 psi = fby2 + fby5 + fby7 + x8 * fby8 + fby10
 Md = 4867 in-lbs = fac * A * (Max(Max Delta or maxset) + CE)
 fd = 20.5 psi = Md / S

COMBINED STRESS (compression)

fat/Fa + fbx/Fbx + fby/Fby +fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 7080 psi = 1.2 * B from page 3

fac / Fa = 0.026
 fbx / Fbx = 0.914
 fby / Fby = 0.057
 fd / Fbxy = 0.003
 SUM = 0.999 < 1.00

Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 22 OF 29
	DATE 3/11/94	DATE 4-4-94	DATE	DATE	6.26

COMBINATION 10 - DL + snow + Dif Set + -16 F (Case 1 + 3 + 8 + 11)

SETTLEMENT WITH COMBINATION

Delta x, x8 = 0 in Maxset = 1.044 = (x8^2 + y8^2)^0.5
 Delta y, y8 = 1.044 in

FIXED SUPPORTS

Rx = 0 lbs = Rfx1 + Rfx3 + x8 * Rfx8 + Rfx11
 Ry = 6128 lbs = Rfy1 + Rfy3 + y8 * Rfy8 + Rfy11
 Rz = 2209 lbs = Rfz1 + Rfz3 + Rfz11
 Rmax per bolt = 3064 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 3064 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 0 lbs = Rgx1 + Rgx3 + x8 * Rgx8 + Rgx11
 Ry = 11118 lbs = Rgy1 + Rgy3 + y8 * Rgy8 + Rgy11
 Rmax per bolt = 5559 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 5559 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.000 in = 0.5 * x8
 Delta y (302) = 0.615 in = Dely1 + Dely3 + 0.5 * y8
 Max Delta = 0.615 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fat = 623 psi, fa11t, pos = tension
 fbx = 6452 psi = fbx1 + fbx3 + y8 * fbx8 + fbx11
 fby = 0 psi = fby1 + fby3 + x8 * fby8 + fby11
 Md = 14962 in-lbs = fac * A * (Max(Max Delta or maxset) + CE)
 fd = 63.0 psi = Md / S

COMBINED STRESS

fat/Fa + fbx/Fbx + fby/Fby + fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 5900 psi for compression from page 3
 Tension Allowable = 9100 psi = Sh * Et, for tension from page 3

	<u>COMPRESSION</u>	<u>TENSION</u>	
fa(c or t) / Fa =	-0.106	0.068	
fbx / Fbx =	1.094	0.709	
fby / Fby =	0.000	0.000	
fd / Fbxy =	0.011	0.007	
SUM =	0.999 < 1.00	0.784 < 1.00	Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C	REVISION:	REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY CHKD BY SHT. 23 OF 29
	DATE 3/11/94	DATE 4-4-94	DATE DATE 6.27.

COMBINATION 11 - DL + wind +Dif set -16 F (Case 1 + 4 + 8 + 11)

SETTLEMENT WITH COMBINATION

Delta x, x8 = 0 in Maxset = 1.186 = (x8^2 + y8^2)^0.5
 Delta y, y8= 1.186 in

FIXED SUPPORTS

Rx = 1275 lbs = Rfx1 + Rfx4 + x8 * Rfx8 + Rfx11
 Ry = 2872 lbs = Rfy1 + Rfy4 + y8 * Rfy8 + Rfy11
 Rz = 2209 lbs = Rfz1 + Rfz4 + Rfz11
 Rmax per bolt = 3220 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = -348 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 874 lbs = Rgx1 + Rgx4 + x8 * Rgx8 + Rgx11
 Ry = 9818 lbs = Rgy1 + Rgy4 + y8 * Rgy8 + Rgy11
 Rmax per bolt = 6132 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 3685 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.014 in = Delx4 + 0.5 * x8
 Delta y (302) = 0.655 in = Dely1 + 0.5 * y8
 Max Delta = 0.655 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fat = 623 psi, fa11t, pos = tension
 fbx = 6051 psi = fbx1 + fbx4 + y8 * fbx8 + fbx11
 fby = 399 psi = fby1 + fby4 + x8 * fby8 + fby11

 Md = 16688 in-lbs = fac * A * (Max(Max Delta or maxset) + CE)
 fd = 70.2 psi = Md / S

COMBINED STRESS

fat/Fa + fbx/Fbx + fby/Fby + fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 5900 psi for compression from page 3
 Tension Allowable = 9100 psi = Sh * Et, for tension from page 3

	<u>COMPRESSION</u>	<u>TENSION</u>
fa(c or t) / Fa =	-0.106	0.068
fbx / Fbx =	1.026	0.665
fby / Fby =	0.068	0.044
fd / Fbxy =	0.012	0.008
SUM =	0.999 < 1.00	0.785 < 1.00

Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C	REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY
	DATE 3/11/94	DATE 4-4-94	DATE	DATE
				SHT 24 OF 29 6.28

COMBINATION 12 - DL + Insulation + Settlement + Vacuum + 100 F (Case 2+7+8+11)

SETTLEMENT WITH COMBINATION

Delta x, x8 = 0 in Maxset = 1.224 = (x8^2 + y8^2)^0.5
 Delta y, y8 = 1.224 in

FIXED SUPPORTS

Rx = 0 lbs = Rfx2 + Rfx7 + x8 * Rfx8 + Rfx11
 Ry = 4023 lbs = Rfy2 + Rfy7 + y8 * Rfy8 + Rfy11
 Rz = 2209 lbs = Rfz2 + Rfz7 + Rfz11
 Rmax per bolt = 2011 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 2011 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 0 lbs = Rgx2 + Rgx7 + x8 * Rgx8 + Rgx11
 Ry = 10856 lbs = Rgy2 + Rgy7 + y8 * Rgy8 + Rgy11
 Rmax per bolt = 5428 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 5428 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.000 in = 0.5 * x8
 Delta y (302) = 0.691 in = Dely2 + 0.5 * y8
 Max Delta = 0.691 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fat = 771 psi, fav7 + fa11t, pos = tension
 fbx = 6576 psi = fbx2 + fbx7 + y8 * fbx8 + fbx11
 fby = 0 psi = fby2 + fby7 + x8 * fby8 + fby11
 Md = 21223 in-lbs = fac * A * (Max(Max Delta or maxset) + CE)
 fd = 89.3 psi = Md / S

COMBINED STRESS

fat/Fa + fbx/Fbx + fby/Fby + fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 5900 psi for compression from page 3
 Tension Allowable = 9100 psi = Sh * Et, for tension from page 3

	<u>COMPRESSION</u>	<u>TENSION</u>
fa(c or t) / Fa =	-0.131	0.085
fbx / Fbx =	1.115	0.723
fby / Fby =	0.000	0.000
fd / Fbxy =	0.015	0.010
SUM =	0.999 < 1.00	0.817 < 1.00

Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C	REVISION:	REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WLC	MADE BY CHKD BY SHT 25 OF 29
	DATE 3/11/94	DATE 4-4-94	DATE DATE .6.29

COMBINATION 13 - DL + Insul + Seis x + Dif settle + Vac + -15 F (Case 2+5+7+8+11)

SETTLEMENT WITH COMBINATION

Delta x, x8 = 0 in Maxset = 1.438 = (x8^2 + y8^2)^0.5
 Delta y, y8= 1.438 in

FIXED SUPPORTS

Rx = 1287 lbs = Rfx2 + Rfx5 + Rfx7 + x8 * Rfx8 + Rfx11
 Ry = 3499 lbs = Rfy2 + Rfy5 + Rfy7 + y8 * Rfy8 + Rfy11
 Rz = 2209 lbs = Rfz2 + Rfz5 + Rfz7 + Rfz11
 Rmax per bolt = 3551 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = -52 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 882 lbs = Rgx2 + Rgx5 + Rgx7 + x8 * Rgx8 + Rgx11
 Ry = 11902 lbs = Rgy2 + Rgy5 + Rgy7 + y8 * Rgy8 + Rgy11
 Rmax per bolt = 7186 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 4716 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.014 in = Delx5 + 0.5 * x8
 Delta y (302) = 0.798 in = Dely2 + 0.5 * y8
 Max Delta = 0.798 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fat = 771 psi, fav7 + fa11t, pos = tension
 fbx = 7342 psi = fbx2 + fbx5 + fbx7 + y8 * fbx8 + fbx11
 fby = 402 psi = fby2 + fby5 + fby7 + x8 * fby8 + fby11

 Md = 24441 in-lbs = fac * A * (Max(Max Delta or maxset) + CE)
 fd = 102.8 psi = Md / S

COMBINED STRESS

fat/Fa + fbx/Fbx + fby/Fby + fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 7080 psi = 1.2 * B from page 3
 Tension Allowable = 10920 psi = 1.2 * Sh * Et, from page 3

	<u>COMPRESSION</u>	<u>TENSION</u>
fa(c or t) / Fa =	-0.109	0.071
fbx / Fbx =	1.037	0.672
fby / Fby =	0.057	0.037
fd / Fbxy =	0.015	0.009
SUM =	0.999 < 1.00	0.789 < 1.00

Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 26 OF 29
	DATE 3/11/94	DATE 4-4-94	DATE	DATE	

COMBINATION 14 - DL + Insul + Seis z + Dif settle + Vac + -16 F (Case 2+5+7+8+11)

SETTLEMENT WITH COMBINATION

Delta x, x8 = 0 in Maxset = 1.411 = (x8^2 + y8^2)^0.5
 Delta y, y8 = 1.411 in

FIXED SUPPORTS

Rx = 0 lbs = Rfx2 + Rfx6 + Rfx7 + x8 * Rfx8 + Rfx11
 Ry = 3565 lbs = Rfy2 + Rfy6 + Rfy7 + y8 * Rfy8 + Rfy11
 Rz = 4378 lbs = Rfz2 + Rfz6 + Rfz7 + Rfz11
 Rmax per bolt = 1782 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 1782 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 0 lbs = Rgx2 + Rgx6 + Rgx7 + x8 * Rgx8 + Rgx11
 Ry = 11770 lbs = Rgy2 + Rgy6 + Rgy7 + y8 * Rgy8 + Rgy11
 Rmax per bolt = 5885 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 5885 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.000 in = 0.5 * x8
 Delta y (302) = 0.785 in = Dely27 + 0.5 * y8
 Max Delta = 0.785 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fat = 660 psi, fav7 + fa11t - Rfz6 / A , pos = tension
 fbx = 7245 psi = fbx2 + fbx5 + fbx7 + y8 * fbx8 + fbx11
 fby = 402 psi = fby2 + fby5 + fby7 + x8 * fby8 + fby11

 Md = 20568 in-lbs = fac * A * (Max(Max Delta or maxset) + CE)
 fd = 86.5 psi = Md / S

COMBINED STRESS (compression)

fa/Fa + fbx/Fbx + fby/Fby +fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 7080 psi = 1.2 * B from page 3
 Tension Allowable = 10920 psi = 1.2*Sh * Et, from page 3

	<u>COMPRESSION</u>	<u>TENSION</u>
fa(c or t) / Fa =	-0.093	0.060
fbx / Fbx =	1.023	0.663
fby / Fby =	0.057	0.037
fd / Fbxy =	<u>0.012</u>	<u>0.008</u>
SUM =	0.999 < 1.00	0.769 < 1.00

Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C	REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY
	DATE 3/11/94	DATE 4-4-94	DATE	DATE
				SHT 27 OF 29 6.31

- CHECK RIM BENDING PER ROARK & YOUNG 4th Ed, TABLE XIII CASE 13

$$M_o = (p / 2 / \lambda^2) * (A / (A + t*c + 2 * t / \lambda))$$

p = 14.7 psi, internal pressure
 $\lambda = 0.7277 = (3 * (1 - 0.3^2) / (((D_o + t) / 2)^2 * t^2))^{0.25}$
 A = 1.5 sq in = tss * wss
 t = 0.127 in = t
 c = 0.375 in = tss

$$M_o = 10.976 \text{ in lbs / in}$$

$$\text{Stress} = 4083 \text{ psi} = 6 * M / t^2 < 3 * S = 17700 \text{ psi}$$

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 28 OF 29
	DATE 3/11/94	DATE 4-4-94	DATE	DATE	6.32

- SUMMARY - Vertical Differential Misalignment Only

Combination	Fixed supports (lbs)			Guided Support (lbs)		Settlement (in)		Unity	
	Rx	Ry	Rz	Rx	Ry	x	y	Comp	Tension
1	0	7018	5959	0	4980	0	0	0.638	
2	0	5601	5959	0	7810	0	0.579	0.999	
3	1287	5089	5959	882	8831	0	0.788	0.999	
4	0	5168	8128	0	8675	0	0.756	0.999	
5	1275	3413	771	874	8697	0	0.965	0.999	
6	0	7018	771	0	4834	0	0	0.384	
7	0	4541	771	0	9780	0	1.012	0.999	
8	1287	4010	771	882	10840	0	1.229	0.999	
9	0	4093	2939	0	10674	0	1.195	0.999	
10	0	6128	2209	0	11118	0	1.044	0.999	0.784
11	1275	2872	2209	874	9818	0	1.186	0.999	0.785
12	0	4023	2209	0	10856	0	1.224	0.999	0.817
13	1287	3499	2209	882	11902	0	1.438	0.999	0.789
14	0	3565	4378	0	11770	0	1.411	0.999	0.769

COMBINATIONS

- 1 - DL + Insulation + Vacuum + 302 F (Case 2 + 7 + 9)
- 2 - DL + Insulation + Settlement + Vacuum + 302 F (Case 2+7+8+9)
- 3 - DL + Insul + Seis x + Dif settle + Vac + 302 F (Case 2+5+7+8+9)
- 4 - DL + Insul + Seis z + Dif settle + Vac + 302 F (Case 2+5+7+8+9)

- 5 - DL + Wind + Dif Set + 100 F (Case 1 + 4 + 8 + 10)
- 6 - DL + Insulation + Vacuum + 100 F (Case 2 + 7 + 10)
- 7 - DL + Insulation + Settlement + Vacuum + 100 F (Case 2+7+8+10)
- 8 - DL + Insul + Seis x + Dif settle + Vac + 100 F (Case 2+5+7+8+10)
- 9 - DL + Insul + Seis z + Dif settle + Vac + 100 F (Case 2+5+7+8+10)

- 10 - DL + snow + Dif Set + -16 F (Case 1 + 3 + 8 + 11)
- 11 - DL + wind +Dif set -16 F (Case 1 + 4 + 8 + 11)
- 12 - DL + Insulation + Settlement + Vacuum + 100 F (Case 2+7+8+11)
- 13 - DL + Insul + Seis x + Dif settle + Vac + -15 F (Case 2+5+7+8+11)
- 14 - DL + Insul + Seis z + Dif settle + Vac + -16 F (Case 2+5+7+8+11)

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Maximum Differential Settlements	OFFICE:	NOE-C	REVISION:		REFERENCE NO. 930212
	MADE BY	RJW	CHKD BY	WJC	SHT 29 OF 29
	DATE	3/11/94	DATE	4-4-94	0.33

- INPUT VARIABLES

Tube outside diameter, Do = 49.004 in
 Beam Tube section length, L = 19.812 m = 65.000 ft
 Beam Tube Span length, Lsp = 18.9484 m = 62.167 ft
 Tube thickness, t = 0.127 in
 Insulation Density, Deni = 24 kg/m = 16.127 lbs/ft

Vacuum Stiffener thickness, ts = 0.1875 in
 Vacuum Stiffener width, ws = 1.75 in
 Vacuum Stiffener spacing, Ls = 29.84 in
 Support Stiffener thickness, tss = 0.375 in
 Support Stiffener width, wss = 4 in

Mod. of Elast. @ ambient, Ea = 28,300 ksi Table TM-1 @ 70
 Mod. of Elast. @ 302 degrees, Eb = 27,000 ksi Table TM-1, Page 664
 Coefficient of expansion, e = 9E-06 in/in/F Average from 70 to 300 degrees F

Anchor bolt spacing, Abs = 30 in
 C. line height of tube, H = 42.000 in
 Support Collar / Saddle width, b = 20.000 in

Expansion Joint O.D., De = 53.75 in
 Expansion Joint I.D., Dei = 48.75 in
 E.J. Concentricity Error, CE = 0.1875 in
 Expansion joint axial spring rate, Kej = 9147.6 lbs/in = 8316 * 1.1
 Spring rate variation, Eej = 10.0%
 Spring Rate, K = 10062 lbs/in = Kej * (1 + Eej)

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 1 OF 29
	DATE 3/11/94	DATE 4-5-94	DATE	DATE	6.34

- ALLOWABLE STRESS PER ASME SECTION VIII DIV 1 UG 23(b)

- Allowable Stresses @ 300 Degrees F.

Yield Stress, Fy = 19,200 psi Table Y1
 Tensile Allowable, Sh = 13000 psi Table 1A
 Joint Efficiency, Et = 0.7
 Compression Allowable UG 23(b)

A = 0.000648 = 0.125 / (Ro/t)
 B = 5900 psi per Figure HA 3, interpolate to 300 degrees F.
 Where B = Fa = Fbx = Fby = Fbxy, local buckling stress
 See below for column buckling

- Allowable Stresses @ Ambient (100 degrees F)

Yield Stress = 25,000 psi Table Y1
 Tensile Allowable, Sa = 16,300 psi Table 1A
 Compression Allowable UG 23(b)

A = 0.000648 = 0.125 / (Ro/t)
 B = 7800 psi per Figure HA 3, 100 degrees F.
 Where B = Fa = Fbx = Fby = Fbxy

- Allowable Stress Increase for Wind and Seismic
 Allowable increase for wind or seismic is 1.20

- Allowable Axial Stress, Column Buckling per AISC

k = 1
 L = 746 in = Lsp * 12
 r = 17.28069 in = rg

kL / r = 43.16958

Cc = 166.6081 = (2 * PI)^2 * Eb * 1000 / Fy^0.5

Fa = 10533 psi = $\frac{(1 - (kL/r)^2 / 2 / Cc^2) * Fy}{(5/3 + 3*(kL/r)/8/Cc - (kL/r)^3/8/Cc^3)}$

Fa > B, Thus use B = 5900 psi

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 3 OF 29
	DATE 3/11/94	DATE 4.5.94	DATE	DATE	6.36

- CALCULATE WIND LOAD PER ASCE 7-88 (Livingston, LA)

$$F = Qz * Gh * Cf * Af$$

$$Qz = 4.1472 = 0.00256 * Kz * (I * V)^2$$

$$Kz = \frac{0.80}{1.00}, \text{ Assume exposure C}$$

$$I = \frac{1.00}{45 \text{ mph}}$$

$$Gh = \frac{1.32}{0.739} \quad h / D = 0.0853$$

$$Cf = \frac{0.739}{h / D = 0.0853}$$

$$h = (Do / 2 + H) / 12$$

$$D = L$$

$$D' / Do = ws / Do = 0.03571$$

$$Af = 4.084 \text{ Sqft / ft} = Do / 12$$

$$F = 16.53 \text{ lbs / ft}$$

$$F = 1074 \text{ lbs / section}$$

Table 4
Eq 3
Table 6
Table 5
Figure 1
Table 8
Table 12

- CALCULATE WIND LOAD PER ASCE 7-88 (Hanford, WA)

$$F = Qz * Gh * Cf * Af$$

$$Qz = 4.1472 = 0.00256 * Kz * (I * V)^2$$

$$Kz = \frac{0.80}{1.00}, \text{ Assume exposure C}$$

$$I = \frac{1.00}{45 \text{ mph}}$$

$$Gh = \frac{1.32}{0.739} \quad h / D = 0.0853$$

$$Cf = \frac{0.739}{h / D = 0.0853}$$

$$h = (Do / 2 + H) / 12$$

$$D = L$$

$$D' / Do = ws / Do = 0.03571$$

$$Af = 4.083667 \text{ Sqft / ft} = Do / 12$$

$$F = 16.53 \text{ lbs / ft} = ww$$

$$F = 1074 \text{ lbs / section}$$

Table 4
Eq 3
Table 6
Table 5
Figure 1
Table 8
Table 12

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 4 OF 29
	DATE 3/11/94	DATE 4.5.94	DATE	DATE	0.37

- CALCULATE SEISMIC LOAD PER UBC (Livingston, LA)

$$V = Z * I * C / R_w * W$$

$$Z = \frac{0}{}$$

$$I = \frac{1.0}{}$$

$$C = \frac{2.75}{}$$

$$R_w = \frac{3}{}$$

$$C/R_w = 0.916667 > 0.075 \text{ use: } 0.91667$$

$$W = 91.00 \text{ lbs/ft} = \text{DL} + \text{Insulation}$$

$$V = 0.0500W = 0.05W \text{ minimum per ASCE 7-88, 9.11.2}$$

$$V = 4.55 \text{ Lbs/ft}$$

$$V = 295.8 \text{ Lbs per section}$$

- CALCULATE SEISMIC LOAD PER ASCE 7-88 (Hanford, WA)

$$V = Z * I * C / R_w * W$$

$$Z = \frac{0.2}{}$$

$$I = \frac{1.0}{}$$

$$C = \frac{2.75}{}$$

$$R_w = \frac{3}{}$$

$$C/R_w = 0.916667 > 0.075 \text{ use: } 0.91667$$

$$W = 91.00 \text{ lbs/ft} = \text{DL} + \text{Insulation}$$

$$V = 0.1833W$$

$$V = 16.68 \text{ Lbs/ft} = \text{wseis}$$

$$V = 1084.4 \text{ Lbs per section}$$

- CALCULATE SNOW LOAD PER ASCE 7-88 (Hanford, WA only)

$$P_f = 15.12 = 0.7 * C_e * C_t * I * P_g \quad \text{Eq 5a}$$

$$C_e = \frac{0.9}{\text{Little shelter assumed}} \quad \text{Table 18}$$

$$C_t = \frac{1.2}{\text{Unheated}} \quad \text{Table 19}$$

$$I = \frac{1}{\text{Category 1}} \quad \text{Table 20}$$

$$P_g = \frac{20}{\text{psf}} \quad \text{Figure 6}$$

$$P_s = 9.828 = C_s * P_f$$

$$C_s = \frac{0.65}{\text{Angle is 35 degrees, } 90 - (180 - 70) / 2, \text{ Figure 8b}}$$

$$\text{Width} = 46.049 \text{ in} = D_o * \sin 70 \quad \text{Per 7.4.3, slope} > 70 \text{ no load}$$

$$\text{Snow Load per foot, } w_{sn} = 37.71 \text{ lbs/ft} = P_s * \text{Width}$$

$$\text{Snow load per section} = 2451 \text{ lbs}$$

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 5 OF 29
	DATE 3/11/94	DATE 4-5-94	DATE	DATE	6.38

- DETERMINE REACTIONS, STRESSES AND DEFLECTIONS FOR INDIVIDUAL LOAD CASES

- Nomenclature

- Rfx1 = Reaction, fixed support, x direction (horizontal), case 1
- Rgy2 = Reaction, guided support, y direction (vertical), case 2
- Rfz1 = Reaction, fixed support, z direction (axial), case 1
- Mx1 = Moment about the horizontal axis due to vertical loads, Case 1.
- My2 = Moment about the vertical axis due to horizontal loads, Case 2.
- fa9c = Stress, axial, case 9, compression, (if tension t is used instead of c)

- Reactions Based on RISA2D

Fixed support = $\frac{7.029}{1}$ kips, per RISA2D
 1/2 of guided support = $\frac{2.41}{1}$ kips, per RISA2D
 Total, two spans = $\frac{11.849}{1}$ kips, per RISA2D

% Fixed support, Kf = 0.593215 (Percentage of 2 spans)
 % Guided support, Kg = 0.406785

- Moments based on RISA2D

Maximum Moment = $\frac{43480}{1}$ lb-ft, at fixed support
 Based on, $\frac{91}{1}$ lbs/ft, DL + Insulation, used in RISA2D

True DL + Insulation = wdi = 91.00
 Estimated Moment = 43961 lb-ft = wdi * (Lsp ft)² / 8

Moment correction factor, Km = 0.989 = RISA2D moment / Estimated moment

- Deflections based on RISA2D

Midspan Deflection = $\frac{0.079}{1}$ in
 Based on, $\frac{91}{1}$ lbs/ft, DL + Insulation, used in RISA2D

True DL + Insulation = wdi = 91.00
 Estimated Deflection = 0.081 in = wdi * Lsp⁴ / 185 / Eb / I * 12³ / 1000

Moment correction factor, Kd = 0.978 = RISA2D deflection / Estimated deflection

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 6 OF 29
	DATE 3/11/94	DATE 4-5-94	DATE	DATE	6.39

- CASE 1: Dead Load Reactions, Stresses and Deflections

FIXED SUPPORT

Rfx1 = 0
 Rfy1 = 5774 lbs = $K_f * 2 * L * wd$
 Rfz1 = 0

GUIDED SUPPORT

Rgx1 = 0
 Rgy1 = 3959 lbs = $K_g * 2 * L * wd$

MOMENTS & BENDING STRESS

Mx1 = 35774 lb-ft = $wd * (Lsp)^2 / 8 * Km$
 fbx1 = 1806 psi = $Mx1 * 12 / S$

My1 = 0
 fby1 = 0 psi = $My1 * 12 / S$

MIDSPAN DEFLECTION

Dely1 (amb)= 0.062 in = $wd * Lsp^4 / 185 / Ea / I * 12^3 / 1000 * Kd$

- CASE 2: Dead Load plus Insulation Reaction, Stresses and Deflections

FIXED SUPPORT

Rfx2 = 0
 Rfy2 = 7018 lbs = $K_f * 2 * L * wdi$
 Rfz2 = 0

GUIDED SUPPORT

Rgx2 = 0
 Rgy2 = 4812 lbs = $K_g * 2 * L * wdi$

MOMENTS & BENDING STRESS

Mx2 = 43480 lb-ft = $wdi * (Lsp)^2 / 8 * Km$
 fbx2 = 2195 psi = $Mx2 * 12 / S$

My2 = 0
 fby2 = 0 psi = $My2 * 12 / S$

MIDSPAN DEFLECTION

Dely2 (302)= 0.079 in = $wdi * Lsp^4 / 185 / Eb / I * 12^3 / 1000 * Kd$

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 7 OF 29
	DATE 3/11/94	DATE 4.5.94	DATE	DATE	6.40

- CASE 3: Snow Load Reaction, Stresses and Deflections (Hanford, WA)

FIXED SUPPORT

Rfx3 = 0
 Rfy3 = 2908 lbs = $K_f * 2 * L * w_{sn}$
 Rfz3 = 0

GUIDED SUPPORT

Rgx3 = 0
 Rgy3 = 1994 lbs = $K_g * 2 * L * w_{sn}$

MOMENTS & BENDING STRESS

Mx3 = 18020 lb-ft = $w_{sn} * (L_{sp})^2 / 8 * K_m$
 fbx3 = 910 psi = $M_{x3} * 12 / S$
 My3 = 0
 fby3 = 0 psi = $M_{y3} * 12 / S$

MIDSPAN DEFLECTION

Del_{y3} (amb) = 0.031 in = $w_{sn} * L_{sp}^4 / 185 / E_a / I * 12^3 / 1000 * K_d$

- CASE 4: Wind Load Reaction, Stresses and Deflections (Livingston, LA)

FIXED SUPPORT

Rfx4 = 1275 lbs = $w_w * 2 * L * K_f$
 Rfy4 = 0
 Rfz4 = 0

GUIDED SUPPORT

Rgx4 = 874 lbs = $w_w * 2 * L * K_g$
 Rgy4 = 0

MOMENTS & BENDING STRESS

Mx4 = 0
 fbx4 = 0 psi = $M_{x4} * 12 / S$
 My4 = 7896 lb-ft = $w_w * (L_{sp})^2 / 8 * K_m$
 fby4 = 399 psi = $M_{y4} * 12 / S$

MIDSPAN DEFLECTION

Del_{x4} (amb) = 0.014 in = $w_w * L_{sp}^4 / 185 / E_a / I * 12^3 / 1000 * K_d$

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 8 OF 29
	DATE 3/11/94	DATE 4-5-94	DATE	DATE	6.41

- CASE 5: Seismic Reaction, Stresses and Deflections (Hanford, WA), X Direction (Horizontal)

FIXED SUPPORT

Rfx5 = 1287 lbs = wseis * 2 * L * Kf
 Rfy5 = 0
 Rfz5 = 0

GUIDED SUPPORT

Rgx5 = 882 lbs = wseis * 2 * L * Kg
 Rgy5 = 0

MOMENTS & BENDING STRESS

Mx5 = 0
 fbx5 = 0 psi = Mx5 * 12 / S

My5 = 7971 lb-ft = wseis * (Lsp)^2 / 8 * Km
 fby5 = 402 psi = My5 * 12 / S

MIDSPAN DEFLECTION

Delx5 (amb)= 0.014 in = wseis * Lsp^4 / 185 / Ea / I * 12^3 / 1000 * Kd

- CASE 6: Seismic Reaction, Stresses and Deflections (Hanford, WA), Z Direction (Axial)

FIXED SUPPORT

Rfx6 = 0
 Rfy6 = 0
 Rfz6 = 2169 lbs = wseis * 2 * L

GUIDED SUPPORT

Rgx6 = 0
 Rgy6 = 0

MOMENTS & BENDING STRESS

Mx6 = 0
 fbx6 = 0 psi = Mx5 * 12 / S

My6 = 0
 fby6 = 0 psi = My6 * 12 / S

MIDSPAN DEFLECTION

Dely6 (amb)= 0.000 in

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 9 OF 29
	DATE 3/11/94	DATE 4.5.94	DATE	DATE	6.42

- CASE 7: Vacuum

Bellows effective area = 2062.9 sq in = $PI() * ((De+Dei)/2)^2 / 4$
 Tube pressure area = 1866.55 sq in = $PI() * (Di)^2 / 4$
 Bellows pressure area = 196.35 sq in = Bellows effective - tube area
 Axial force, Pzp = 2886 lbs = 14.7 * Bellows pressure area, pos.=tension

Axial Stress due to vacuum, fav7 = 148.0 psi = Pzp / A, pos. = tension

FIXED SUPPORT

Rfx7 = 0 lbs
 Rfy7 = 0
 Rfz7 = 0

GUIDED SUPPORT

Rgx7 = 0 lbs
 Rgy7 = 0
 fbx7 = fby7 = 0
 Dely7 = 0

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 10 OF 29
	DATE 3/11/94	DATE 4-5-94	DATE	DATE	0.43

- CASE 8: Differential Settlement of Fixed Support

- Reactions Based on RISA2D with 1" differential settlement

Fixed support = -2.37 kips, per RISA2D
 1/2 of guided support = 2.47 kips, per RISA2D
 Based on, 1 lbs/ft, DL, used in RISA2D

FIXED SUPPORTS

Rfx8 = -2447 lbs = (RISA2D fixed) - DL * 2 * L * Kf
 Rfy8 = -2447 lbs = (RISA2D fixed) - DL * 2 * L * Kf

GUIDED SUPPORTS

Rgx8 = 4887 lbs = 2 * (RISA2D guided) - DL * 2 * L * Kg
 Rgy8 = 4887 lbs = 2 * (RISA2D guided) - DL * 2 * L * Kg

- Moments based on RISA2D with 1" differential settlement

Maximum Moment = 71360 lb-ft, at fixed support
 Based on, 1 lbs/ft, DL + Insulation, used in RISA2D

Mx8 = My8 = 70882 lb-ft = (RISA2D Moment) - DL * Lsp² / 8 * Km
 fbx8 = fby8 = 3579 psi = Mx8 * 12 / S

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 11 OF 29
	DATE 3/11/94	DATE 4-5-94	DATE	DATE	6.44

- CASE 9: Expansion Joint Forces at 302 Degrees F.

Maximum Bellows spring rate, K = 10062 lbs/in = $Kej * (1 + Eej)$
 Est. temp. change above 70, Tch = 232 degrees Fahrenheit
 Thermal growth hot, x = 3.257 in = $e * 2 * L * 12 * Tch$
 Longitudinal Comp. force, Pbc = 32776 lbs = $K * x$
 fa9c = -1681 psi = Pbc / A , stress on tube, neg is comp

FIXED SUPPORT

Rfx9 = 0 lbs
 Rfy9 = 0
 Rfz9 = 5959 lbs = $2 * Eej * Kej * x$

GUIDED SUPPORT

Rgx9 = 0
 Rgy9 = 167.8 lbs = $Rfz9 * H / Lsp / 12 / 2$
 fbx9 = fby9 = 0
 Delx9 = Dely9 = 0

- CASE 10: Expansion Joint Forces at 100 Degrees F:

Maximum Bellows spring rate, K = 10062 lbs/in = $Kej * (1 + Eej)$
 Est. temp. change above 70, Tch = 30 degrees Fahrenheit
 Thermal growth hot, xw = 0.421 in = $e * 2 * L * 12 * Tch$
 Longitudinal Comp. force, Pbc = 4238 lbs = $K * xw$
 fa10c = -217 psi = Pbc / A , stress on tube, neg is comp

FIXED SUPPORT

Rfx10 = 0 lbs
 Rfy10 = 0
 Rfz10 = 771 lbs = $2 * Eej * Kej * xw$

GUIDED SUPPORT

Rgx10 = 0
 Rgy10 = 21.7 lbs = $Rfz10 * H / Lsp / 12 / 2$
 fbx10 = fby10 = 0
 Delx10 = Dely10 = 0

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 12 OF 29
	DATE 3/11/94	DATE 4-5-94	DATE	DATE	6.45

- CASE 11: Expansion Joint Forces at -16 Degrees F.

Maximum Bellows spring rate, K = 10062 lbs/in = $Kej * (1 + Eej)$
 Est. temp. change below 70, Tchc = 86 degrees Fahrenheit
 Thermal shrinkage, xs = $-1.207 \text{ in} = e * 2 * L * 12 * Tchc$
 Longitudinal Tension force, Pbt = 12150 lbs = $K * xs$
 fa11t = 623 psi = $-Pbt / A$ Positive is tension

FIXED SUPPORT

Rfx11 = 0 lbs
 Rfy11 = 0
 Rfz11 = 2209 lbs = $-2 * Eej * Kej * xs$

GUIDED SUPPORT

Rgx11 = 0
 Rgy11 = 62.2 lbs = $Rfz11 * H / Lsp / 12 / 2$
 fbx11 = fby11 = 0
 Delx11 = Dely11 = 0

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 13 OF 29
	DATE 3/11/94	DATE 4-5-94	DATE	DATE	v. 46

- LOAD COMBINATIONS FOR MAXIMUM REACTIONS, STRESS AND DEFLECTIONS
COMBINATION 1 - DL + Insulation + Vacuum + 302 F (Case 2 + 7 + 9)

FIXED SUPPORTS

Rx = 0 lbs = Rfx2 + Rfx7 + Rfx9 (lateral)
 Ry = 7018 lbs = Rfy2 + Rfy7 + Rfy9 (Vertical)
 Rz = 5959 lbs = Rfz2 + Rfz7 + Rfz9 (axial)
 Rmax per bolt = 3509 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 3509 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 0 lbs = Rgx2 + Rgx7 + Rgx9
 Ry = 4980 lbs = Rgy2 + Rgy7 + Rgy9 (Vertical)
 Rmax per bolt = 2490 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 2490 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.000 in = Delx2 + Delx7 + Delx9
 Delta y (302) = 0.079 in = Dely2 + Dely7 + Dely9
 Max Delta = 0.079 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fac = -1533 psi, fav7 + fa9c , neg = compression
 fbx = 2195 psi = fbx2 + fbx7 + fbx9
 fby = 0 psi = fby2 + fby7 + fby9
 Md = 7966 in-lbs = fac * A * (Max Delta + CE)
 fd = 33.5 psi = Md / S

COMBINED STRESS (compression)

fat/Fa + fbx/Fbx + fby/Fby + fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 5900 psi for compression from page 3

fac / Fa = 0.260
 fbx / Fbx = 0.372
 fby / Fby = 0.000
 fd / Fbxy = 0.006
 SUM = 0.638 < 1.00 Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 14 OF 29
	DATE 3/11/94	DATE 4-5-94	DATE	DATE	.6.47

COMBINATION 2 - DL + Insulation + Settlement + Vacuum + 302 F (Case 2+7+8+9)

SETTLEMENT WITH COMBINATION

Delta x, x8 = 0 in Maxset = 0.579 = (x8^2 + y8^2)^0.5
 Delta y, y8= 0.579 in

FIXED SUPPORTS

Rx = 0 lbs = Rfx2 + Rfx7 + x8 * Rfx8 + Rfx9
 Ry = 5601 lbs = Rfy2 + Rfy7 + y8 * Rfy8 + Rfy9
 Rz = 5959 lbs = Rfz2 + Rfz7 + Rfz9
 Rmax per bolt = 2800 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 2800 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 0 lbs = Rgx2 + Rgx7 + x8 * Rgx8 + Rgx9
 Ry = 7810 lbs = Rgy2 + Rgy7 + y8 * Rgy8 + Rgy9
 Rmax per bolt = 3905 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 3905 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.000 in = 0.5 * x8
 Delta y (302) = 0.369 in = Dely2 + 0.5 * y8
 Max Delta = 0.369 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fac = -1533 psi, fav7 + fa9c , neg = compression
 fbx = 4267 psi = fbx2 + fbx7 + y8 * fbx8 + fbx9
 fby = 0 psi = fby2 + fby7 + x8 * fby8 + fby9
 Md = 22910 in-lbs = fac * A * (Max(Max Delta or maxset) + CE)
 fd = 96.4 psi = Md / S

COMBINED STRESS (compression)

fat/Fa + fbx/Fbx + fby/Fby + fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 5900 psi for compression from page 3

fac / Fa = 0.260
 fbx / Fbx = 0.723
 fby / Fby = 0.000
 fd / Fbxy = 0.016
 SUM = 0.999 < 1.00 Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 15 OF 29
	DATE 3/11/94	DATE 4-5-94	DATE	DATE	6.48

COMBINATION 3 - DL + Insul + Seis x + Dif settle + Vac + 302 F (Case 2+5+7+8+9)

SETTLEMENT WITH COMBINATION

Delta x, x8 = 0 in Maxset = 0.579 = (x8^2 + y8^2)^0.5
 Delta y, y8 = 0.579 in

FIXED SUPPORTS

Rx = 1287 lbs = Rfx2 + Rfx5 + Rfx7 + x8 * Rfx8 + Rfx9
 Ry = 5601 lbs = Rfy2 + Rfy5 + Rfy7 + y8 * Rfy8 + Rfy9
 Rz = 5959 lbs = Rfz2 + Rfz5 + Rfz7 + Rfz9
 Rmax per bolt = 4602 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 999 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 882 lbs = Rgx2 + Rgx5 + Rgx7 + x8 * Rgx8 + Rgx9
 Ry = 7810 lbs = Rgy2 + Rgy5 + Rgy7 + y8 * Rgy8 + Rgy9
 Rmax per bolt = 5140 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 2670 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.014 in = Delx5 + 0.5 * x8
 Delta y (302) = 0.369 in = Dely2 + 0.5 * y8
 Max Delta = 0.369 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fac = -1533 psi, fav7 + fa9c , neg = compression
 fbx = 4267 psi = fbx2 + fbx5 + fbx7 + y8 * fbx8 + fbx9
 fby = 402 psi = fby2 + fby5 + fby7 + x8 * fby8 + fby9
 Md = 22910 in-lbs = fac * A * (Max(Max Delta or maxset) + CE)
 fd = 96.4 psi = Md / S

COMBINED STRESS (compression)

fat/Fa + fbx/Fbx + fby/Fby + fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 7080 psi = 1.2 * B from page 3

fac / Fa = 0.216
 fbx / Fbx = 0.603
 fby / Fby = 0.057
 fd / Fbxy = 0.014
 SUM = 0.890 < 1.00

Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 16 OF 29
	DATE 3/11/94	DATE 4-5-94	DATE	DATE	6.49

COMBINATION 4 - DL + Insul + Seis z + Dif settle + Vac + 302 F (Case 2+5+7+8+9)

SETTLEMENT WITH COMBINATION

Delta x, x8 = 0 in Maxset = 0.579 = (x8^2 + y8^2)^0.5
 Delta y, y8 = 0.579 in

FIXED SUPPORTS

Rx = 0 lbs = Rfx2 + Rfx6 + Rfx7 + x8 * Rfx8 + Rfx9
 Ry = 5601 lbs = Rfy2 + Rfy6 + Rfy7 + y8 * Rfy8 + Rfy9
 Rz = 8128 lbs = Rfz2 + Rfz6 + Rfz7 + Rfz9
 Rmax per bolt = 2800 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 2800 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 0 lbs = Rgx2 + Rgx6 + Rgx7 + x8 * Rgx8 + Rgx9
 Ry = 7810 lbs = Rgy2 + Rgy6 + Rgy7 + y8 * Rgy8 + Rgy9
 Rmax per bolt = 3905 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 3905 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.000 in = 0.5 * x8
 Delta y (302) = 0.369 in = Dely27 + 0.5 * y8
 Max Delta = 0.369 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fac = -1644 psi, fav7 + fa9c - Rfz6 / A , neg = compression
 fbx = 4267 psi = fbx2 + fbx5 + fbx7 + y8 * fbx8 + fbx9
 fby = 402 psi = fby2 + fby5 + fby7 + x8 * fby8 + fby9
 Md = 24573 in-lbs = fac * A * (Max(Max Delta or maxset) + CE)
 fd = 103.4 psi = Md / S

COMBINED STRESS (compression)

fat/Fa + fbx/Fbx + fby/Fby + fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 7080 psi = 1.2 * B from page 3

fac / Fa = 0.232
 fbx / Fbx = 0.603
 fby / Fby = 0.057
 fd / Fbxy = 0.015
 SUM = 0.906 < 1.00 Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 17 OF 29
	DATE 3/11/94	DATE 4-5-94	DATE	DATE	6.50

COMBINATION 5 - DL + Wind + Dif Set + 100 F (Case 1 + 4 + 8 + 10)

SETTLEMENT WITH COMBINATION

Delta x, x8 = 0 in Maxset = 0.579 = (x8^2 + y8^2)^0.5
 Delta y, y8 = 0.579 in

FIXED SUPPORTS

Rx = 1275 lbs = Rfx1 + Rfx4 + x8 * Rfx8 + Rfx10
 Ry = 4357 lbs = Rfy1 + Rfy4 + y8 * Rfy8 + Rfy10
 Rz = 771 lbs = Rfz1 + Rfz4 + Rfz10
 Rmax per bolt = 3963 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 394 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 874 lbs = Rgx1 + Rgx4 + x8 * Rgx8 + Rgx10
 Ry = 6811 lbs = Rgy1 + Rgy4 + y8 * Rgy8 + Rgy10
 Rmax per bolt = 4629 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 2182 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.014 in = Delx4 + 0.5 * x8
 Delta y (302) = 0.352 in = Dely1 + 0.5 * y8
 Max Delta = 0.352 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fac = -217 psi, fav4 + fa10c , neg = compression
 fbx = 3878 psi = fbx1 + fbx4 + y8 * fbx8 + fbx10
 fby = 399 psi = fby1 + fby4 + x8 * fby8 + fby10
 Md = 3249 in-lbs = fac * A * (Max(Max Delta or maxset) + CE)
 fd = 13.7 psi = Md / S

COMBINED STRESS (compression)

fat/Fa + fbx/Fbx + fby/Fby + fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 5900 psi for compression from page 3

fac / Fa = 0.037
 fbx / Fbx = 0.657
 fby / Fby = 0.068
 fd / Fbxy = 0.002
 SUM = 0.764 < 1.00 Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 18 OF 29
	DATE 3/11/94	DATE 4-1-94	DATE	DATE	6.51

COMBINATION 6 - DL + Insulation + Vacuum + 100 F (Case 2 + 7 + 10)

FIXED SUPPORTS

Rx = 0 lbs = Rfx2 + Rfx7 + Rfx10 (lateral)
 Ry = 7018 lbs = Rfy2 + Rfy7 + Rfy10 (Vertical)
 Rz = 771 lbs = Rfz2 + Rfz7 + Rfz10 (axial)
 Rmax per bolt = 3509 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 3509 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 0 lbs = Rgx2 + Rgx7 + Rgx10
 Ry = 4834 lbs = Rgy2 + Rgy7 + Rgy10 (Vertical)
 Rmax per bolt = 2417 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 2417 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.000 in = Delx2 + Delx7 + Delx10
 Delta y (302) = 0.079 in = Dely2 + Dely7 + Dely10
 Max Delta = 0.079 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fac = -69 psi, fav7 + fa10c , neg = compression
 fbx = 2195 psi = fbx2 + fbx7 + fbx10
 fby = 0 psi = fby2 + fby7 + fby10
 Md = 360 in-lbs = fac * A * (Max Delta + CE)
 fd = 1.5 psi = Md / S

COMBINED STRESS (compression)

fat/Fa + fbx/Fbx + fby/Fby + fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 5900 psi for compression from page 3

fac / Fa = 0.012
 fbx / Fbx = 0.372
 fby / Fby = 0.000
 fd / Fbxy = 0.000
 SUM = 0.384 < 1.00 Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY VJC	MADE BY	CHKD BY	SHT 19 OF 29
	DATE -3/11/94	DATE 4-5-94	DATE	DATE	4.52

COMBINATION 7 - DL + Insulation + Settlement + Vacuum + 100 F (Case 2+7+8+10)

SETTLEMENT WITH COMBINATION

Delta x, x8 = 0 in Maxset = 0.579 = (x8^2 + y8^2)^0.5
 Delta y, y8 = 0.579 in

FIXED SUPPORTS

Rx = 0 lbs = Rfx2 + Rfx7 + x8 * Rfx8 + Rfx10
 Ry = 5601 lbs = Rfy2 + Rfy7 + y8 * Rfy8 + Rfy10
 Rz = 771 lbs = Rfz2 + Rfz7 + Rfz10
 Rmax per bolt = 2800 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 2800 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 0 lbs = Rgx2 + Rgx7 + x8 * Rgx8 + Rgx10
 Ry = 7664 lbs = Rgy2 + Rgy7 + y8 * Rgy8 + Rgy10
 Rmax per bolt = 3832 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 3832 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.000 in = 0.5 * x8
 Delta y (302) = 0.369 in = Dely2 + 0.5 * y8
 Max Delta = 0.369 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fac = -69 psi, fav7 + fa10c , neg = compression
 fbx = 4267 psi = fbx2 + fbx7 + y8 * fbx8 + fbx10
 fby = 0 psi = fby2 + fby7 + x8 * fby8 + fby10
 Md = 1036 in-lbs = fac * A * (Max(Max Delta or maxset) + CE)
 fd = 4.4 psi = Md / S

COMBINED STRESS (compression)

fat/Fa + fbx/Fbx + fby/Fby + fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 5900 psi for compression from page 3

fac / Fa = 0.012
 fbx / Fbx = 0.723
 fby / Fby = 0.000
 fd / Fbxy = 0.001
 SUM = 0.736 < 1.00 Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJK	MADE BY	CHKD BY	SHT 20 OF 29
	DATE 3/11/94	DATE 4-5-94	DATE	DATE	6.53

COMBINATION 8 - DL + Insul + Seis x + Dif settle + Vac + 100 F (Case 2+5+7+8+10)

SETTLEMENT WITH COMBINATION

Delta x, x8 = 0 in Maxset = 0.579 = (x8² + y8²)^{0.5}
 Delta y, y8= 0.579 in

FIXED SUPPORTS

Rx = 1287 lbs = Rfx2 + Rfx5 + Rfx7 + x8 * Rfx8 + Rfx10
 Ry = 5601 lbs = Rfy2 + Rfy5 + Rfy7 + y8 * Rfy8 + Rfy10
 Rz = 771 lbs = Rfz2 + Rfz5 + Rfz7 + Rfz10
 Rmax per bolt = 4602 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 999 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 882 lbs = Rgx2 + Rgx5 + Rgx7 + x8 * Rgx8 + Rgx10
 Ry = 7664 lbs = Rgy2 + Rgy5 + Rgy7 + y8 * Rgy8 + Rgy10
 Rmax per bolt = 5067 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 2597 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.014 in = Delx5 + 0.5 * x8
 Delta y (302) = 0.369 in = Dely2 + 0.5 * y8
 Max Delta = 0.369 in = (Delta x ² + Delta y ²)^{0.5}

TUBE STRESSES

fac = -69 psi, fav7 + fa10c , neg = compression
 fbx = 4267 psi = fbx2 + fbx5 + fbx7 + y8 * fbx8 + fbx10
 fby = 402 psi = fby2 + fby5 + fby7 + x8 * fby8 + fby10
 Md = 1036 in-lbs = fac * A * (Max(Max Delta or maxset) + CE)
 fd = 4.4 psi = Md / S

COMBINED STRESS (compression)

fat/Fa + fbx/Fbx + fby/Fby + fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 7080 psi = 1.2 * B from page 3

fac / Fa = 0.010
 fbx / Fbx = 0.603
 fby / Fby = 0.057
 fd / Fbxy = 0.001
 SUM = 0.670 < 1.00 Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 21 OF 27
	DATE 3/11/94	DATE 4-5-94	DATE	DATE	6.54

COMBINATION 9 - DL + Insul + Seis z + Dif settle + Vac + 100 F (Case 2+5+7+8+10)

SETTLEMENT WITH COMBINATION

Delta x, x8 = 0 in Maxset = 0.579 = (x8^2 + y8^2)^0.5
 Delta y, y8= 0.579 in

FIXED SUPPORTS

Rx = 0 lbs = Rfx2 + Rfx6 + Rfx7 + x8 * Rfx8 + Rfx10
 Ry = 5601 lbs = Rfy2 + Rfy6 + Rfy7 + y8 * Rfy8 + Rfy10
 Rz = 2939 lbs = Rfz2 + Rfz6 + Rfz7 + Rfz10
 Rmax per bolt = 2800 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 2800 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 0 lbs = Rgx2 + Rgx6 + Rgx7 + x8*Rgx8 + Rgx10
 Ry = 7664 lbs = Rgy2 + Rgy6 + Rgy7 + y8*Rgy8 + Rgy10
 Rmax per bolt = 3832 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 3832 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.000 in = 0.5 * x8
 Delta y (302) = 0.369 in = Dely27 + 0.5 * y8
 Max Delta = 0.369 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fac = -181 psi, fav7 + fa10c - Rfz6 / A , neg = compression
 fbx = 4267 psi = fbx2 + fbx5 + fbx7 + y8 * fbx8 + fbx10
 fby = 402 psi = fby2 + fby5 + fby7 + x8 * fby8 + fby10

 Md = 2699 in-lbs = fac * A * (Max(Max Delta or maxset) + CE)
 fd = 11.4 psi = Md / S

COMBINED STRESS (compression)

fat/Fa + fbx/Fbx + fby/Fby +fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 7080 psi = 1.2 * B from page 3

fac / Fa = 0.026
 fbx / Fbx = 0.603
 fby / Fby = 0.057
 fd / Fbxy = 0.002
 SUM = 0.687 < 1.00 Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 22 OF 29
	DATE 3/11/94	DATE 4-5-94	DATE	DATE	6,55.

COMBINATION 10 - DL + snow + Dif Set + -16 F (Case 1 + 3 + 8 + 11)

SETTLEMENT WITH COMBINATION

Delta x, x8 = 0 in Maxset = 0.579 = (x8^2 + y8^2)^0.5
 Delta y, y8 = 0.579 in

FIXED SUPPORTS

Rx = 0 lbs = Rfx1 + Rfx3 + x8 * Rfx8 + Rfx11
 Ry = 7266 lbs = Rfy1 + Rfy3 + y8 * Rfy8 + Rfy11
 Rz = 2209 lbs = Rfz1 + Rfz3 + Rfz11
 Rmax per bolt = 3633 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 3633 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 0 lbs = Rgx1 + Rgx3 + x8 * Rgx8 + Rgx11
 Ry = 8846 lbs = Rgy1 + Rgy3 + y8 * Rgy8 + Rgy11
 Rmax per bolt = 4423 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 4423 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.000 in = 0.5 * x8
 Delta y (302) = 0.383 in = Dely1 + Dely3 + 0.5 * y8
 Max Delta = 0.383 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fat = 623 psi, fa11t, pos = tension
 fbx = 4788 psi = fbx1 + fbx3 + y8 * fbx8 + fbx11
 fby = 0 psi = fby1 + fby3 + x8 * fby8 + fby11
 Md = 9313 in-lbs = fac * A * (Max(Max Delta or maxset) + CE)
 fd = 39.2 psi = Md / S

COMBINED STRESS

fat/Fa + fbx/Fbx + fby/Fby + fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 5900 psi for compression from page 3
 Tension Allowable = 9100 psi = Sh * Et, for tension from page 3

	<u>COMPRESSION</u>	<u>TENSION</u>	
fa(c or t) / Fa =	-0.106	0.068	
fbx / Fbx =	0.812	0.526	
fby / Fby =	0.000	0.000	
fd / Fbxy =	<u>0.007</u>	<u>0.004</u>	
SUM =	0.713 < 1.00	0.599 < 1.00	Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 23 OF 29
	DATE 3/11/94	DATE 4-5-94	DATE	DATE	6.56.

COMBINATION 11 - DL + wind +Dif set -16 F (Case 1 + 4 + 8 + 11)

SETTLEMENT WITH COMBINATION

Delta x, x8 = 0 in Maxset = 0.579 = (x8^2 + y8^2)^0.5
 Delta y, y8= 0.579 in

FIXED SUPPORTS

Rx = 1275 lbs = Rfx1 + Rfx4 +x8 * Rfx8 + Rfx11
 Ry = 4357 lbs = Rfy1 + Rfy4+ y8 * Rfy8 + Rfy11
 Rz = 2209 lbs = Rfz1 + Rfz4 + Rfz11
 Rmax per bolt = 3963 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 394 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 874 lbs = Rgx1 + Rgx4 + x8*Rgx8 + Rgx11
 Ry = 6851 lbs = Rgy1 + Rgy4 + y8*Rgy8 + Rgy11
 Rmax per bolt = 4649 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 2202 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.014 in =Delx4 + 0.5 * x8
 Delta y (302) = 0.352 in = Dely1 + 0.5 * y8
 Max Delta = 0.352 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fat = 623 psi, fa11t , pos = tension
 fbx = 3878 psi = fbx1 + fbx4 + y8 * fbx8 + fbx11
 fby = 399 psi = fby1 + fby4 + x8 * fby8 + fby11

 Md = 9313 in-lbs = fac * A * (Max(Max Delta or maxset) + CE)
 fd = 39.2 psi = Md / S

COMBINED STRESS

fat/Fa + fbx/Fbx + fby/Fby +fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 5900 psi for compression from page 3
 Tension Allowable = 9100 psi = Sh * Et, for tension from page 3

	<u>COMPRESSION</u>	<u>TENSION</u>
fa(c or t) / Fa =	-0.106	0.068
fbx / Fbx =	0.657	0.426
fby / Fby =	0.068	0.044
fd / Fbxy =	0.007	0.004
SUM =	0.626 < 1.00	0.543 < 1.00

Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 24 OF 29
	DATE 3/11/94	DATE 4-5-94	DATE	DATE	6.57

COMBINATION 12 - DL + Insulation + Settlement + Vacuum + 100 F (Case 2+7+8+11)

SETTLEMENT WITH COMBINATION

$$\begin{aligned} \text{Delta x, } x8 &= \underline{0 \text{ in}} & \text{Maxset} &= 0.579 = (x8^2 + y8^2)^{0.5} \\ \text{Delta y, } y8 &= \underline{0.579 \text{ in}} \end{aligned}$$

FIXED SUPPORTS

$$\begin{aligned} R_x &= 0 \text{ lbs} = R_{fx2} + R_{fx7} + x8 * R_{fx8} + R_{fx11} \\ R_y &= 5601 \text{ lbs} = R_{fy2} + R_{fy7} + y8 * R_{fy8} + R_{fy11} \\ R_z &= 2209 \text{ lbs} = R_{fz2} + R_{fz7} + R_{fz11} \\ R_{\text{max per bolt}} &= 2800 \text{ lbs} = R_y/2 + R_x * H / \text{Abs} \\ R_{\text{min per bolt}} &= 2800 \text{ lbs} = R_y/2 - R_x * H / \text{Abs} \end{aligned}$$

GUIDED SUPPORTS

$$\begin{aligned} R_x &= 0 \text{ lbs} = R_{gx2} + R_{gx7} + x8 * R_{gx8} + R_{gx11} \\ R_y &= 7704 \text{ lbs} = R_{gy2} + R_{gy7} + y8 * R_{gy8} + R_{gy11} \\ R_{\text{max per bolt}} &= 3852 \text{ lbs} = R_y/2 + R_x * H / \text{Abs} \\ R_{\text{min per bolt}} &= 3852 \text{ lbs} = R_y/2 - R_x * H / \text{Abs} \end{aligned}$$

MIDSPAN DEFLECTION

$$\begin{aligned} \text{Delta x (302)} &= 0.000 \text{ in} = 0.5 * x8 \\ \text{Delta y (302)} &= 0.369 \text{ in} = \text{Dely2} + 0.5 * y8 \\ \text{Max Delta} &= 0.369 \text{ in} = (\text{Delta x}^2 + \text{Delta y}^2)^{0.5} \end{aligned}$$

TUBE STRESSES

$$\begin{aligned} f_{at} &= 771 \text{ psi, } f_{av7} + f_{a11t}, \text{ pos} = \text{tension} \\ f_{bx} &= 4267 \text{ psi} = f_{bx2} + f_{bx7} + y8 * f_{bx8} + f_{bx11} \\ f_{by} &= 0 \text{ psi} = f_{by2} + f_{by7} + x8 * f_{by8} + f_{by11} \\ \\ M_d &= 11525 \text{ in-lbs} = f_{ac} * A * (\text{Max}(\text{Max Delta or maxset}) + \text{CE}) \\ f_d &= 48.5 \text{ psi} = M_d / S \end{aligned}$$

COMBINED STRESS

$$\begin{aligned} f_{at}/F_a + f_{bx}/F_{bx} + f_{by}/F_{by} + f_d/F_{bxy} &<= 1.00 \\ \text{Where } B=F_a=F_{bx}=F_{by}=F_{bxy} &= 5900 \text{ psi for compression from page 3} \\ \text{Tension Allowable} &= 9100 \text{ psi} = S_h * E_t, \text{ for tension from page 3} \end{aligned}$$

	<u>COMPRESSION</u>	<u>TENSION</u>
$f_{a(c \text{ or } t)} / F_a =$	-0.131	0.085
$f_{bx} / F_{bx} =$	0.723	0.469
$f_{by} / F_{by} =$	0.000	0.000
$f_d / F_{bxy} =$	<u>0.008</u>	<u>0.005</u>
SUM =	0.601 < 1.00	0.559 < 1.00

Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE:	NOE-C	REVISION:		REFERENCE NO. 930212
	MADE BY	CHKD BY	MADE BY	CHKD BY	SHT 25 OF 29
	RJW	WJC			
	DATE	DATE	DATE	DATE	v.58
	3/11/94	4-5-94			

COMBINATION 13 - DL + Insul + Seis x + Dif settle + Vac + -15 F (Case 2+5+7+8+11)

SETTLEMENT WITH COMBINATION

Delta x, x8 = 0 in Maxset = 0.579 = (x8^2 + y8^2)^0.5
 Delta y, y8 = 0.579 in

FIXED SUPPORTS

Rx = 1287 lbs = Rfx2 + Rfx5 + Rfx7 + x8 * Rfx8 + Rfx11
 Ry = 5601 lbs = Rfy2 + Rfy5 + Rfy7 + y8 * Rfy8 + Rfy11
 Rz = 2209 lbs = Rfz2 + Rfz5 + Rfz7 + Rfz11
 Rmax per bolt = 4602 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 999 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 882 lbs = Rgx2 + Rgx5 + Rgx7 + x8 * Rgx8 + Rgx11
 Ry = 7704 lbs = Rgy2 + Rgy5 + Rgy7 + y8 * Rgy8 + Rgy11
 Rmax per bolt = 5087 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 2617 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.014 in = Delx5 + 0.5 * x8
 Delta y (302) = 0.369 in = Dely2 + 0.5 * y8
 Max Delta = 0.369 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fat = 771 psi, fav7 + fa11t, pos = tension
 fbx = 4267 psi = fbx2 + fbx5 + fbx7 + y8 * fbx8 + fbx11
 fby = 402 psi = fby2 + fby5 + fby7 + x8 * fby8 + fby11
 Md = 11525 in-lbs = fac * A * (Max(Max Delta or maxset) + CE)
 fd = 48.5 psi = Md / S

COMBINED STRESS

fat/Fa + fbx/Fbx + fby/Fby + fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 7080 psi = 1.2 * B from page 3
 Tension Allowable = 10920 psi = 1.2 * Sh * Et, from page 3

	<u>COMPRESSION</u>	<u>TENSION</u>
fa(c or t) / Fa =	-0.109	0.071
fbx / Fbx =	0.603	0.391
fby / Fby =	0.057	0.037
fd / Fbxy =	0.007	0.004
SUM =	0.558 < 1.00	0.503 < 1.00

Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 26 OF 29
	DATE 3/11/94	DATE 4.5.94	DATE	DATE	6.59

COMBINATION 14 - DL + Insul + Seis z + Dif settle + Vac + -16 F (Case 2+5+7+8+11)

SETTLEMENT WITH COMBINATION

Delta x, x8 = 0 in Maxset = 0.579 = (x8^2 + y8^2)^0.5
 Delta y, y8= 0.579 in

FIXED SUPPORTS

Rx = 0 lbs = Rfx2 + Rfx6 + Rfx7 + x8 * Rfx8 + Rfx11
 Ry = 5601 lbs = Rfy2 + Rfy6 + Rfy7 + y8 * Rfy8 + Rfy11
 Rz = 4378 lbs = Rfz2 + Rfz6 + Rfz7 + Rfz11
 Rmax per bolt = 2800 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 2800 lbs = Ry/2 - Rx * H / Abs

GUIDED SUPPORTS

Rx = 0 lbs = Rgx2 + Rgx6 + Rgx7 + x8 * Rgx8 + Rgx11
 Ry = 7704 lbs = Rgy2 + Rgy6 + Rgy7 + y8 * Rgy8 + Rgy11
 Rmax per bolt = 3852 lbs = Ry/2 + Rx * H / Abs
 Rmin per bolt = 3852 lbs = Ry/2 - Rx * H / Abs

MIDSPAN DEFLECTION

Delta x (302) = 0.000 in = 0.5 * x8
 Delta y (302) = 0.369 in = Dely27 + 0.5 * y8
 Max Delta = 0.369 in = (Delta x ^2 + Delta y ^2)^0.5

TUBE STRESSES

fat = 660 psi, fav7 + fa11t - Rfz6 / A , pos = tension
 fbx = 4267 psi = fbx2 + fbx5 + fbx7 + y8 * fbx8 + fbx11
 fby = 402 psi = fby2 + fby5 + fby7 + x8 * fby8 + fby11
 Md = 9863 in-lbs = fac * A * (Max(Max Delta or maxset) + CE)
 fd = 41.5 psi = Md / S

COMBINED STRESS (compression)

fat/Fa + fbx/Fbx + fby/Fby +fd/Fbxy <= 1.00
 Where B=Fa=Fbx=Fby=Fbxy= 7080 psi = 1.2 * B from page 3
 Tension Allowable = 10920 psi = 1.2*Sh * Et, from page 3

	<u>COMPRESSION</u>	<u>TENSION</u>
fa(c or t) / Fa =	-0.093	0.060
fbx / Fbx =	0.603	0.391
fby / Fby =	0.057	0.037
fd / Fbxy =	<u>0.006</u>	<u>0.004</u>
SUM =	0.572 < 1.00	0.492 < 1.00

Tube is adequate

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 27 OF 29
	DATE 3/11/94	DATE 4-5-94	DATE	DATE	6.60

- CHECK RIM BENDING PER ROARK & YOUNG 4th Ed, TABLE XIII CASE 13

$$M_o = (p / 2 / \lambda^2) * (A / (A + t*c + 2 * t / \lambda))$$

- p = 14.7 psi, internal pressure
- $\lambda = 0.7277 = (3 * (1 - 0.3^2) / (((D_o + t) / 2)^2 * t^2))^{0.25}$
- A = 1.5 sq in = tss * wss
- t = 0.127 in = t
- c = 0.375 in = tss

$$M_o = 10.976 \text{ in lbs / in}$$

$$\text{Stress} = 4083 \text{ psi} = 6 * M / t^2 < 3 * S = 17700 \text{ psi}$$

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 28 OF 29
	DATE 3/11/94	DATE 4-5-94	DATE	DATE	6.61

- SUMMARY - Vertical Differential Misalignment Only

Combination	Fixed supports (lbs)			Guided Support (lbs)		Settlement (in)		Unity	
	Rx	Ry	Rz	Rx	Ry	x	y	Comp	Tension
1	0	7018	5959	0	4980	0	0	0.638	
2	0	5601	5959	0	7810	0	0.579	0.999	
3	1287	5601	5959	882	7810	0	0.579	0.890	
4	0	5601	8128	0	7810	0	0.579	0.906	
5	1275	4357	771	874	6811	0	0.579	0.764	
6	0	7018	771	0	4834	0	0	0.384	
7	0	5601	771	0	7664	0	0.579	0.736	
8	1287	5601	771	882	7664	0	0.579	0.670	
9	0	5601	2939	0	7664	0	0.579	0.687	
10	0	7266	2209	0	8846	0	0.579	0.713	0.599
11	1275	4357	2209	874	6851	0	0.579	0.626	0.543
12	0	5601	2209	0	7704	0	0.579	0.601	0.559
13	1287	5601	2209	882	7704	0	0.579	0.558	0.503
14	0	5601	4378	0	7704	0	0.579	0.572	0.492

COMBINATIONS

- 1 - DL + Insulation + Vacuum + 302 F (Case 2 + 7 + 9)
- 2 - DL + Insulation + Settlement + Vacuum + 302 F (Case 2+7+8+9)
- 3 - DL + Insul + Seis x + Dif settle + Vac + 302 F (Case 2+5+7+8+9)
- 4 - DL + Insul + Seis z + Dif settle + Vac + 302 F (Case 2+5+7+8+9)

- 5 - DL + Wind + Dif Set + 100 F (Case 1 + 4 + 8 + 10)
- 6 - DL + Insulation + Vacuum + 100 F (Case 2 + 7 + 10)
- 7 - DL + Insulation + Settlement + Vacuum + 100 F (Case 2+7+8+10)
- 8 - DL + Insul + Seis x + Dif settle + Vac + 100 F (Case 2+5+7+8+10)
- 9 - DL + Insul + Seis z + Dif settle + Vac + 100 F (Case 2+5+7+8+10)

- 10 - DL + snow + Dif Set + -16 F (Case 1 + 3 + 8 + 11)
- 11 - DL + wind +Dif set -16 F (Case 1 + 4 + 8 + 11)
- 12 - DL + Insulation + Settlement + Vacuum + 100 F (Case 2+7+8+11)
- 13 - DL + Insul + Seis x + Dif settle + Vac + -15 F (Case 2+5+7+8+11)
- 14 - DL + Insul + Seis z + Dif settle + Vac + -16 F (Case 2+5+7+8+11)

SUBJECT LIGO - BEAM TUBE DESIGN Configuration 3, K = 8316 + 20% Vertical Differential Settlement = 0.579"	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 29 OF 29
	DATE 3/11/94	DATE 4-5-94	DATE	DATE	0.638

STRESS INPUT FOR 2U RUNS

2U CASE #1

DL + INSULATION + VACUUM + 0.0" DIF SETTLE + 302°
 BEAM TUBE DESIGN COMBINATION 1 (Pg 14)

$$s_a = 1533 \text{ psi}$$

$$s_b = 2195$$

$$s_d = \frac{33.5}{3762 \text{ psi}}$$

2U CASE #2

DL + INSULATION + VACUUM + AXIAL SEISMIC + 0.0" SETTLE + 302°

NOTE: AXIAL SEISMIC CAUSES HIGHER TOTAL STRESS THAN
 LATERAL SEISMIC. CAN BE SEEN BY COMPARING
 COMBINATION 3 & 4

BEAM TUBE COMBINATION 4 W/O 0.579" SETTLEMENT (Pg 17 & 11)

$$s_a = 1644 \text{ psi}$$

$$s_{b_x} = 2195 = 4267 - (y/B)(s_{b_y} B) = 4267 - .579(3579)$$

$$s_{b_y} = 402$$

$$s_b = \frac{103}{4344 \text{ psi}}$$

2U CASE #3

DL + INS + VACUUM + 0.579 SETTLEMENT + 302°

BEAM TUBE DESIGN COMBINATION 2 (Pg 15)

$$s_a = 1533$$

$$s_b = 4267$$

$$s_d = \frac{96}{5896 \text{ psi}}$$

SUBJECT Longitudinal Stress Values for API 2U Beam Tube Factor of Safety Determination	OFFICE CBI NGE-C		REVISION		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 1 OF 3
	DATE 10.MAR.94	DATE 4-5-94	DATE	DATE	7.1

ZU CASE #4

DL + INS + VACUUM + 0.0 SETTLE + 100°

BEAM TUBE DESIGN COMBINATION 6 (pg 19)

$$\begin{aligned}
 f_a &= 69 \\
 f_b &= 2195 \\
 f_d &= \frac{1}{2265 \text{ psi}}
 \end{aligned}$$

ZU CASE #5

DL + INS + VACUUM + AXIAL SEISMIC + 100°

BEAM TUBE DESIGN COMBINATION 9 - (yB x f_{bx}) (pg 21 & 11)

$$\begin{aligned}
 f_a &= 181 \\
 f_{bx} &= 2195 = 4267 - .579(3579) \\
 f_{by} &= 402 \\
 f_d &= \frac{1}{2789 \text{ psi}}
 \end{aligned}$$

ZU CASE #6

DL + INS + VACUUM + 0.25 SETTLEMENT + 100°

BEAM TUBE DESIGN COMBINATION 6 + 0.25 x f_{bx} B (pg 19 & 11)

$$\begin{aligned}
 f_a &= 69 \\
 f_{bx} &= 3090 = 2195 + 0.25(3579) \\
 f_d &= \frac{2}{3161 \text{ psi}}
 \end{aligned}$$

ZU CASE #7

DL + INS + VACUUM + .579 SETTLE + 100°

BEAM TUBE DESIGN COMBINATION 7 pg 20

$$\begin{aligned}
 f_a &= 69 \\
 f_b &= 4267 \\
 f_d &= \frac{4}{4340 \text{ psi}}
 \end{aligned}$$

SUBJECT Longitudinal Stress Values for API 2U Beam Tube Factor of Safety Determination	OFFICE CBI <u>NDG-C</u>		REVISION		REFERENCE NO. 930212
	MADE BY <u>RSW</u>	CHKD BY <u>WJC</u>	MADE BY	CHKD BY	SHT <u>2</u> OF <u>3</u>
	DATE <u>10 MAR 94</u>	DATE <u>4-5-94</u>	DATE	DATE	7.2

2U CASE #8

DL + INS + VACUUM + AXIAL SEISMIC + 0.579 SETTLE + 100°
BEAM TUBE DESIGN COMBINATION 9 (Pg 22)

$$\begin{aligned} S_a &= 151 \text{ psi} \\ S_{bx} &= 4267 \\ S_{by} &= 402 \\ S_a &= \frac{11}{4861} \end{aligned}$$

2U CASE #9

DL + INS + VACUUM + 1" SETTLEMENT + 100° (DURING ADJUSTMENT)
BEAM TUBE DESIGN COMBINATION 7 (Pg 22)

$$\begin{aligned} S_a &= 69 \text{ psi} \\ S_{bx} &= 5817 \\ S_{by} &= 0 \\ S_a &= \frac{7}{5893} \text{ psi} \end{aligned}$$

SUBJECT Longitudinal Stress Values for API 2U Beam Tube Factor of Safety Determination	OFFICE CBI 1108-C		REVISION		REFERENCE NO. 930212
	MADE BY RSW	CHKD BY WJC	MADE BY	CHKD BY	SHT <u>3</u> OF <u>3</u>
	DATE 10/11/94	DATE 4-5-94	DATE	DATE	7.3

**SUMMARY OF RESULTS -- ANALYSIS OF BEAM TUBE CONTROLLING LOAD COMBINATIONS
PER API BULLETIN 2U, "STABILITY DESIGN OF CYLINDRICAL SHELLS"**

The table below summarizes the results of the analysis of the LIGO beam tube utilizing API Bulletin 2U, "Bulletin on Stability Design of Cylindrical Shells", 1st Edition (May 1987). API Bulletin 2U provides stability criteria for measuring the structural adequacy against buckling of circular cylindrical members, either stiffened or unstiffened, when subjected to axial load, bending and external pressure acting independently or in combination.

API Bulletin 2U, unlike the ASME Code, considers the interaction between compressive axial and hoop stresses in evaluating the susceptibility of a stiffened cylinder to local buckling between stiffeners. It is for this reason that the beam tube was evaluated per the API Bulletin 2U provisions.

These results are discussed further in the section on the design of the beam tube.

API Bul 2U Case No	300F Bake	100F Oper'ng	Vacuum	Axial Seismic	Suppt Sett'nt	Tube Axial Stress, psi	Tube Hoop Stress, psi	API Bul 2U Factor of Safety	
								Actual	Recommended
1	Yes	---	Yes	---	---	- 3,762	- 2,836	1.88	2.00
2	Yes	---	Yes	Yes	---	- 4,344	- 2,836	1.78	1.50
3	Yes	---	Yes	---	Yes, 0.579"	- 5,896	- 2,836	1.55	1.50
4	---	Yes	Yes	---	---	- 2,266	- 2,836	2.25	2.00
5	---	Yes	Yes	Yes	---	- 2,789	- 2,836	2.19	1.50
6	---	Yes	Yes	---	Yes, 0.250"	- 3,161	- 2,836	2.14	2.00
7	---	Yes	Yes	---	Yes, 0.579"	- 4,340	- 2,836	1.99	2.00
8	---	Yes	Yes	Yes	Yes, 0.579"	- 4,681	- 2,836	1.91	1.50
9	---	Yes	Yes	---	Yes, 1.000"	- 5,893	- 2,836	1.77	1.50

SUBJECT	OFFICE C61 1108-c		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD BY R1W	MADE BY	CHKD BY	SHEET 1 OF 1
	DATE 3-17-94	DATE 18 MAR 94	DATE	DATE	

7.2

**API BULLETIN 2U, FIRST EDITION, MAY 1, 1987
BULLETIN ON STABILITY DESIGN OF CYLINDRICAL SHELLS**

SECTION 4

**PREDICTED SHELL BUCKLING STRESSES FOR AXIAL
LOAD, BENDING AND EXTERNAL PRESSURE**

					Ring Stiffener
Lr =	30.00 in.	b =	0.00 in.	tw =	0.1875 in.
R =	24.439 in.	Fy =	19,200 psi	Ar =	0.3280 sq.in.
t =	0.127 in.	E =	27,000,000 psi	Zr =	0.9385 in.
(Rt) ^{.5} =	1.762 in.	Rt =	192.43	lr =	0.08374 in. ^{.4}
Mx = Lr/(Rt) ^{.5} =	17.03 (eq. 4.1)			Rr =	25.377 in.
Mc = b/(Rt) ^{.5} =	0.00 (eq. 4.1)			Le = [1.56(Rt) ^{.5} + tw =	2.936 in.
Nx =	477.77 #/in.	No =	360.18 #/in.		
	= 3,762 psi (Axial)		= 2,836 psi (Hoop)		

Section 4.1 Local Buckling of Unstiffened or Ring Stiffened Cylinders

Section 4.1.1 Axial Compression or Bending (No = 0)

a. Elastic Buckling Stresses

$F_{xeL} = (axL)(Cx)(E)(t/R)$ (eq. 4-2)

Cx = 0.630	if Mx ≤ 1.5	(eq. 4-3)
Cx = $[0.904/(Mx)^2] + 0.1013(Mx)^2 = 2.941$	if 1.5 < Mx < 1.73	
Cx = 0.605	if Mx > 1.73	

Mx = 17.03 → Cx = 0.605

axL = 0.207	if R/t ≥ 610	(eq. 4-4)
axL = $169c/(195 + R/t) < 0.9 = 0.436$	if R/t < 610	

R/t = 192 → axL = 0.436

c = 2.64	if Mx ≤ 1.5	(eq. 4-5)
c = $3.13/(Mx)^{0.42} = 0.952$	if 1.5 < Mx ≤ 15	
c = 1.0	if Mx > 15	

Mx = 17.03 → c = 1.000

$F_{xeL} = (axL)(Cx)(E)(t/R) = 37,029 \text{ psi}$ (eq. 4-2)

b. Inelastic Buckling Stresses

Smaller of:

$F_{xcL} = F_{xeL} = 37,029 \text{ psi}$ eq. 4-6

$F_{xcL} = 233(Fy)/(166 + R/t) = 12,481 \text{ psi}$	if R/t < 300	(eq. 4-7)
$F_{xcL} = 0.5(Fy) = 9,600 \text{ psi}$	if R/t ≥ 300	

R/t = 192 → $F_{xcL} = 12,481 \text{ psi}$

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 300F Bakeout + Vacuum, No Seismic No Support Settlement	OFFICE: NOE	REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD Rjw	MADE BY	CHKD BY
	DATE 3/9/94	DATE 18 MAR 94	DATE	DATE
	SHT 1 OF 6			7.5

Section 4.1.2 External Pressure (Nx/No = 0 or 0.5)

Use Nx/No = 0
 Nx/No = 0 for radial pressure
 Nx/No = 0.5 for hydrostatic pressure

a. Elastic Buckling Stresses

F_{reL} or $F_{heL} = (a_c L)(P_{eL})(R)/(K_o L)^2$ (eq. 4-8)

$K_o L = 1.0$	if	$M_x \geq 3.42$
$K_o L = 1 - e^y = 1.000$	if	$M_x < 3.42$ (eq. 11-7)
$M_x = 17.03$	\implies	$K_o L = 1.000$

$e = (1 - 0.3k)/[1 + (L_e)/V A]$
$A = (A_r)(R/R_r)^2 = 0.304196$ sq.in.
$A_r = 0.328$ sq.in.
$R_r = 25.377$ in.
$L_e = [1.56(R_r)^{.5}] + t_w = 2.935797$ in.
$k = N_x/No = 0$
$e = 0.449$

$y = 1.0$	if	$M_x \leq 1.26$
$y = 1.58 - 0.46 M_x = -6.25$	if	$1.26 < M_x < 3.42$
$y = 0.0$	if	$M_x \geq 3.42$
$M_x = 17.03$	\implies	$y = 0.000$

$P_{eL} = [1.27 (E) (V/R)^2]/[A^{1.18} + 0.5] = 34.8387$	if	$M_x > 1.5$ & $A < 2.5$	$A = M_x - 1.17 + 1.068k = 15.859$
$P_{eL} = [0.92 (E) (V/R)^2]/A = 42.3001$	if	$2.5 < A < 0.208 R/t$	$M_x = 17.03$
$P_{eL} = 0.836(C_p)^{-1.061} (E)(V/R)^3 = 44.7594$	if	$0.208 < C_p < 2.85$	$k = 0$
$P_{eL} = 0.275 (E)(V/R)^3 = 1.0420$	if	$C_p > 2.85$	$0.208 R/t = 40.02526$
$P_{eL} = 42.3001$			$C_p = A/(R/t) = 0.082413$

b. Imperfection Factors

$a_o L = 0.8$

F_{reL} or $F_{heL} = (a_o L)(P_{eL})(R)/(K_o L)^2 = 6.512$ psi (eq. 4-8)

c. Inelastic Buckling Stresses

$F_{rcL} = n (F_{reL})$ (eq. 4-10)

$F_{hcL} = n (F_{heL})$ (eq. 4-11)

$n = 1.0$	if	$d \leq 0.55$	$d = F_{reL}/F_y = 0.3392$
$n = 0.45/d + 0.18 = 1.51$	if	$0.55 < d \leq 1.6$	$n = 1.000$ (eq. 5-3)
$n = 1.31/(1 + 1.15d) = 0.94$	if	$1.6 < d < 6.25$	
$n = 1/d = 2.95$	if	$d \geq 6.25$	

$F_{rcL} = n (F_{reL}) = 6.512$ psi (eq. 4-10)

d. Failure Pressures

$P_{oL} = (n)(a_o L)(P_{eL}) = 33.840$ psi (eq. 4-12)

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 300F Bakeout + Vacuum, No Seismic No Support Settlement	OFFICE: NOE	REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD (2)W	MADE BY	CHKD BY
	DATE 3/9/94	DATE 10/11/94	DATE	DATE
				SHT 2 OF 6 7.6

**API BULLETIN 2U, FIRST EDITION, MAY 1, 1987
BULLETIN ON STABILITY DESIGN OF CYLINDRICAL SHELLS**

SECTION 4

**PREDICTED SHELL BUCKLING STRESSES FOR AXIAL
LOAD, BENDING AND EXTERNAL PRESSURE**

Ring Stiffener

Lr =	30.00 in.	b =	0.00 in.	tw =	0.1875 in.
R =	24.439 in.	Fy =	19,200 psi	Ar =	0.3280 sq.in.
t =	0.127 in.	E =	27,000,000 psi	Zr =	0.9385 in.
(Rt) ^{.5} =	1.762 in.	R/t =	192.43	lr =	0.08374 in. ^{.4}
Mx = Lr/(Rt) ^{.5} =	17.03 (eq. 4.1)			Rr =	25.377 in.
Mc = b/(Rt) ^{.5} =	0.00 (eq. 4.1)			Le = [1.56(Rt) ^{.5} + tw =	2.936 in.
Nx =	477.77 #/in.	No =	360.18 #/in.		
	= 3,762 psi (Axial)		= 2,836 psi (Hoop)		

Section 4.1 Local Buckling of Unstiffened or Ring Stiffened Cylinders

Section 4.1.1 Axial Compression or Bending (No = 0)

a. Elastic Buckling Stresses

$F_{xeL} = (axL)(C_x)(E)(t/R)$ (eq. 4-2)

Cx = 0.630	if Mx ≤ 1.5	(eq. 4-3)
Cx = [0.904/(Mx) ²] + 0.1013(Mx) ² = 2.941	if 1.5 < Mx < 1.73	
Cx = 0.605	if Mx > 1.73	

Mx = 17.03 → Cx = 0.605

axL = 0.207	if R/t ≥ 610	(eq. 4-4)
axL = 169c/(195 + R/t) < 0.9 = 0.436	if R/t < 610	

R/t = 192 → axL = 0.436

c = 2.64	if Mx ≤ 1.5	(eq. 4-5)
c = 3.13/(Mx) ^{0.42} = 0.952	if 1.5 < Mx ≤ 15	
c = 1.0	if Mx > 15	

Mx = 17.03 → c = 1.000

$F_{xeL} = (axL)(C_x)(E)(t/R) = 37,029 \text{ psi}$ (eq. 4-2)

b. Inelastic Buckling Stresses

Smaller of:

F_{xcl} = F_{xel} = 37,029 psi eq. 4-6

F _{xcl} = 233(Fy)/(166 + R/t) = 12,481 psi	if R/t < 300	(eq. 4-7)
F _{xcl} = 0.5(Fy) = 9,600 psi	if R/t ≥ 300	

R/t = 192 → F_{xcl} = 12,481 psi

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 300F Bakeout + Vacuum, No Seismic No Support Settlement	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RJW	MADE BY	CHKD BY	SHT 1 OF 6
	DATE 3/9/94	DATE 18 MAR 94	DATE	DATE	7.5

Section 4.1.2 External Pressure (Nx/No = 0 or 0.5)

Use Nx/No = 0

Nx/No = 0 for radial pressure

Nx/No = 0.5 for hydrostatic pressure

a. Elastic Buckling Stresses

F_{reL} or $F_{heL} = (a_c L)(P_{eL})(R)/(K_o L)^2$ (eq. 4-8)

$K_o L = 1.0$	if	$M_x \geq 3.42$
$K_o L = 1 - e^* y = 1.000$	if	$M_x < 3.42$ (eq. 11-7)
$M_x = 17.03$	\implies	$K_o L = 1.000$

$e = (1 - 0.3k)/[1 + (L_e)/A]$
$A = (A_r)(R/R_r)^2 = 0.304196$ sq.in.
$A_r = 0.328$ sq.in.
$R_r = 25.377$ in.
$L_e = [1.56(R_r)^5] + t_w = 2.935797$ in.
$k = N_x/No = 0$
$e = 0.449$

$y = 1.0$	if	$M_x \leq 1.26$
$y = 1.58 - 0.46 M_x = -6.25$	if	$1.26 < M_x < 3.42$
$y = 0.0$	if	$M_x \geq 3.42$
$M_x = 17.03$	\implies	$y = 0.000$

$P_{eL} = [1.27 (E) (V/R)^2]/[A^{1.18} + 0.5] = 34.8387$	if	$M_x > 1.5$ & $A < 2.5$	$A = M_x - 1.17 + 1.068k = 15.859$
$P_{eL} = [0.92 (E) (V/R)^2]/A = 42.3001$	if	$2.5 < A < 0.208 R/t$	$M_x = 17.03$
$P_{eL} = 0.836(C_p)^{-1.061} (E)(V/R)^3 = 44.7594$	if	$0.208 < C_p < 2.85$	$k = 0$
$P_{eL} = 0.275 (E)(V/R)^3 = 1.0420$	if	$C_p > 2.85$	$0.208 R/t = 40.02526$
$P_{eL} = 42.3001$			$C_p = A/(R/t) = 0.082413$

b. Imperfection Factors

$a_o L = 0.8$

F_{reL} or $F_{heL} = (a_c L)(P_{eL})(R)/(K_o L)^2 = 6,512$ psi (eq. 4-8)

c. Inelastic Buckling Stresses

$F_{rcL} = n (F_{reL})$ (eq. 4-10)

$F_{hcL} = n (F_{heL})$ (eq. 4-11)

$n = 1.0$	if	$d \leq 0.55$	$d = F_{reL}/F_y = 0.3392$
$n = 0.45/d + 0.18 = 1.51$	if	$0.55 < d \leq 1.6$	$n = 1.000$ (eq. 5-3)
$n = 1.31/(1 + 1.15d) = 0.94$	if	$1.6 < d < 6.25$	
$n = 1/d = 2.95$	if	$d \geq 6.25$	

$F_{rcL} = n (F_{reL}) = 6,512$ psi (eq. 4-10)

d. Failure Pressures

$P_o L = (n)(a_c L)(P_{eL}) = 33,640$ psi (eq. 4-12)

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 300F Bakeout + Vacuum, No Seismic No Support Settlement	OFFICE: NOE	REVISION	REFERENCE NO. 930212
	MADE BY WJC	CHKD RJCW	MADE BY CHKD BY
	DATE 3/9/94	DATE 19 MAR 94	DATE DATE
	SHT 2 OF 6 7.6		

Section 4.2 General Instability of Ring Stiffened Cylinders

Section 4.2.1 Axial Compression or Bending (No = 0)

a. Elastic Buckling Stresses

$F_{xeG} = (axG)(0.605)(E)(t/R)(1 + A'r)^{0.5}$ (eq. 4-13)

$A'r = Ar/[Lr] t = 0.0861$ $Ar = \text{ring area} = 0.328 \text{ sq.in.}$
 $Lr = \text{ring spacing} = 30 \text{ in.}$

b. Imperfection Factors

$axG = 0.72$	if $A'r \geq 0.2$	(eq. 4-14)
$axG = [3.6 - 5.0(axL)] (A'r) + axL = 0.558$	if $0.06 < A'r < 0.2$	
$axG = axL = 0.436209$	if $A'r \leq 0.06$	
$axG = 0.558$	axL from eq. 4-4 with $c = 1.0$	

$F_{xeG} = (axG)(0.605)(E)(t/R)(1 + A'r)^{0.5} = 49,397 \text{ psi}$ (eq. 4-13)

c. Inelastic Buckling Stresses

$F_{xcG} = n (F_{xeG})$ (eq. 4-15)

$n = 1.0$	if $d \leq 0.55$	(eq. 5-3)
$n = 0.45/d + 0.18 = 0.35$	if $0.55 < d \leq 1.6$	
$n = 1.31/(1 + 1.15d) = 0.33$	if $1.6 < d < 6.25$	
$n = 1/d = 0.39$	if $d \geq 6.25$	
$d = F_{xeG}/F_y = 2.5728$		$n = 0.331$

$F_{xcG} = n (F_{xeG}) = 16,346 \text{ psi}$ (eq. 4-15)

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 300F Bakeout + Vacuum, No Seismic No Support Settlement	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD R1W	MADE BY	CHKD BY	SHT 3 OF 6
	DATE 3/9/94	DATE 18 MAR 94	DATE	DATE	7.7

Section 4.2.2 External Pressure (Nx/No = 0 or 0.5)

Use Nx/No = 0

Nx/No = 0 for radial pressure

Nx/No = 0.5 for hydrostatic pressure

a. Buckling Stresses With or Without End Pressure

$FreG$ or $FheG = (acG)(PeG)(Ro)(KoG)/t$ (eq. 4-16)

$KoG = (1 - 0.3k)(Le)(t)/[(Ar + (Le)(t))] = 0.531994$ (eq. 11-9)

$Ar = 0.328$ sq.in.
 $Le = [1.56(Rt)^{.5}] + tw = 2.935797$ in.
 $k = 0$

$PeG = \frac{(E)(t/R)(Lg)^4}{[N^2 + k(Lg)^2 - 1][N^2 + (Lg)^2]^2} + \frac{(E)(ler)(N^2 - 1)}{(Lr)(Rc)^2(R)}$ (eq. 4-17)

$\lambda G = (\pi)(R)/Lb = 2.559194$ $Lb = 30$ in.

$k = 0$

$ler = lr + (Ar)(Zr)^2 (Le)(t)/[(Ar + (Le)(t))] + (Le)(t)^3/12 = 0.233$ in.^4 (eq. 4-18)

$lr = 0.08374$ in.^4
 $Ar = 0.328$ sq.in.
 $Zr = 0.9385$ in.
 $t = 0.127$ in.
 $Rr = 25.3767$ in.

$Le = 1.1(Dt)^{0.5} = 2.741$ in. if $Mx > 1.56$
 $Le = Lr = 30.000$ in. if $Mx < 1.56$
 $Mx = 17.03 \implies Le = 2.741$ in.

$N = 4$ use N that gives PeG min. $2 < N$

$PeG = 988.937$ psi (eq. 4-17)

b. Imperfection Factors

$aoG = 0.8$

$FreG$ or $FheG = (aoG)(PeG)(Ro)(KoG)/t = 81,201$ psi (eq. 4-16)

c. Inelastic Buckling Stresses

$FrcG = n(FreG)$ (eq. 4-19)

$FhcG = n(FheG)$ (eq. 4-20)

$n = 1.0$	if $d < 0.55$	$d = FreG/Fy = 4.2292$	(eq. 5-3)
$n = 0.45/d + 0.18 = 0.286$	if $0.55 < d < 1.6$	$n = 0.223$	
$n = 1.31/(1 + 1.15d) = 0.223$	if $1.6 < d < 6.25$		
$n = 1/d = 0.236$	if $d > 6.25$		

$FrcG = n(FreG) = 18,141$ psi (eq. 4-10)

d. Failure Pressures

$poG = (n)(aoG)(peG) = 176,752$ psi (eq. 4-12)

SUBJECT LIGO Beam Tube -- 49.00' OD x 0.127' Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 300F Bakeout + Vacuum, No Seismic No Support Settlement	OFFICE: NOE	REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD R JW	MADE BY	CHKD BY
	DATE 3/9/94	DATE 10/11/99	DATE	DATE
				SHT 4 OF 6 7.9

SECTION 6 PREDICTED SHELL BUCKLING STRESSES FOR COMBINED LOADS

Section 6.3 Axial Compression, Bending and Hoop Compression

$(Ra)^2 - c(Ra)(Rh) + (Rh)^2 = 1.0$ (eq. 6-3)

$Ra = F_{icj} / F_{xcj}$ $Rh = F_{ocj} / F_{rcj}$

a. Unstiffened and Ring Stiffened Cylinders (all buckling modes)

$c = (F_{xcj} + F_{rcj}) / F_y - 1.0$ (eq. 6-4)

Local Buckling

$c = [F_{xcL} + F_{rcL}] / F_y - 1 = -0.01078$

Let $F_{icL} = (F_{ocL})(k)(K_{xL}/K_{oL}) = 1.3265 (F_{ocL})$

$Ra = F_{icL} / F_{xcL} = 0.000106 (F_{ocL})$

$K_{xL} = 1.0$

$Rh = F_{ocL} / F_{rcL} = 0.000154 (F_{ocL})$

$K_{oL} = 1.0$ if $M_x \geq 3.42$
 $K_{oL} = 1 - e^*y = 1.000$ if $M_x < 3.42$ (eq. 11-7)
 $M_x = 17.03 \implies K_{oL} = 1.000$

$e = (1 - 0.3k) / [1 + (Le)/VA]$
 $A = (Ar)(R/Rr)^2 = 0.304196 \text{ sq.in.}$
 $Ar = 0.328 \text{ sq.in.}$
 $Rr = 25.377 \text{ in.}$
 $Le = [1.56(Rt)^{.5}] + tw = 2.748297 \text{ in.}$
 $k = N_x / N_o = 1.326$
 $e = 0.280$

Solve eq. 6-3 for F_{ocL}

$F_{ocL} = 5.341 \text{ psi}$ (eq. 6-3)

$F_{icL} = (F_{ocL})(k)(K_{xL}/K_{oL}) = 7.085 \text{ psi}$

General Instability

$c = [F_{xcG} + F_{rcG}] / F_y - 1 = 0.796234$

$K_{xG} = 1.0$

$K_{oG} = (1 - 0.3k)(Le)(t) / [(Ar) + (Le)(t)] = 0.3104$ (eq. 11-9)
 (If $K_{oG} < 0$ then ring is in tension and the interaction equation is not applicable. Then $F_{icG} = F_{xcG}$)

$k = N_x / N_o = 1.326$
 $Ar = 0.328 \text{ sq.in.}$
 $Le = [1.56(Rt)^{.5}] + tw = 2.748297 \text{ in.}$

Let $F_{icG} = (F_{ocG})(k)(K_{xG}/K_{oG}) = 4.273774 (F_{ocG})$

$Ra = F_{icG} / F_{xcG} = 0.000261 (F_{ocG})$

$Rh = F_{ocG} / F_{rcG} = 5.51E-05 (F_{ocG})$

Solve eq. 6-3 for F_{ocG}

$F_{ocG} = 4.085 \text{ psi}$ (eq. 6-3)

$F_{icG} = (F_{ocG})(k)(K_{xG}/K_{oG}) = 17.459 \text{ psi}$

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 300F Bakeout + Vacuum, No Seismic No Support Settlement	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD Rjw	MADE BY	CHKD BY	SHT 5 OF 6
	DATE 3/9/94	DATE 18 MAR 94	DATE	DATE	7.9

SECTION 9 ALLOWABLE STRESSES

Section 9.1 Allowable Stresses For Shell Buckling Mode

Section 9.1.5 Axial Compression and Hoop Compression and Axial Compression, Bending and Hoop Compression

$F_a = F_b = F_{icL} / FS$
(eq. 9-7)

$F_o = F_{ocL} / FS$

Factor of Safety (FS) = 1.88

F_a = F_b = 3,769 psi = 478.61 #/in.

N_x = 477.77 #/in.

F_o = 2,841 psi = 360.81 #/in.

N_o = 360.18 #/in.

SECTION 11 STRESS CALCULATIONS

Section 11.3 Hoop Stresses

Section 11.3(b)2 Stress in a Ring Stiffener

Ring Stiffener Stress = $(p * R_o / t) * (1.00 - 0.30 * (N_x / N_o)) * (L_e * t) / (A_r + L_e * t)$ (Eq 11-8)

Solving Eq 11-8,

=> Ring Stiffener Stress = 1,941 psi

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 300F Bakeout + Vacuum, No Seismic No Support Settlement	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RSW	MADE BY	CHKD BY	SHT 6 OF 6
	DATE 3/9/94	DATE 10MAR94	DATE	DATE	7.10

**API BULLETIN 2U, FIRST EDITION, MAY 1, 1987
BULLETIN ON STABILITY DESIGN OF CYLINDRICAL SHELLS**

SECTION 4

**PREDICTED SHELL BUCKLING STRESSES FOR AXIAL
LOAD, BENDING AND EXTERNAL PRESSURE**

Ring Stiffener

Lr =	30.00 in.	b =	0.00 in.	tw =	0.1875 in.
R =	24.439 in.	Fy =	19,200 psi	Ar =	0.3280 sq.in.
t =	0.127 in.	E =	27,000,000 psi	Zr =	0.9385 in.
(Rt) ^{1.5} =	1.762 in.	R/t =	192.43	lr =	0.08374 in. ^{1.4}
Mx = Lr/(Rt) ^{1.5} =	17.03 (eq. 4.1)			Rr =	25.377 in.
Mc = b/(Rt) ^{1.5} =	0.00 (eq. 4.1)			Le = [1.56(Rt) ^{1.5}] + tw =	2.936 in.
Nx = 551.69 #/in.		No = 360.18 #/in.			
= 4,344 psi (Axial)		= 2,836 psi (Hoop)			

Section 4.1 Local Buckling of Unstiffened or Ring Stiffened Cylinders

Section 4.1.1 Axial Compression or Bending (No = 0)

a. Elastic Buckling Stresses

$F_{xEL} = (axL)(Cx)(E)(t/R)$ (eq. 4-2)

Cx = 0.630	if Mx ≤ 1.5	(eq. 4-3)
Cx = [0.904/(Mx) ²] + 0.1013(Mx) ² = 2.941	if 1.5 < Mx < 1.73	
Cx = 0.605	if Mx > 1.73	

Mx = 17.03 → Cx = 0.605

axL = 0.207	if R/t ≥ 610	(eq. 4-4)
axL = 169c/(195 + R/t) < 0.9 = 0.436	if R/t < 610	

R/t = 192 → axL = 0.436

c = 2.64	if Mx ≤ 1.5	(eq. 4-5)
c = 3.13/(Mx) ^{0.42} = 0.952	if 1.5 < Mx ≤ 15	
c = 1.0	if Mx > 15	

Mx = 17.03 → c = 1.000

$F_{xEL} = (axL)(Cx)(E)(t/R) = 37,029 \text{ psi}$ (eq. 4-2)

b. Inelastic Buckling Stresses

Smaller of:

$F_{xCL} = F_{xEL} = 37,029 \text{ psi}$ eq. 4-6

$F_{xCL} = 233(Fy)/(166 + R/t) ≤ Fy = 12,481 \text{ psi}$	if R/t < 300	(eq. 4-7)
$F_{xCL} = 0.5(Fy) = 9,600 \text{ psi}$	if R/t ≥ 300	

R/t = 192 → $F_{xCL} = 12,481 \text{ psi}$

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 300F Bakeout + Vacuum + Axial Seismic No Support Settlement	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD R/N	MADE BY	CHKD BY	SHT 1 OF 6
	DATE 3/10/94	DATE 10/MAR/94	DATE	DATE	7.11

Section 4.1.2 External Pressure (Nx/No = 0 or 0.5)

Use Nx/No = 0
 Nx/No = 0 for radial pressure
 Nx/No = 0.5 for hydrostatic pressure

a. Elastic Buckling Stresses

F_{reL} or $F_{heL} = (acL)(PeL)(R)(KoL)/t$ (eq. 4-8)

$KoL = 1.0$	if	$Mx \geq 3.42$
$KoL = 1 - e^*y = 1.000$	if	$Mx < 3.42$ (eq. 11-7)
$Mx = 17.03$	====>	$KoL = 1.000$

$e = (1 - 0.3k)/[1 + (Le)/VA]$
$A = (Ar)(R/Rr)^2 = 0.304196$ sq.in.
$Ar = 0.328$ sq.in.
$Rr = 25.377$ in.
$Le = [1.56(Rt)^{.5}] + tw = 2.935797$ in.
$k = Nx/No = 0$
$e = 0.449$

$y = 1.0$	if	$Mx \leq 1.26$
$y = 1.58 - 0.46 Mx = -6.25$	if	$1.26 < Mx \leq 3.42$
$y = 0.0$	if	$Mx \geq 3.42$
$Mx = 17.03$	====>	$y = 0.000$

$PeL = [1.27 (E) (VR)^2]/[A^{1.18} + 0.5] = 34.8387$	if	$Mx > 1.5$ & $A < 2.5$	$A = Mx - 1.17 + 1.068k = 15.859$
$PeL = [0.92 (E) (VR)^2]/A = 42.3001$	if	$2.5 < A < 0.208 R/t$	$Mx = 17.03$
$PeL = 0.836(Cp)^{-1.061} (E)(VR)^3 = 44.7594$	if	$0.208 < Cp < 2.85$	$k = 0$
$PeL = 0.275 (E)(VR)^3 = 1.0420$	if	$Cp > 2.85$	$0.208 R/t = 40.02526$
$PeL = 42.3001$			$Cp = A/(R/t) = 0.082413$

b. Imperfection Factors

$aoL = 0.8$

F_{reL} or $F_{heL} = (aoL)(PeL)(R)(KoL)/t = 6.512$ psi (eq. 4-8)

c. Inelastic Buckling Stresses

$F_{rcL} = n (F_{reL})$ (eq. 4-10)

$F_{hcL} = n (F_{heL})$ (eq. 4-11)

$n = 1.0$	if	$d \leq 0.55$	$d = F_{reL}/F_y = 0.3392$
$n = 0.45/d + 0.18 = 1.51$	if	$0.55 < d \leq 1.6$	$n = 1.000$ (eq. 5-3)
$n = 1.31/(1 + 1.15d) = 0.94$	if	$1.6 < d < 6.25$	
$n = 1/d = 2.95$	if	$d \geq 6.25$	

$F_{rcL} = n (F_{reL}) = 6.512$ psi (eq. 4-10)

d. Failure Pressures

$p_{oL} = (n)(aoL)(peL) = 33.840$ psi (eq. 4-12)

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 300F Bakeout + Vacuum + Axial Seismic No Support Settlement	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD (2) W	MADE BY	CHKD BY	SHT 2 OF 6
	DATE 3/10/94	DATE 10/18/94	DATE	DATE	7.12

Section 4.2 General Instability of Ring Stiffened Cylinders

Section 4.2.1 Axial Compression or Bending (No = 0)

a. Elastic Buckling Stresses

$F_{xeG} = (axG)(0.605)(E)(t/R)(1 + A'r)^{0.5}$ (eq. 4-13)

$A'r = Ar/[Lr] t = 0.0861$ $Ar = \text{ring area} = 0.328 \text{ sq.in.}$
 $Lr = \text{ring spacing} = 30 \text{ in.}$

b. Imperfection Factors

$axG = 0.72$	if $A'r \geq 0.2$	(eq. 4-14)
$axG = [3.6 - 5.0(axL)] (A'r) + axL = 0.558$	if $0.06 < A'r < 0.2$	
$axG = axL = 0.436209$	if $A'r \leq 0.06$	
$axG = 0.558$	axL from eq. 4-4 with $c = 1.0$	

$F_{xeG} = (axG)(0.605)(E)(t/R)(1 + A'r)^{0.5} = 49,397 \text{ psi}$ (eq. 4-13)

c. Inelastic Buckling Stresses

$F_{xcG} = n (F_{xeG})$ (eq. 4-15)

$n = 1.0$	if $d \leq 0.55$	(eq. 5-3)
$n = 0.45/d + 0.18 = 0.35$	if $0.55 < d \leq 1.6$	
$n = 1.31/(1 + 1.15d) = 0.33$	if $1.6 < d < 6.25$	
$n = 1/d = 0.39$	if $d \geq 6.25$	
		$d = F_{xeG}/F_y = 2.5728$
		$n = 0.331$

$F_{xcG} = n (F_{xeG}) = 16,346 \text{ psi}$ (eq. 4-15)

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 300F Bakeout + Vacuum + Axial Seismic No Support Settlement	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RJW	MADE BY	CHKD BY	SHT 3 OF 6
	DATE 3/10/94	DATE 18 MAR 94	DATE	DATE	
					7.13

Section 4.2.2 External Pressure (Nx/No = 0 or 0.5)

Use Nx/No = 0
 Nx/No = 0 for radial pressure
 Nx/No = 0.5 for hydrostatic pressure

a. Buckling Stresses With or Without End Pressure

$FreG$ or $FheG = (acG)(PeG)(Ro)(KoG)/t$ (eq. 4-16)

$KoG = (1 - 0.3k)(Le)(t)/[(Ar + (Le)(t))] = 0.531994$ (eq. 11-9)

$Ar = 0.328$ sq.in.
$Le = [1.56(Rt)^{0.5}] + tw = 2.935797$ in.
$k = 0$

$PeG = \frac{(E)(t/R)(Lg)^4}{[N^2 + k(Lg)^2 - 1][N^2 + (Lg)^2]^2} + \frac{(E)(ler)(N^2 - 1)}{(Lr)(Rc)^2(R)}$ (eq. 4-17)

$\lambda G = (\pi)(R)/Lb = 2.559194$ $Lb = 30$ in.

$k = 0$

$ler = lr + (Ar)(Zr)^2 (Le)(t)/[(Ar + (Le)(t))] + (Le)(t)^3/12 = 0.233$ in.⁴ (eq. 4-18)

- $lr = 0.08374$ in.⁴
- $Ar = 0.328$ sq.in.
- $Zr = 0.9385$ in.
- $t = 0.127$ in.
- $Rr = 25.3767$ in.

$Le = 1.1(Dt)^{0.5} = 2.741$ in.	if $Mx > 1.56$
$Le = Lr = 30.000$ in.	if $Mx < 1.56$
$Mx = 17.03$ \implies	$Le = 2.741$ in.

$N = 4$ use N that gives PeG min. $2 < N$

$PeG = 988.937$ psi (eq. 4-17)

b. Imperfection Factors

$aoG = 0.8$

$FreG$ or $FheG = (aoG)(PeG)(Ro)(KoG)/t = 31,201$ psi (eq. 4-16)

c. Inelastic Buckling Stresses

$FrcG = n(FreG)$ (eq. 4-19)

$FhcG = n(FheG)$ (eq. 4-20)

$n = 1.0$	if $d \leq 0.55$	$d = FreG/Fy = 4.2292$	(eq. 5-3)
$n = 0.45/d + 0.18 = 0.286$	if $0.55 < d \leq 1.6$	$n = 0.223$	
$n = 1.31/(1 + 1.15d) = 0.223$	if $1.6 < d < 6.25$		
$n = 1/d = 0.236$	if $d \geq 6.25$		

$FrcG = n(FreG) = 18,141$ psi (eq. 4-10)

d. Failure Pressures

$poG = (n)(aoG)(peG) = 176,752$ psi (eq. 4-12)

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 300F Bakeout + Vacuum + Axial Seismic No Support Settlement	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RWN	MADE BY	CHKD BY	SHT 4 OF 6
	DATE 3/10/94	DATE 10/11/94	DATE	DATE	7.14

SECTION 6 PREDICTED SHELL BUCKLING STRESSES FOR COMBINED LOADS

Section 6.3 Axial Compression, Bending and Hoop Compression

$(Ra)^2 - c (Ra)(Rh) + (Rh)^2 = 1.0$ (eq. 6-3)

$Ra = F_{icj} / F_{xcj}$ $Rh = F_{ocj} / F_{rcj}$

a. Unstiffened and Ring Stiffened Cylinders (all buckling modes)

$c = (F_{xcj} + F_{rcj}) / F_y - 1.0$ (eq. 6-4)

Local Buckling

$c = [F_{xcL} + F_{rcL}] / F_y - 1 = -0.01078$

Let $F_{icL} = (F_{ocL}) (k) (K_{xL} / K_{oL}) = 1.5317 (F_{ocL})$

$Ra = F_{icL} / F_{xcL} = 0.000123 (F_{ocL})$

$K_{xL} = 1.0$

$Rh = F_{ocL} / F_{rcL} = 0.000154 (F_{ocL})$

$K_{oL} = 1.0$ if $M_x \geq 3.42$
 $K_{oL} = 1 - e^y = 1.000$ if $M_x < 3.42$ (eq. 11-7)
 $M_x = 17.03 \implies K_{oL} = 1.000$

$e = (1 - 0.3k)[1 + (L_e)/A]$
 $A = (A_r)(R/R_r)^2 = 0.304196$ sq.in.
 $A_r = 0.328$ sq.in.
 $R_r = 25.377$ in.
 $L_e = [1.56(R/t)^{.5}] + t_w = 2.748297$ in.
 $k = N_x / N_o = 1.532$
 $e = 0.252$

Solve eq. 6-3 for F_{ocL}

$F_{ocL} = 5,074$ psi (eq. 6-3)

$F_{icL} = (F_{ocL}) (k) (K_{xL} / K_{oL}) = 7,771$ psi

General Instability

$c = [F_{xcG} + F_{rcG}] / F_y - 1 = 0.796234$

$K_{xG} = 1.0$

$K_{oG} = (1 - 0.3k)(L_e)(t) / [A_r + (L_e)(t)] = 0.2786$ (eq. 11-9)
 (If $K_{oG} < 0$ then ring is in tension and the interaction equation is not applicable. Then $F_{icG} = F_{xcG}$)

$k = N_x / N_o = 1.532$
 $A_r = 0.328$ sq.in.
 $L_e = [1.56(R/t)^{.5}] + t_w = 2.748297$ in.

Let $F_{icG} = (F_{ocG}) (k) (K_{xG} / K_{oG}) = 5.497063 (F_{ocG})$

$Ra = F_{icG} / F_{xcG} = 0.000336 (F_{ocG})$

$Rh = F_{ocG} / F_{rcG} = 5.51E-05 (F_{ocG})$

Solve eq. 6-3 for F_{ocG}

$F_{ocG} = 3,141$ psi (eq. 6-3)

$F_{icG} = (F_{ocG}) (k) (K_{xG} / K_{oG}) = 17,266$ psi

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 300F Bakeout + Vacuum + Axial Seismic No Support Settlement	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RIN	MADE BY	CHKD BY	SHT 5 OF 6
	DATE 3/10/94	DATE 18 MAR 94	DATE	DATE	
					7.15

SECTION 9 ALLOWABLE STRESSES

Section 9.1 Allowable Stresses For Shell Buckling Mode

Section 9.1.5 Axial Compression and Hoop Compression and Axial Compression, Bending and Hoop Compression

$F_a = F_b = F_{iCL} / FS$
(eq. 9-7)

$F_o = F_{oCL} / FS$

Factor of Safety (FS) = 1.78

$F_a = F_b = 4,366 \text{ psi} = 554.47 \text{ #/in.}$

$N_x = 551.69 \text{ #/in.}$

$F_o = 2,850 \text{ psi} = 362.00 \text{ #/in.}$

$N_o = 360.18 \text{ #/in.}$

SECTION 11 STRESS CALCULATIONS

Section 11.3 Hoop Stresses

Section 11.3(b)2 Stress in a Ring Stiffener

Ring Stiffener Stress = $(p * R_o / t) * (1.00 - 0.30 * (N_x / N_o)) * (L_e * t) / (A_r + L_e * t)$ (Eq 11-8)

Solving Eq 11-8,

==> Ring Stiffener Stress = 1,742 psi

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 300F Bakeout + Vacuum + Axial Seismic No Support Settlement	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RW	MADE BY	CHKD BY	SHT 6 OF 6
	DATE 3/10/94	DATE 1/11/94	DATE	DATE	7.1b

**API BULLETIN 2U, FIRST EDITION, MAY 1, 1987
BULLETIN ON STABILITY DESIGN OF CYLINDRICAL SHELLS**

SECTION 4

**PREDICTED SHELL BUCKLING STRESSES FOR AXIAL
LOAD, BENDING AND EXTERNAL PRESSURE**

Ring Stiffener

Lr =	30.00	in.	b =	0.00	in.	tw =	0.1875	in.
R =	24.439	in.	Fy =	19,200	psi	Ar =	0.3280	sq.in.
t =	0.127	in.	E =	27,000,000	psi	Zr =	0.9385	in.
(Rt) ^{.5} =	1.762	in.	R/t =	192.43		lr =	0.08374	in. ^{.4}
Mx = Lr/(Rt) ^{.5} =	17.03	(eq. 4.1)				Rr =	25.377	in.
Mc = b/(Rt) ^{.5} =	0.00	(eq. 4.1)				Le = [1.56(Rt) ^{.5} + tw =	2.936	in.
Nx = 748.79 #/in.			No = 360.18 #/in.					
= 5,896 psi (Axial)			= 2,836 psi (Hoop)					

Section 4.1 Local Buckling of Unstiffened or Ring Stiffened Cylinders

Section 4.1.1 Axial Compression or Bending (No = 0)

a. Elastic Buckling Stresses

$F_{xeL} = (axL)(Cx)(E)(t/R)$ (eq. 4-2)

Cx = 0.630	if Mx ≤ 1.5
Cx = [0.904/(Mx) ²] + 0.1013(Mx) ² = 2.941	if 1.5 < Mx < 1.73
Cx = 0.605	if Mx > 1.73

(eq. 4-3)

Mx = 17.03 ⇒ Cx = 0.605

axL = 0.207	if R/t ≥ 610
axL = 169c/(195 + R/t) < 0.9 = 0.436	if R/t < 610

(eq. 4-4)

R/t = 192 ⇒ axL = 0.436

c = 2.64	if Mx ≤ 1.5
c = 3.13/(Mx) ^{0.42} = 0.952	if 1.5 < Mx ≤ 15
c = 1.0	if Mx > 15

(eq. 4-5)

Mx = 17.03 ⇒ c = 1.000

$F_{xeL} = (axL)(Cx)(E)(t/R) = 37,029 \text{ psi}$ (eq. 4-2)

b. Inelastic Buckling Stresses

Smaller of:

$F_{xcL} = F_{xeL} = 37,029 \text{ psi}$ eq. 4-6

$F_{xcL} = 233(Fy)/(166 + R/t) ≤ Fy = 12,481 \text{ psi}$	if R/t < 300
$F_{xcL} = 0.5(Fy) = 9,600 \text{ psi}$	if R/t ≥ 300

(eq. 4-7)

R/t = 192 ⇒ $F_{xcL} = 12,481 \text{ psi}$

SUBJECT

LIGO Beam Tube -- 49.00" OD x 0.127" Wall
1.75" x 0.1875" Vacuum Stiffener, 30" Spacing
300F Bakeout + Vacuum, No Seismic
Support Settlement of 0.579"

OFFICE: NOE		REVISION		REFERENCE NO. 930212
MADE BY WJC	CHKD RJW	MADE BY	CHKD BY	SHT 1 OF 6
DATE 3/9/94	DATE 18 MAR 94	DATE	DATE	7.17

Section 4.1.2 External Pressure (Nx/No = 0 or 0.5)

Use Nx/No = 0

Nx/No = 0 for radial pressure

Nx/No = 0.5 for hydrostatic pressure

a. Elastic Buckling Stresses

F_{reL} or $F_{heL} = (a_c L)(P_e L)(R)(K_o L)/t$ (eq. 4-8)

$K_o L = 1.0$	if	$M_x \geq 3.42$
$K_o L = 1 - e^y = 1.000$	if	$M_x < 3.42$ (eq. 11-7)
$M_x = 17.03$	\implies	$K_o L = 1.000$

$e = (1 - 0.3k)[1 + (L_e)/VA]$
$A = (A_r)(R/R_r)^2 = 0.304196$ sq.in.
$A_r = 0.328$ sq.in.
$R_r = 25.377$ in.
$L_e = [1.56(R_r)^{.5}] + t_w = 2.935797$ in.
$k = N_x/No = 0$
$e = 0.449$

$y = 1.0$	if	$M_x \leq 1.26$
$y = 1.58 - 0.46 M_x = -6.25$	if	$1.26 < M_x \leq 3.42$
$y = 0.0$	if	$M_x \geq 3.42$
$M_x = 17.03$	\implies	$y = 0.000$

$P_e L = [1.27 (E) (t/R)^2][A^{1.18 + 0.5}] = 34.8387$	if	$M_x > 1.5$ & $A < 2.5$	$A = M_x - 1.17 + 1.068k = 15.859$
$P_e L = [0.92 (E) (t/R)^2]/A = 42.3001$	if	$2.5 < A < 0.208 R/t$	$M_x = 17.03$
$P_e L = 0.836(C_p)^{-1.061} (E)(t/R)^3 = 44.7594$	if	$0.208 < C_p < 2.85$	$k = 0$
$P_e L = 0.275 (E)(t/R)^3 = 1.0420$	if	$C_p > 2.85$	$0.208 R/t = 40.02526$
$P_e L = 42.3001$			$C_p = A/(R/t) = 0.082413$

b. Imperfection Factors

$a_o L = 0.8$

F_{reL} or $F_{heL} = (a_o L)(P_e L)(R)(K_o L)/t = 6,512$ psi (eq. 4-8)

c. Inelastic Buckling Stresses

$F_{rcL} = n (F_{reL})$ (eq. 4-10)

$F_{hcL} = n (F_{heL})$ (eq. 4-11)

$n = 1.0$	if	$d \leq 0.55$	$d = F_{reL}/F_y = 0.3392$
$n = 0.45/d + 0.18 = 1.51$	if	$0.55 < d \leq 1.6$	$n = 1.000$ (eq. 5-3)
$n = 1.31/(1 + 1.15d) = 0.94$	if	$1.6 < d < 6.25$	
$n = 1/d = 2.95$	if	$d \geq 6.25$	

$F_{rcL} = n (F_{reL}) = 6,512$ psi (eq. 4-10)

d. Failure Pressures

$P_e L = (n)(a_o L)(P_e L) = 33,640$ psi (eq. 4-12)

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 300F Bakeout + Vacuum, No Seismic Support Settlement of 0.579"	OFFICE: NOE	REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RSW	MADE BY	CHKD BY
	DATE 3/9/94	DATE 12/14/94	DATE	DATE
				SHT 2 OF 6 7.18

Section 4.2 General Instability of Ring Stiffened Cylinders

Section 4.2.1 Axial Compression or Bending (No = 0)

a. Elastic Buckling Stresses

$F_{xeG} = (axG)(0.605)(E)(t/R)(1 + A'r)^{0.5}$ (eq. 4-13)

$A'r = Ar/[Lr] t] = 0.0861$ $Ar = \text{ring area} = 0.328 \text{ sq.in.}$
 $Lr = \text{ring spacing} = 30 \text{ in.}$

b. Imperfection Factors

$axG = 0.72$	if $A'r \geq 0.2$	(eq. 4-14)
$axG = [3.6 - 5.0(axL)] (A'r) + axL = 0.558$	if $0.06 < A'r < 0.2$	
$axG = axL = 0.436209$	if $A'r \leq 0.06$	
$axG = 0.558$	axL from eq. 4-4 with $c = 1.0$	

$F_{xeG} = (axG)(0.605)(E)(t/R)(1 + A'r)^{0.5} = 49,397 \text{ psi}$ (eq. 4-13)

c. Inelastic Buckling Stresses

$F_{xcG} = n (F_{xeG})$ (eq. 4-15)

$n = 1.0$	if $d \leq 0.55$	(eq. 5-3)
$n = 0.45/d + 0.18 = 0.35$	if $0.55 < d \leq 1.6$	
$n = 1.31/(1 + 1.15d) = 0.33$	if $1.6 < d < 6.25$	
$n = 1/d = 0.39$	if $d \geq 6.25$	

$F_{xcG} = n (F_{xeG}) = 16,346 \text{ psi}$ (eq. 4-15)

SUBJECT LIGO Beam Tube – 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 300F Bakeout + Vacuum, No Seismic Support Settlement of 0.579"	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD <i>RJW</i>	MADE BY	CHKD BY	SHT 3 OF 6
	DATE 3/9/94	DATE <i>10 March 94</i>	DATE	DATE	7.19

Section 4.2.2 External Pressure (Nx/No = 0 or 0.5)

Use Nx/No = 0
 Nx/No = 0 for radial pressure
 Nx/No = 0.5 for hydrostatic pressure

a. Buckling Stresses With or Without End Pressure

$FreG$ or $FheG = (acG)(PeG)(Ro)(KoG)/t$ (eq. 4-16)

$KoG = (1 - 0.3k)(Le)(t)/[(Ar + (Le)(t))] = 0.531994$ (eq. 11-9)

$Ar = 0.328$ sq.in.
 $Le = [1.56(Rt)^{0.5}] + tw = 2.935797$ in.
 $k = 0$

$PeG = \frac{(E)(t/R)(Lg)^4}{[N^2 + k(Lg)^2 - 1][N^2 + (Lg)^2]^2} + \frac{(E)(ler)(N^2 - 1)}{(Lr)(Rc)^2(R)}$ (eq. 4-17)

$lamda G (Lg) = (pi)(R)/Lb = 2.559194$ $Lb = 30$ in.

$k = 0$

$ler = lr + (Ar)(Zr)^2 (Le)(t)/[(Ar + (Le)(t))] + (Le)(t)^3/12 = 0.233$ in.^4 (eq. 4-18)

- $lr = 0.08374$ in.^4
- $Ar = 0.328$ sq.in.
- $Zr = 0.9385$ in.
- $t = 0.127$ in.
- $Rr = 25.3767$ in.

$Le = 1.1(Dt)^{0.5} = 2.741$ in. if $Mx > 1.56$
 $Le = Lr = 30.000$ in. if $Mx \leq 1.56$
 $Mx = 17.03 \implies Le = 2.741$ in.

$N = 4$ use N that gives PeG min. $2 < N$

$PeG = 988.937$ psi (eq. 4-17)

b. Imperfection Factors

$aoG = 0.8$

$FreG$ or $FheG = (aoG)(PeG)(Ro)(KoG)/t = 81,201$ psi (eq. 4-16)

c. Inelastic Buckling Stresses

$FrcG = n (FreG)$ (eq. 4-19)

$FhcG = n (FheG)$ (eq. 4-20)

$n = 1.0$	if $d \leq 0.55$	$d = FreG/Fy = 4.2292$ $n = 0.223$ (eq. 5-3)
$n = 0.45/d + 0.18 = 0.286$	if $0.55 < d \leq 1.6$	
$n = 1.31/(1 + 1.15d) = 0.223$	if $1.6 < d < 6.25$	
$n = 1/d = 0.236$	if $d \geq 6.25$	

$FrcG = n (FreG) = 18,141$ psi (eq. 4-10)

d. Failure Pressures

$poG = (n)(aoG)(peG) = 176,752$ psi (eq. 4-12)

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 300F Bakeout + Vacuum, No Seismic Support Settlement of 0.579"	OFFICE: NOE	REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RJW	MADE BY	CHKD BY
	DATE 3/9/94	DATE 10 MAR 94	DATE	DATE
				SHT 4 OF 6 7.20

SECTION 6 PREDICTED SHELL BUCKLING STRESSES FOR COMBINED LOADS

Section 6.3 Axial Compression, Bending and Hoop Compression

$(Ra)^2 - c (Ra)(Rh) + (Rh)^2 = 1.0$ (eq. 6-3)

$Ra = Fc_j / Fx_j$ $Rh = Foc_j / Frc_j$

a. Unstiffened and Ring Stiffened Cylinders (all buckling modes)

$c = (Fxc_j + Frc_j) / Fy - 1.0$ (eq. 6-4)

Local Buckling

$c = [FxcL + FrcL] / Fy - 1 = -0.01078$

Let $FicL = (FocL) (k) (KxL/KoL) = 2.0789 (FocL)$

$Ra = FicL / FxcL = 0.000167 (FocL)$

$KxL = 1.0$

$Rh = FocL / FrcL = 0.000154 (FocL)$

KoL = 1.0	if	Mx >= 3.42
KoL = 1 - e*y = 1.000	if	Mx < 3.42 (eq. 11-7)
Mx = 17.03	====>	KoL = 1.000

$e = (1 - 0.3k) / [1 + (Le)t/A]$
A = (Ar)(R/Rr)^2 = 0.304196 sq.in.
Ar = 0.328 sq.in.
Rr = 25.377 in.
Le = [1.56(Rt)^.5] + tw = 2.748297 in.
k = Nx/No = 2.079
e = 0.175

Solve eq. 6-3 for FocL

FocL = 4,402 psi (eq. 6-3)

FicL = (FocL) (k) (KxL/KoL) = 9,152 psi

General Instability

$c = [FxcG + FrcG] / Fy - 1 = 0.796234$

$KxG = 1.0$

$KoG = (1 - 0.3k)(Le)(t) / [(Ar + (Le)(t))] = 0.1940$ (eq. 11-9)
 (If KoG < 0 then ring is in tension and the interaction equation is not applicable. Then FicG = FxcG)

k = Nx / No = 2.079
Ar = 0.328 sq.in.
Le = [1.56(Rt)^.5] + tw = 2.748297 in.

Let $FicG = (FocG) (k) (KxG/KoG) = 10.71595 (FocG)$

$Ra = FicG / FxcG = 0.000656 (FocG)$

$Rh = FocG / FrcG = 5.51E-05 (FocG)$

Solve eq. 6-3 for FocG

FocG = 1,573 psi (eq. 6-3)

FicG = (FocG) (k) (KxG/KoG) = 18,859 psi

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 300F Bakeout + Vacuum, No Seismic Support Settlement of 0.579"	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RWN	MADE BY	CHKD BY	SHT 5 OF 6
	DATE 3/9/94	DATE 10/27/94	DATE	DATE	7.21

SECTION 9 ALLOWABLE STRESSES

Section 9.1 Allowable Stresses For Shell Buckling Mode

Section 9.1.5 Axial Compression and Hoop Compression and Axial Compression, Bending and Hoop Compression

$F_a = F_b = F_{cL} / FS$
 (eq. 9-7)
 $F_o = F_{oL} / FS$

Factor of Safety (FS) = 1.55

$F_a = F_b = 5,904 \text{ psi} = 749.85 \text{ #/in.}$

$N_x = 748.79 \text{ #/in.}$

$F_o = 2,840 \text{ psi} = 360.69 \text{ #/in.}$

$N_o = 360.18 \text{ #/in.}$

SECTION 11 STRESS CALCULATIONS

Section 11.3 Hoop Stresses

Section 11.3(b)2 Stress in a Ring Stiffener

Ring Stiffener Stress = $(p \cdot R_o / t) \cdot (1.00 - 0.30 \cdot (N_x / N_o)) \cdot (L_e \cdot t) / (A_r + L_e \cdot t)$ (Eq 11-8)

Solving Eq 11-8,

\Rightarrow Ring Stiffener Stress = 1,213 psi

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 300F Bakeout + Vacuum, No Seismic Support Settlement of 0.579"	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RIN	MADE BY	CHKD BY	SHT 6 OF 6
	DATE 3/9/94	DATE 10 MAR 94	DATE	DATE	7.22

**API BULLETIN 2U, FIRST EDITION, MAY 1, 1987
BULLETIN ON STABILITY DESIGN OF CYLINDRICAL SHELLS**

SECTION 4

**PREDICTED SHELL BUCKLING STRESSES FOR AXIAL
LOAD, BENDING AND EXTERNAL PRESSURE**

Ring Stiffener

Lr =	30.00 in.	b =	0.00 in.	tw =	0.1875 in.
R =	24.439 in.	Fy =	25,000 psi	Ar =	0.3280 sq.in.
t =	0.127 in.	E =	28,300,000 psi	Zr =	0.9385 in.
(Rt) ^{.5} =	1.762 in.	R/t =	192.43	lr =	0.08374 in. ^{.4}
Mx = Lr/(Rt) ^{.5} =	17.03 (eq. 4.1)			Fr =	25.377 in.
Mc = b/(Rt) ^{.5} =	0.00 (eq. 4.1)			Le = [1.56(Rt) ^{.5}] + tw =	2.936 in.
Nx =	287.78 #/in.	No =	360.18 #/in.		
	= 2,266 psi (Axial)		= 2,836 psi (Hoop)		

Section 4.1 Local Buckling of Unstiffened or Ring Stiffened Cylinders

Section 4.1.1 Axial Compression or Bending (No = 0)

a. Elastic Buckling Stresses

$F_{xeL} = (axL)(Cx)(E)(t/R)$ (eq. 4-2)

Cx = 0.630	if Mx ≤ 1.5	(eq. 4-3)
Cx = [0.904/(Mx) ²] + 0.1013(Mx) ² = 2.941	if 1.5 < Mx < 1.73	
Cx = 0.605	if Mx > 1.73	

Mx = 17.03 ⇒ Cx = 0.605

axL = 0.207	if R/t ≥ 610	(eq. 4-4)
axL = 169c/(195 + R/t) < 0.9 = 0.436	if R/t < 610	

R/t = 192 ⇒ axL = 0.436

c = 2.64	if Mx ≤ 1.5	(eq. 4-5)
c = 3.13/(Mx) ^{0.42} = 0.952	if 1.5 < Mx ≤ 15	
c = 1.0	if Mx > 15	

Mx = 17.03 ⇒ c = 1.000

$F_{xeL} = (axL)(Cx)(E)(t/R) = 38,812 \text{ psi}$ (eq. 4-2)

b. Inelastic Buckling Stresses

Smaller of:

$F_{xcL} = F_{xeL} = 38,812 \text{ psi}$ eq. 4-6

$F_{xcL} = 233(Fy)/(166 + R/t) = Fy = 16,251 \text{ psi}$	if R/t < 300	(eq. 4-7)
$F_{xcL} = 0.5(Fy) = 12,500 \text{ psi}$	if R/t ≥ 300	

R/t = 192 ⇒ $F_{xcL} = 16,251 \text{ psi}$

SUBJECT

LIGO Beam Tube -- 49.00" OD x 0.127" Wall
1.75" x 0.1875" Vacuum Stiffener, 30" Spacing
100F Operating + Vacuum, No Seismic
No Support Settlement

OFFICE: NOE

REVISION

REFERENCE NO.

MADE BY
WJC

CHKD
RJW

MADE BY

CHKD BY

SHT 1 OF 6

DATE
3/9/94

DATE
18 MAR 94

DATE

DATE

7.23

Section 4.1.2 External Pressure ($N_x/N_o = 0$ or 0.5)

Use $N_x/N_o = 0$

$N_x/N_o = 0$ for radial pressure

$N_x/N_o = 0.5$ for hydrostatic pressure

a. Elastic Buckling Stresses

F_{reL} or $F_{heL} = (a_c L)(P_{eL})(R)(K_o L)/t$ (eq. 4-8)

$K_o L = 1.0$	if	$M_x \geq 3.42$
$K_o L = 1 - e^* y = 1.000$	if	$M_x < 3.42$ (eq. 11-7)
$M_x = 17.03$	\implies	$K_o L = 1.000$

$e = (1 - 0.3k)/[1 + (L_e)/VA]$
$A = (A_r)(R/R_r)^2 = 0.304196$ sq.in.
$A_r = 0.328$ sq.in.
$R_r = 25.377$ in.
$L_e = [1.56(R/t)^{.5}] + t_w = 2.935797$ in.
$k = N_x/N_o = 0$
$e = 0.449$

$y = 1.0$	if	$M_x \leq 1.26$
$y = 1.58 - 0.46 M_x = -6.25$	if	$1.26 < M_x \leq 3.42$
$y = 0.0$	if	$M_x \geq 3.42$
$M_x = 17.03$	\implies	$y = 0.000$

$P_{eL} = [1.27 (E) (t/R)^2]/[A^{1.18} + 0.5] = 36.5161$	if	$M_x > 1.5$ & $A < 2.5$	$A = M_x - 1.17 + 1.068k = 15.859$
$P_{eL} = [0.92 (E) (t/R)^2]/A = 44.3368$	if	$2.5 < A < 0.208 R/t$	$M_x = 17.03$
$P_{eL} = 0.836(C_p)^{-1.061} (E)(t/R)^3 = 46.9145$	if	$0.208 < C_p < 2.85$	$k = 0$
$P_{eL} = 0.275 (E)(t/R)^3 = 1.0922$	if	$C_p > 2.85$	$0.208 R/t = 40.02526$
$P_{eL} = 44.3368$			$C_p = A/(R/t) = 0.082413$

b. Imperfection Factors

$a_o L = 0.8$

F_{reL} or $F_{heL} = (a_o L)(P_{eL})(R)(K_o L)/t = 6.825$ psi (eq. 4-8)

c. Inelastic Buckling Stresses

$F_{rcL} = n (F_{reL})$ (eq. 4-10)

$F_{hcL} = n (F_{heL})$ (eq. 4-11)

$n = 1.0$	if	$d \leq 0.55$	$d = F_{reL}/F_y = 0.2730$
$n = 0.45/d + 0.18 = 1.83$	if	$0.55 < d \leq 1.6$	$n = 1.000$ (eq. 5-3)
$n = 1.31/(1 + 1.15d) = 1.00$	if	$1.6 < d < 6.25$	
$n = 1/d = 3.66$	if	$d \geq 6.25$	

$F_{rcL} = n (F_{reL}) = 6.825$ psi (eq. 4-10)

d. Failure Pressures

$p_{cL} = (n)(a_o L)(p_{eL}) = 35.469$ psi (eq. 4-12)

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum, No Seismic No Support Settlement	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RJW	MADE BY	CHKD BY	SHT 2 OF 6
	DATE 3/9/94	DATE 19 MAR 94	DATE	DATE	7.24

Section 4.2 General Instability of Ring Stiffened Cylinders

Section 4.2.1 Axial Compression or Bending (No = 0)

a. Elastic Buckling Stresses

$F_{xeG} = (axG)(0.605)(E)(t/R)(1 + A'r)^{0.5}$ (eq. 4-13)

$A'r = Ar/[Lr] t] = 0.0861$ $Ar = \text{ring area} = 0.328 \text{ sq.in.}$
 $Lr = \text{ring spacing} = 30 \text{ in.}$

b. Imperfection Factors

$axG = 0.72$	if $A'r \geq 0.2$	(eq. 4-14)
$axG = [3.6 - 5.0(axL)] (A'r) + axL = 0.558$	if $0.06 < A'r < 0.2$	
$axG = axL = 0.436209$	if $A'r \leq 0.06$	
$axG = 0.558$	axL from eq. 4-4 with $c = 1.0$	

$F_{xeG} = (axG)(0.605)(E)(t/R)(1 + A'r)^{0.5} = 51,775 \text{ psi}$ (eq. 4-13)

c. Inelastic Buckling Stresses

$F_{xcG} = n (F_{xeG})$ (eq. 4-15)

$n = 1.0$	if $d \leq 0.55$	(eq. 5-3)
$n = 0.45/d + 0.18 = 0.40$	if $0.55 < d \leq 1.6$	
$n = 1.31/(1 + 1.15d) = 0.39$	if $1.6 < d < 6.25$	
$n = 1/d = 0.48$	if $d \geq 6.25$	

$d = F_{xeG}/F_y = 2.0710$
 $n = 0.387$

$F_{xcG} = n (F_{xeG}) = 20,057 \text{ psi}$ (eq. 4-15)

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum, No Seismic No Support Settlement	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RJW	MADE BY	CHKD BY	SHT 3 OF 6
	DATE 3/9/94	DATE 15 MAR 94	DATE	DATE	7.25

Section 4.2.2 External Pressure (Nx/No = 0 or 0.5)

Use Nx/No = 0
 Nx/No = 0 for radial pressure
 Nx/No = 0.5 for hydrostatic pressure

a. Buckling Stresses With or Without End Pressure

$FreG$ or $FheG = (acG)(PeG)(Ro)(KoG)/t$ (eq. 4-16)

$KoG = (1 - 0.3k)(Le)(t)/[(Ar + (Le)(t))] = 0.531994$ (eq. 11-9)

$Ar = 0.328$ sq.in.
 $Le = [1.56(Rt)^{0.5}] + tw = 2.935797$ in.
 $k = 0$

$PeG = \frac{(E)(t/R)(Lg)^4}{[N^2 + k(Lg)^2 - 1][N^2 + (Lg)^2]^2} + \frac{(E)(ler)(N^2 - 1)}{(Lr)(Rc)^2(R)}$ (eq. 4-17)

$\lambda G (Lg) = (\pi)(R)/Lb = 2.559194$ $Lb = 30$ in.

$k = 0$

$ler = lr + (Ar)(Zr)^2 (Le)(t)/[(Ar + (Le)(t))] + (Le)(t)^3/12 = 0.233$ in.^4 (eq. 4-18)

- $lr = 0.08374$ in.^4
- $Ar = 0.328$ sq.in.
- $Zr = 0.9385$ in.
- $t = 0.127$ in.
- $Rr = 25.3767$ in.

$Le = 1.1(Dt)^{0.5} = 2.741$ in. if $Mx > 1.56$
 $Le = Lr = 30.000$ in. if $Mx \leq 1.56$
 $Mx = 17.03 \implies Le = 2.741$ in.

$N = 4$ use N that gives PeG min. $2 < N$

$PeG = 1036.55$ psi (eq. 4-17)

b. Imperfection Factors

$aoG = 0.8$

$FreG$ or $FheG = (aoG)(PeG)(Ro)(KoG)/t = 85,111$ psi (eq. 4-16)

c. Inelastic Buckling Stresses

$FrcG = n (FreG)$ (eq. 4-19)

$FhcG = n (FheG)$ (eq. 4-20)

$n = 1.0$	if $d \leq 0.55$	$d = FreG/Fy = 3.4044$ $n = 0.267$	(eq. 5-3)
$n = 0.45/d + 0.18 = 0.312$	if $0.55 < d \leq 1.6$		
$n = 1.31/(1 + 1.15d) = 0.267$	if $1.6 < d < 6.25$		
$n = 1/d = 0.294$	if $d \geq 6.25$		

$FrcG = n (FreG) = 22,684$ psi (eq. 4-10)

d. Failure Pressures

$poG = (n)(aoG)(peG) = 221,014$ psi (eq. 4-12)

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum, No Seismic No Support Settlement	OFFICE: NOE	REVISION	REFERENCE NO. 930212	
	MADE BY WJC	CHKD R JW	MADE BY CHKD BY	
	DATE 3/9/94	DATE 19 MAR 94	DATE DATE	SHT 4 OF 6 7,26

SECTION 6 PREDICTED SHELL BUCKLING STRESSES FOR COMBINED LOADS

Section 6.3 Axial Compression, Bending and Hoop Compression

$(Ra)^2 - c (Ra)(Rh) + (Rh)^2 = 1.0$ (eq. 6-3)

$Ra = F_{icj} / F_{xcj}$ $Rh = F_{ocj} / F_{rcj}$

a. Unstiffened and Ring Stiffened Cylinders (all buckling modes)

$c = (F_{xcj} + F_{rcj}) / F_y - 1.0$ (eq. 6-4)

Local Buckling

$c = [F_{xcL} + F_{rcL}] / F_y - 1 = -0.07693$

Let $F_{icL} = (F_{ocL}) (k) (K_{xL} / K_{oL}) = 0.7990 (F_{ocL})$

$Ra = F_{icL} / F_{xcL} = 4.92E-05 (F_{ocL})$

$K_{xL} = 1.0$

$Rh = F_{ocL} / F_{rcL} = 0.000147 (F_{ocL})$

$K_{oL} = 1.0$ if $M_x \geq 3.42$
 $K_{oL} = 1 - e^*y = 1.000$ if $M_x < 3.42$ (eq. 11-7)
 $M_x = 17.03 \implies K_{oL} = 1.000$

$e = (1 - 0.3k) / [1 + (Le) / VA]$
 $A = (Ar)(R / Rr)^2 = 0.304196$ sq.in.
 $Ar = 0.328$ sq.in.
 $Rr = 25.377$ in.
 $Le = [1.56(Rt)^{.5}] + tw = 2.748297$ in.
 $k = N_x / N_o = 0.799$
 $e = 0.354$

Solve eq. 6-3 for F_{ocL}

$F_{ocL} = 6,397$ psi (eq. 6-3)

$F_{icL} = (F_{ocL}) (k) (K_{xL} / K_{oL}) = 5,111$ psi

General Instability

$c = [F_{xcG} + F_{rcG}] / F_y - 1 = 0.709645$

$K_{xG} = 1.0$

$K_{oG} = (1 - 0.3k)(Le)(t) / [Ar + (Le)(t)] = 0.3920$ (eq. 11-9)
 (If $K_{oG} < 0$ then ring is in tension and the interaction equation is not applicable. Then $F_{icG} = F_{xcG}$)

$k = N_x / N_o = 0.799$
 $Ar = 0.328$ sq.in.
 $Le = [1.56(Rt)^{.5}] + tw = 2.748297$ in.

Let $F_{icG} = (F_{ocG}) (k) (K_{xG} / K_{oG}) = 2.038459 (F_{ocG})$

$Ra = F_{icG} / F_{xcG} = 0.000102 (F_{ocG})$

$Rh = F_{ocG} / F_{rcG} = 4.41E-05 (F_{ocG})$

Solve eq. 6-3 for F_{ocG}

$F_{ocG} = 10,487$ psi (eq. 6-3)

$F_{icG} = (F_{ocG}) (k) (K_{xG} / K_{oG}) = 21,377$ psi

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum, No Seismic No Support Settlement	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RJW	MADE BY	CHKD BY	SHT 5 OF 6 7.27
	DATE 3/9/94	DATE 10/MAR/94	DATE	DATE	

SECTION 9 ALLOWABLE STRESSES

Section 9.1 Allowable Stresses For Shell Buckling Mode

Section 9.1.5 Axial Compression and Hoop Compression and Axial Compression, Bending and Hoop Compression

$F_a = F_b = F_{cL} / FS$
(eq. 9-7)

$F_o = F_{oL} / FS$

Factor of Safety (FS) = 2.25

$F_a = F_b = 2,272 \text{ psi} = 288.50 \text{ #/in.}$

$N_x = 287.78 \text{ #/in.}$

$F_o = 2,843 \text{ psi} = 361.07 \text{ #/in.}$

$N_o = 360.18 \text{ #/in.}$

SECTION 11 STRESS CALCULATIONS

Section 11.3 Hoop Stresses

Section 11.3(b)2 Stress in a Ring Stiffener

Ring Stiffener Stress = $(p \cdot R_o / t) \cdot (1.00 - 0.30 \cdot (N_x / N_o)) \cdot (L_e \cdot t) / (A_r + L_e \cdot t)$ (Eq 11-8)

Solving Eq 11-8,

\Rightarrow Ring Stiffener Stress = 2,451 psi

SUBJECT

LIGO Beam Tube -- 49.00" OD x 0.127" Wall
1.75" x 0.1875" Vacuum Stiffener, 30" Spacing
100F Operating + Vacuum, No Seismic
No Support Settlement

OFFICE: NOE

REVISION

REFERENCE NO.

MADE BY

CHKD

MADE BY

CHKD BY

SHT 6 OF 6

DATE
3/9/94

DATE
18 MAR 94

DATE

DATE

7.28

**API BULLETIN 2U, FIRST EDITION, MAY 1, 1987
BULLETIN ON STABILITY DESIGN OF CYLINDRICAL SHELLS**

SECTION 4

**PREDICTED SHELL BUCKLING STRESSES FOR AXIAL
LOAD, BENDING AND EXTERNAL PRESSURE**

Ring Stiffener

Lr =	30.00	in.	b =	0.00	in.	tw =	0.1875	in.
R =	24.439	in.	Fy =	25,000	psi	Ar =	0.3280	sq.in.
t =	0.127	in.	E =	28,300,000	psi	Zr =	0.9385	in.
(Rt) ^{.5} =	1.762	in.	R/t =	192.43		lr =	0.08374	in. ^{.4}
Mx = Lr/(Rt) ^{.5} =	17.03	(eq. 4.1)				Rr =	25.377	in.
Mc = b/(Rt) ^{.5} =	0.00	(eq. 4.1)				Le = [1.56(Rt) ^{.5}] + tw =	2.936	in.

Nx = 354.20 #/in. No = 360.18 #/in.
 = 2,789 psi (Axial) = 2,836 psi (Hoop)

Section 4.1 Local Buckling of Unstiffened or Ring Stiffened Cylinders

Section 4.1.1 Axial Compression or Bending (No = 0)

a. Elastic Buckling Stresses

$F_{xeL} = (axL)(Cx)(E)(t/R)$ (eq. 4-2)

Cx = 0.630	if Mx ≤ 1.5	(eq. 4-3)
Cx = [0.904/(Mx) ²] + 0.1013(Mx) ² = 2.941	if 1.5 < Mx < 1.73	
Cx = 0.605	if Mx > 1.73	

Mx = 17.03 → Cx = 0.605

axL = 0.207	if R/t ≥ 610	(eq. 4-4)
axL = 169c/(195 + R/t) < 0.9 = 0.436	if R/t < 610	

R/t = 192 → axL = 0.436

c = 2.64	if Mx ≤ 1.5	(eq. 4-5)
c = 3.13/(Mx) ^{0.42} = 0.952	if 1.5 < Mx ≤ 15	
c = 1.0	if Mx > 15	

Mx = 17.03 → c = 1.000

$F_{xeL} = (axL)(Cx)(E)(t/R) = 38,812 \text{ psi}$ (eq. 4-2)

b. Inelastic Buckling Stresses

Smaller of:

$F_{xcL} = F_{xeL} = 38,812 \text{ psi}$ eq. 4-6

$F_{xcL} = 233(Fy)/(166 + R/t) ≤ Fy = 16,251 \text{ psi}$	if R/t < 300	(eq. 4-7)
$F_{xcL} = 0.5(Fy) = 12,500 \text{ psi}$	if R/t ≥ 300	

R/t = 192 → $F_{xcL} = 16,251 \text{ psi}$

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum + Axial Seismic No Support Settlement	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD R/W	MADE BY	CHKD BY	SHT 1 OF 6
	DATE 3/9/94	DATE 18 MAR 94	DATE	DATE	7.29

Section 4.1.2 External Pressure (Nx/No = 0 or 0.5)

Use Nx/No = 0
 Nx/No = 0 for radial pressure
 Nx/No = 0.5 for hydrostatic pressure

a. Elastic Buckling Stresses

F_{reL} or $F_{heL} = (a_c L)(P_{eL})(R)(K_{oL})/t$ (eq. 4-8)

$K_{oL} = 1.0$	if	$Mx \geq 3.42$
$K_{oL} = 1 - e^*y = 1.000$	if	$Mx < 3.42$ (eq. 11-7)
$Mx = 17.03$	\implies	$K_{oL} = 1.000$

$e = (1 - 0.3k)/[1 + (L_e)/VA]$
$A = (A_r)(R/R_r)^2 = 0.304196$ sq.in.
$A_r = 0.328$ sq.in.
$R_r = 25.377$ in.
$L_e = [1.56(R/t)^{0.5}] + t_w = 2.935797$ in.
$k = N_x/No = 0$
$e = 0.449$

$y = 1.0$	if	$Mx \leq 1.26$
$y = 1.58 - 0.46 Mx = -6.25$	if	$1.26 < Mx \leq 3.42$
$y = 0.0$	if	$Mx \geq 3.42$
$Mx = 17.03$	\implies	$y = 0.000$

$P_{eL} = [1.27 (E) (t/R)^2][A^{1.18} + 0.5] = 36.5161$	if	$Mx > 1.5$ & $A < 2.5$	$A = Mx - 1.17 + 1.068k = 15.859$
$P_{eL} = [0.92 (E) (t/R)^2]/A = 44.3368$	if	$2.5 < A < 0.208 R/t$	$Mx = 17.03$
$P_{eL} = 0.836(C_p)^{-1.061} (E)(t/R)^3 = 46.9145$	if	$0.208 < C_p < 2.85$	$k = 0$
$P_{eL} = 0.275 (E)(t/R)^3 = 1.0922$	if	$C_p > 2.85$	$0.208 R/t = 40.02526$
$P_{eL} = 44.3368$			$C_p = A/(R/t) = 0.082413$

b. Imperfection Factors

$a_{oL} = 0.8$

F_{reL} or $F_{heL} = (a_{oL})(P_{eL})(R)(K_{oL})/t = 6.825$ psi (eq. 4-8)

c. Inelastic Buckling Stresses

$F_{rcL} = n (F_{reL})$ (eq. 4-10)

$F_{hcL} = n (F_{heL})$ (eq. 4-11)

$n = 1.0$	if	$d \leq 0.55$	$d = F_{reL}/F_y = 0.2730$
$n = 0.45/d + 0.18 = 1.83$	if	$0.55 < d \leq 1.6$	$n = 1.000$ (eq. 5-3)
$n = 1.31/(1 + 1.15d) = 1.00$	if	$1.6 < d < 6.25$	
$n = 1/d = 3.66$	if	$d \geq 6.25$	

$F_{rcL} = n (F_{reL}) = 6.825$ psi (eq. 4-10)

d. Failure Pressures

$P_{oL} = (n)(a_{oL})(P_{eL}) = 35.469$ psi (eq. 4-12)

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum + Axial Seismic No Support Settlement	OFFICE: NOE	REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RIN	MADE BY	CHKD BY
	DATE 3/9/94	DATE 16 MAR 94	DATE	DATE
				SHT 2 OF 6 7.30

Section 4.2 General Instability of Ring Stiffened Cylinders

Section 4.2.1 Axial Compression or Bending (No = 0)

a. Elastic Buckling Stresses

$F_{xeG} = (axG)(0.605)(E)(t/R)(1 + A'r)^{0.5}$ (eq. 4-13)

$A'r = Ar/[Lr] t] = 0.0861$ $Ar = \text{ring area} = 0.328 \text{ sq.in.}$
 $Lr = \text{ring spacing} = 30 \text{ in.}$

b. Imperfection Factors

$axG = 0.72$	if $A'r \geq 0.2$	(eq. 4-14)
$axG = [3.6 - 5.0(axL)] (A'r) + axL = 0.558$	if $0.06 < A'r < 0.2$	
$axG = axL = 0.436209$	if $A'r \leq 0.06$	
$axG = 0.558$	axL from eq. 4-4 with $c = 1.0$	

$F_{xeG} = (axG)(0.605)(E)(t/R)(1 + A'r)^{0.5} = 51,775 \text{ psi}$ (eq. 4-13)

c. Inelastic Buckling Stresses

$F_{xcG} = n (F_{xeG})$ (eq. 4-15)

$n = 1.0$	if $d \leq 0.55$	$d = F_{xeG}/F_y = 2.0710$ $n = 0.387$	(eq. 5-3)
$n = 0.45/d + 0.18 = 0.40$	if $0.55 < d \leq 1.6$		
$n = 1.31/(1 + 1.15d) = 0.39$	if $1.6 < d < 6.25$		
$n = 1/d = 0.48$	if $d \geq 6.25$		

$F_{xcG} = n (F_{xeG}) = 20,057 \text{ psi}$ (eq. 4-15)

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum + Axial Seismic No Support Settlement	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD <i>RJW</i>	MADE BY	CHKD BY	SHT 3 OF 6
	DATE 3/9/94	DATE <i>10 MAR 94</i>	DATE	DATE	
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Section 4.2.2 External Pressure (Nx/No = 0 or 0.5)

Use Nx/No = 0
 Nx/No = 0 for radial pressure
 Nx/No = 0.5 for hydrostatic pressure

a. Buckling Stresses With or Without End Pressure

$FreG$ or $FheG = (acG)(PeG)(Ro)(KoG)/t$ (eq. 4-16)

$KoG = (1 - 0.3k)(Le)(t)/[(Ar + (Le)(t))] = 0.531994$ (eq. 11-9)

$Ar = 0.328$ sq.in.
 $Le = [1.56(Rt)^{.5}] + tw = 2.935797$ in.
 $k = 0$

$PeG = \frac{(E)(t/R)(Lg)^4}{[N^2 + k(Lg)^2 - 1][N^2 + (Lg)^2]^2} + \frac{(E)(ler)(N^2 - 1)}{(Lr)(Rc)^2(R)}$ (eq. 4-17)

$\lambda G (Lg) = (\pi)(R)/Lb = 2.559194$ $Lb = 30$ in.

$k = 0$

$ler = lr + (Ar)(Zr)^2 (Le)(t)/[(Ar + (Le)(t))] + (Le)(t)^3/12 = 0.233$ in.^4 (eq. 4-18)

- $lr = 0.08374$ in.^4
- $Ar = 0.328$ sq.in.
- $Zr = 0.9385$ in.
- $t = 0.127$ in.
- $Rr = 25.3767$ in.

$Le = 1.1(Dt)^{0.5} = 2.741$ in. if $Mx > 1.56$
 $Le = Lr = 30.000$ in. if $Mx \leq 1.56$
 $Mx = 17.03 \implies Le = 2.741$ in.

$N = 4$ use N that gives PeG min. $2 < N$

$PeG = 1036.55$ psi (eq. 4-17)

b. Imperfection Factors

$acG = 0.8$

$FreG$ or $FheG = (acG)(PeG)(Ro)(KoG)/t = 85,111$ psi (eq. 4-16)

c. Inelastic Buckling Stresses

$FrcG = n(FreG)$ (eq. 4-19)

$FhcG = n(FheG)$ (eq. 4-20)

$n = 1.0$	if $d \leq 0.55$	$d = FreG/Fy = 3.4044$ $n = 0.267$	(eq. 5-3)
$n = 0.45/d + 0.18 = 0.312$	if $0.55 < d \leq 1.6$		
$n = 1.31/(1 + 1.15d) = 0.267$	if $1.6 < d < 6.25$		
$n = 1/d = 0.294$	if $d \geq 6.25$		

$FrcG = n(FreG) = 22,684$ psi (eq. 4-10)

d. Failure Pressures

$pcG = (n)(acG)(peG) = 221,014$ psi (eq. 4-12)

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum + Axial Seismic No Support Settlement	OFFICE: NOE	REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RJM	MADE BY	CHKD BY
	DATE 3/9/94	DATE 18 MAR 94	DATE	DATE
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SECTION 6 PREDICTED SHELL BUCKLING STRESSES FOR COMBINED LOADS

Section 6.3 Axial Compression, Bending and Hoop Compression

$(Ra)^2 - c (Ra)(Rh) + (Rh)^2 = 1.0$ (eq. 6-3)

$Ra = F_{icj} / F_{xcj}$ $Rh = F_{ocj} / F_{rcj}$

a. Unstiffened and Ring Stiffened Cylinders (all buckling modes)

$c = (F_{xcj} + F_{rcj}) / F_y - 1.0$ (eq. 6-4)

Local Buckling

$c = [F_{xcL} + F_{rcL}] / F_y - 1 = -0.07693$

Let $F_{icL} = (F_{ocL}) (k) (K_{xL} / K_{oL}) = 0.9834 (F_{ocL})$

$Ra = F_{icL} / F_{xcL} = 6.05E-05 (F_{ocL})$

$K_{xL} = 1.0$

$Rh = F_{ocL} / F_{rcL} = 0.000147 (F_{ocL})$

$K_{oL} = 1.0$ if $M_x \geq 3.42$
 $K_{oL} = 1 - e^*y = 1.000$ if $M_x < 3.42$ (eq. 11-7)
 $M_x = 17.03 \implies K_{oL} = 1.000$

$e = (1 - 0.3k) / [1 + (L_e) / \sqrt{A}]$
 $A = (A_r) (R / R_r)^2 = 0.304196 \text{ sq.in.}$
 $A_r = 0.328 \text{ sq.in.}$
 $R_r = 25.377 \text{ in.}$
 $L_e = [1.56(R_r)^{.5}] + t_w = 2.748297 \text{ in.}$
 $k = N_x / N_o = 0.983$
 $e = 0.328$

Solve eq. 6-3 for F_{ocL}

$F_{ocL} = 6,225 \text{ psi}$ (eq. 6-3)

$F_{icL} = (F_{ocL}) (k) (K_{xL} / K_{oL}) = 6,121 \text{ psi}$

General Instability

$c = [F_{xcG} + F_{rcG}] / F_y - 1 = 0.709645$

$K_{xG} = 1.0$

$K_{oG} = (1 - 0.3k)(L_e)(t) / [(A_r) + (L_e)(t)] = 0.3634$ (eq. 11-9)
 (if $K_{oG} < 0$ then ring is in tension and the interaction equation is not applicable. Then $F_{icG} = F_{xcG}$)

$k = N_x / N_o = 0.983$
 $A_r = 0.328 \text{ sq.in.}$
 $L_e = [1.56(R_r)^{.5}] + t_w = 2.748297 \text{ in.}$

Let $F_{icG} = (F_{ocG}) (k) (K_{xG} / K_{oG}) = 2.705832 (F_{ocG})$

$Ra = F_{icG} / F_{xcG} = 0.000135 (F_{ocG})$

$Rh = F_{ocG} / F_{rcG} = 4.41E-05 (F_{ocG})$

Solve eq. 6-3 for F_{ocG}

$F_{ocG} = 7,925 \text{ psi}$ (eq. 6-3)

$F_{icG} = (F_{ocG}) (k) (K_{xG} / K_{oG}) = 21,443 \text{ psi}$

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum + Axial Seismic No Support Settlement	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD <i>RW</i>	MADE BY	CHKD BY	SHT 5 OF 6
	DATE 3/9/94	DATE 18 MAR 94	DATE	DATE	7.33

SECTION 9 ALLOWABLE STRESSES

Section 9.1 Allowable Stresses For Shell Buckling Mode

Section 9.1.5 Axial Compression and Hoop Compression and Axial Compression, Bending and Hoop Compression

$F_a = F_b = F_{cL} / FS$
(eq. 9-7)

$F_o = F_{ocL} / FS$

Factor of Safety (FS) = 2.19

$F_a = F_b = 2,795 \text{ psi} = 354.98 \text{ #/in.}$	Nx = 354.20 #/in.
$F_c = 2,842 \text{ psi} = 360.97 \text{ #/in.}$	No = 360.18 #/in.

SECTION 11 STRESS CALCULATIONS

Section 11.3 Hoop Stresses

Section 11.3(b)2 Stress in a Ring Stiffener

Ring Stiffener Stress = $(p \cdot R_o / t) \cdot (1.00 - 0.30 \cdot (N_x / N_o)) \cdot (L_e \cdot t) / (A_r + L_e \cdot t)$ (Eq 11-8)

Solving Eq 11-8,

==> Ring Stiffener Stress = 2,273 psi

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum + Axial Seismic No Support Settlement	OFFICE: NOE	REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD R.J.W.	MADE BY	CHKD BY
	DATE 3/9/94	DATE 18 MAR 94	DATE	DATE
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API BULLETIN 2U, FIRST EDITION, MAY 1, 1987
BULLETIN ON STABILITY DESIGN OF CYLINDRICAL SHELLS

SECTION 4

**PREDICTED SHELL BUCKLING STRESSES FOR AXIAL
LOAD, BENDING AND EXTERNAL PRESSURE**

Ring Stiffener

Lr =	30.00 in.	b =	0.00 in.	tw =	0.1875 in.
R =	24.439 in.	Fy =	25,000 psi	Ar =	0.3280 sq.in.
t =	0.127 in.	E =	28,300,000 psi	Zr =	0.9385 in.
(Rt) ^{.5} =	1.762 in.	R/t =	192.43	lr =	0.08374 in. ^{.4}
Mx = Lr/(Rt) ^{.5} =	17.03 (eq. 4.1)			Fr =	25.377 in.
Mc = b/(Rt) ^{.5} =	0.00 (eq. 4.1)			Le = [1.56(Rt) ^{.5} + tw =	2.936 in.
Nx =	401.45 #/in.	No =	360.18 #/in.		
	= 3,161 psi (Axial)		= 2,836 psi (Hoop)		

Section 4.1 Local Buckling of Unstiffened or Ring Stiffened Cylinders

Section 4.1.1 Axial Compression or Bending (No = 0)

a. Elastic Buckling Stresses

$F_{xeL} = (axL)(C_x)(E)(t/R)$ (eq. 4-2)

C _x = 0.630	if M _x ≤ 1.5	(eq. 4-3)
C _x = [0.904/(M _x) ²] + 0.1013(M _x) ² = 2.941	if 1.5 < M _x < 1.73	
C _x = 0.605	if M _x > 1.73	

M_x = 17.03 ⇒ C_x = 0.605

axL = 0.207	if R/t ≥ 610	(eq. 4-4)
axL = 169c/(195 + R/t) < 0.9 = 0.436	if R/t < 610	

R/t = 192 ⇒ axL = 0.436

c = 2.64	if M _x ≤ 1.5	(eq. 4-5)
c = 3.13/(M _x) ^{0.42} = 0.952	if 1.5 < M _x ≤ 15	
c = 1.0	if M _x > 15	

M_x = 17.03 ⇒ c = 1.000

$F_{xeL} = (axL)(C_x)(E)(t/R) = 38,812 \text{ psi}$ (eq. 4-2)

b. Inelastic Buckling Stresses

Smaller of:

F_{xcL} = F_{xeL} = 38,812 psi (eq. 4-6)

F _{xcL} = 233(F _y)/(166 + R/t) = 16,251 psi	if R/t < 300	(eq. 4-7)
F _{xcL} = 0.5(F _y) = 12,500 psi	if R/t ≥ 300	

R/t = 192 ⇒ F_{xcL} = 16,251 psi

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum, No Seismic Support Settlement of 0.250"	OFFICE: NOE	REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RSW	MADE BY	CHKD BY
	DATE 3/9/94	DATE 18MAY94	DATE	DATE
				SHT 1 OF 6 7.35

Section 4.1.2 External Pressure (Nx/No = 0 or 0.5)

Use Nx/No = 0

Nx/No = 0 for radial pressure

Nx/No = 0.5 for hydrostatic pressure

a. Elastic Buckling Stresses

F_{reL} or $F_{heL} = (a_c L)(P_{eL})(R)/(K_o L)/t$ (eq. 4-8)

$K_o L = 1.0$	if	$M_x \geq 3.42$
$K_o L = 1 - e^y = 1.000$	if	$M_x < 3.42$ (eq. 11-7)
$M_x = 17.03$	\implies	$K_o L = 1.000$

$e = (1 - 0.3k)[1 + (L_e)/A]$
$A = (A_r)(R/R_r)^2 = 0.304196$ sq.in.
$A_r = 0.328$ sq.in.
$R_r = 25.377$ in.
$L_e = [1.56(R/t)^{0.5}] + t_w = 2.935797$ in.
$k = N_x/No = 0$
$e = 0.449$

$y = 1.0$	if	$M_x \leq 1.26$
$y = 1.58 - 0.46 M_x = -6.25$	if	$1.26 < M_x \leq 3.42$
$y = 0.0$	if	$M_x \geq 3.42$
$M_x = 17.03$	\implies	$y = 0.000$

$P_{eL} = [1.27 (E) (V/R)^2]/[A^{1.18} + 0.5] = 36.5161$	if	$M_x > 1.5$ & $A < 2.5$	$A = M_x - 1.17 + 1.068k = 15.859$
$P_{eL} = [0.92 (E) (V/R)^2]/A = 44.3368$	if	$2.5 < A < 0.208 R/t$	$M_x = 17.03$
$P_{eL} = 0.836(C_p)^{-1.061} (E)(V/R)^3 = 46.9145$	if	$0.208 < C_p < 2.85$	$k = 0$
$P_{eL} = 0.275 (E)(V/R)^3 = 1.0922$	if	$C_p > 2.85$	$0.208 R/t = 40.02526$
$P_{eL} = 44.3368$			$C_p = A/(R/t) = 0.082413$

b. Imperfection Factors

$a_o L = 0.8$

F_{reL} or $F_{heL} = (a_o L)(P_{eL})(R)/(K_o L)/t = 6.825$ psi (eq. 4-8)

c. Inelastic Buckling Stresses

$F_{rcL} = n (F_{reL})$ (eq. 4-10)

$F_{hcL} = n (F_{heL})$ (eq. 4-11)

$n = 1.0$	if	$d \leq 0.55$	$d = F_{reL}/F_y = 0.2730$
$n = 0.45/d + 0.18 = 1.83$	if	$0.55 < d \leq 1.6$	$n = 1.000$ (eq. 5-3)
$n = 1.31/(1 + 1.15d) = 1.00$	if	$1.6 < d < 6.25$	
$n = 1/d = 3.66$	if	$d \geq 6.25$	

$F_{rcL} = n (F_{reL}) = 6.825$ psi (eq. 4-10)

d. Failure Pressures

$p_{oL} = (n)(a_o L)(p_{eL}) = 35.469$ psi (eq. 4-12)

SUBJECT

LIGO Beam Tube -- 49.00" OD x 0.127" Wall
1.75" x 0.1875" Vacuum Stiffener, 30" Spacing
100F Operating + Vacuum, No Seismic
Support Settlement of 0.250"

OFFICE: NOE		REVISION		REFERENCE NO. 930212
MADE BY WJC	CHKD RJN	MADE BY	CHKD BY	SHT 2 OF 6
DATE 3/9/94	DATE 18 MAR 94	DATE	DATE	7.36

Section 4.2 General Instability of Ring Stiffened Cylinders

Section 4.2.1 Axial Compression or Bending (No = 0)

a. Elastic Buckling Stresses

$F_{xeG} = (axG)(0.605)(E)(t/R)(1 + A'r)^{0.5}$ (eq. 4-13)

$A'r = Ar/[Lr] t] = 0.0861$ $Ar = \text{ring area} = 0.328 \text{ sq.in.}$
 $Lr = \text{ring spacing} = 30 \text{ in.}$

b. Imperfection Factors

$axG = 0.72$	if $A'r \geq 0.2$	(eq. 4-14)
$axG = [3.6 - 5.0(axL)] (A'r) + axL = 0.558$	if $0.06 < A'r < 0.2$	
$axG = axL = 0.436209$	if $A'r \leq 0.06$	
$axG = 0.558$	axL from eq. 4-4 with $c = 1.0$	

$F_{xeG} = (axG)(0.605)(E)(t/R)(1 + A'r)^{0.5} = 51,775 \text{ psi}$ (eq. 4-13)

c. Inelastic Buckling Stresses

$F_{xcG} = n (F_{xeG})$ (eq. 4-15)

$n = 1.0$	if $d \leq 0.55$	(eq. 5-3)
$n = 0.45/d + 0.18 = 0.40$	if $0.55 < d \leq 1.6$	
$n = 1.31/(1 + 1.15d) = 0.39$	if $1.6 < d < 6.25$	
$n = 1/d = 0.48$	if $d \geq 6.25$	

$d = F_{xeG}/F_y = 2.0710$
 $n = 0.387$

$F_{xcG} = n (F_{xeG}) = 20,057 \text{ psi}$ (eq. 4-15)

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum, No Seismic Support Settlement of 0.250"	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RSN	MADE BY	CHKD BY	SHT 3 OF 6
	DATE 3/9/94	DATE 18 MAR 94	DATE	DATE	7.37

Section 4.2.2 External Pressure (Nx/No = 0 or 0.5)

Use Nx/No = 0
 Nx/No = 0 for radial pressure
 Nx/No = 0.5 for hydrostatic pressure

a. Buckling Stresses With or Without End Pressure

$FreG$ or $FheG = (acG)(PeG)(Ro)(KoG)/t$ (eq. 4-16)

$KoG = (1 - 0.3k)(Le)(t)/[(Ar + (Le)(t))] =$

0.531994 (eq. 11-9)

$Ar =$	0.328	sq.in.
$Le = [1.56(Rt)^{0.5}] + tw =$	2.935797	in.
$k =$	0	

$PeG = \frac{(E)(t/R)(Lg)^4}{[N^2 + k(Lg)^2 - 1][N^2 + (Lg)^2]^2}$

$+$ $\frac{(E)(ler)(N^2 - 1)}{(Lr)(Rc)^2(R)}$ (eq. 4-17)

$\lambda G (Lg) = (\pi)(R)/Lb = 2.559194$

$Lb = 30$ in.

$k = 0$

$ler = lr + (Ar)(Zr)^2 (Le)(t)/[(Ar + (Le)(t))] + (Le)(t)^3/12 =$

0.233 in.⁴ (eq. 4-18)

$lr = 0.08374$ in.⁴
 $Ar = 0.328$ sq.in.
 $Zr = 0.9385$ in.
 $t = 0.127$ in.
 $Rr = 25.3767$ in.

$Le = 1.1(Dt)^{0.5} =$	2.741 in.	if $Mx > 1.56$
$Le = Lr =$	30.000 in.	if $Mx < 1.56$
$Mx = 17.03$	\implies	$Le = 2.741$ in.

$N = 4$ use N that gives PeG min. $2 < N$

$PeG = 1036.55$ psi (eq. 4-17)

b. Imperfection Factors

$aoG = 0.8$

$FreG$ or $FheG = (aoG)(PeG)(Ro)(KoG)/t = 85.111$ psi (eq. 4-16)

c. Inelastic Buckling Stresses

$FrcG = n (FreG)$ (eq. 4-19)

$FhcG = n (FheG)$ (eq. 4-20)

$n = 1.0$	if $d \leq 0.55$	$d = FreG/Fy = 3.4044$
$n = 0.45/d + 0.18 = 0.312$	if $0.55 < d \leq 1.6$	$n = 0.267$
$n = 1.31/(1 + 1.15d) = 0.267$	if $1.6 < d < 6.25$	
$n = 1/d = 0.294$	if $d \geq 6.25$	

(eq. 5-3)

$FrcG = n (FreG) = 22.654$ psi (eq. 4-10)

d. Failure Pressures

$poG = (n)(aoG)(peG) = 221.014$ psi (eq. 4-12)

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum, No Seismic Support Settlement of 0.250"	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD DJW	MADE BY	CHKD BY	SHT 4 OF 6
	DATE 3/9/94	DATE 19 April 94	DATE	DATE	7.38

SECTION 6 PREDICTED SHELL BUCKLING STRESSES FOR COMBINED LOADS

Section 6.3 Axial Compression, Bending and Hoop Compression

$(Ra)^2 - c (Ra)(Rh) + (Rh)^2 = 1.0$ (eq. 6-3)

$Ra = F_{lcj} / F_{xcj}$ $Rh = F_{ocj} / F_{rcj}$

a. Unstiffened and Ring Stiffened Cylinders (all buckling modes)

$c = (F_{xcj} + F_{rcj}) / F_y - 1.0$ (eq. 6-4)

Local Buckling

$c = [F_{xcL} + F_{rcL}] / F_y - 1 = -0.07693$

Let $F_{lcL} = (F_{ocL}) (k) (K_{xL} / K_{oL}) = 1.1146 (F_{ocL})$

$Ra = F_{lcL} / F_{xcL} = 6.86E-05 (F_{ocL})$

$Rh = F_{ocL} / F_{rcL} = 0.000147 (F_{ocL})$

$K_{xL} = 1.0$

$K_{oL} = 1.0$ if $M_x \geq 3.42$
 $K_{oL} = 1 - e^*y = 1.000$ if $M_x < 3.42$ (eq. 11-7)
 $M_x = 17.03 \implies K_{oL} = 1.000$

$e = (1 - 0.3k) / [1 + (Le) / \sqrt{A}]$
 $A = (Ar)(R/Rr)^2 = 0.304196$ sq.in.
 $Ar = 0.328$ sq.in.
 $Rr = 25.377$ in.
 $Le = [1.56(Rt)^{.5}] + tw = 2.748297$ in.
 $k = N_x / N_o = 1.115$
 $e = 0.310$

Solve eq. 6-3 for F_{ocL}

$F_{ocL} = 6,092$ psi (eq. 6-3)

$F_{lcL} = (F_{ocL}) (k) (K_{xL} / K_{oL}) = 6,790$ psi

General Instability

$c = [F_{xcG} + F_{rcG}] / F_y - 1 = 0.709645$

$K_{xG} = 1.0$

$K_{oG} = (1 - 0.3k)(Le)(t) / [(Ar) + (Le)(t)] = 0.3432$ (eq. 11-9)
 (If $K_{oG} < 0$ then ring is in tension and the interaction equation is not applicable. Then $F_{lcG} = F_{xcG}$)

$k = N_x / N_o = 1.115$
 $Ar = 0.328$ sq.in.
 $Le = [1.56(Rt)^{.5}] + tw = 2.748297$ in.

Let $F_{lcG} = (F_{ocG}) (k) (K_{xG} / K_{oG}) = 3.248037 (F_{ocG})$

$Ra = F_{lcG} / F_{xcG} = 0.000162 (F_{ocG})$

$Rh = F_{ocG} / F_{rcG} = 4.41E-05 (F_{ocG})$

Solve eq. 6-3 for F_{ocG}

$F_{ocG} = 6,579$ psi (eq. 6-3)

$F_{lcG} = (F_{ocG}) (k) (K_{xG} / K_{oG}) = 21,369$ psi

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum, No Seismic Support Settlement of 0.250"	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD LW	MADE BY	CHKD BY	SHT 5 OF 6
	DATE 3/9/94	DATE 10 MAR 94	DATE	DATE	7, 39

SECTION 9		ALLOWABLE STRESSES	
Section 9.1		Allowable Stresses For Shell Buckling Mode	
Section 9.1.5		Axial Compression and Hoop Compression and Axial Compression, Bending and Hoop Compression	
$F_a = F_b = F_{cL} / FS$ (eq. 9-7)			
$F_o = F_{ocL} / FS$			
Factor of Safety (FS) = 2.14			
$F_a = F_b = 3,173 \text{ psi} = 402.98 \text{ #/in.}$		$N_x = 401.45 \text{ #/in.}$	
$F_c = 2,847 \text{ psi} = 361.55 \text{ #/in.}$		$N_o = 360.18 \text{ #/in.}$	
SECTION 11		STRESS CALCULATIONS	
Section 11.3		Hoop Stresses	
Section 11.3(b)2		Stress In a Ring Stiffener	
$\text{Ring Stiffener Stress} = (p \cdot R_o / t) \cdot (1.00 - 0.30 \cdot (N_x / N_o)) \cdot (L_e \cdot t) / (A_r + L_e \cdot t)$ (Eq 11-8)			
Solving Eq 11-8,			
$\Rightarrow \text{Ring Stiffener Stress} = 2,146 \text{ psi}$			
SUBJECT		OFFICE: NOE	REVISION
LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum, No Seismic Support Settlement of 0.250"		MADE BY WJC	CHKD RSW
		MADE BY	CHKD BY
		DATE 3/9/94	DATE 18 MAR 94
		REFERENCE NO. 930212	SHT 6 OF 6 7.40

**API BULLETIN 2U, FIRST EDITION, MAY 1, 1987
BULLETIN ON STABILITY DESIGN OF CYLINDRICAL SHELLS**

SECTION 4

**PREDICTED SHELL BUCKLING STRESSES FOR AXIAL
LOAD, BENDING AND EXTERNAL PRESSURE**

Ring Stiffener

Lr =	30.00	in.	b =	0.00	in.	tw =	0.1875	in.
R =	24.439	in.	Fy =	25,000	psi	Ar =	0.3280	sq.in.
t =	0.127	in.	E =	28,300,000	psi	Zr =	0.9385	in.
(Rt) ^{.5} =	1.762	in.	R/t =	192.43		lr =	0.08374	in. ^{.4}
Mx = Lr/(Rt) ^{.5} =	17.03	(eq. 4.1)				Fr =	25.377	in.
Mc = b/(Rt) ^{.5} =	0.00	(eq. 4.1)				Le = [1.56(Rt) ^{.5}] + tw =	2.936	in.
Nx =	551.18	#/in.	No =	360.18	#/in.			
	= 4,340	psi (Axial)		= 2,836	psi (Hoop)			

Section 4.1 Local Buckling of Unstiffened or Ring Stiffened Cylinders

Section 4.1.1 Axial Compression or Bending (No = 0)

a. Elastic Buckling Stresses

$F_{xeL} = (axL)(Cx)(E)(t/R)$ (eq. 4-2)

Cx = 0.630	if Mx ≤ 1.5	(eq. 4-3)
Cx = $[0.904/(Mx)^2] + 0.1013(Mx)^2 = 2.941$	if 1.5 < Mx < 1.73	
Cx = 0.605	if Mx > 1.73	

Mx = 17.03 → Cx = 0.605

axL = 0.207	if R/t ≥ 610	(eq. 4-4)
axL = $169c/(195 + R/t) < 0.9 = 0.436$	if R/t < 610	

R/t = 192 → axL = 0.436

c = 2.64	if Mx ≤ 1.5	(eq. 4-5)
c = $3.13/(Mx)^{0.42} = 0.952$	if 1.5 < Mx ≤ 15	
c = 1.0	if Mx > 15	

Mx = 17.03 → c = 1.000

$F_{xeL} = (axL)(Cx)(E)(t/R) = 38,812 \text{ psi}$ (eq. 4-2)

b. Inelastic Buckling Stresses

Smaller of:

$F_{xcL} = F_{xeL} = 38,812 \text{ psi}$ eq. 4-6

$F_{xcL} = 233(Fy)/(166 + R/t) = 16,251 \text{ psi}$	if R/t < 300	(eq. 4-7)
$F_{xcL} = 0.5(Fy) = 12,500 \text{ psi}$	if R/t ≥ 300	

R/t = 192 → $F_{xcL} = 16,251 \text{ psi}$

SUBJECT

LIGO Beam Tube -- 49.00" OD x 0.127" Wall
1.75" x 0.1875" Vacuum Stiffener, 30" Spacing
100F Operating + Vacuum, No Seismic
Support Settlement of 0.579"

OFFICE: NOE

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CHKD BY

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3/9/94

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Section 4.1.2 External Pressure ($N_x/N_o = 0$ or 0.5)

Use $N_x/N_o = 0$
 $N_x/N_o = 0$ for radial pressure
 $N_x/N_o = 0.5$ for hydrostatic pressure

a. Elastic Buckling Stresses

F_{reL} or $F_{heL} = (a_c L)(P_{eL})(R)/(K_o L)^2$ (eq. 4-8)

$K_o L = 1.0$	if	$M_x \geq 3.42$
$K_o L = 1 - e^* y = 1.000$	if	$M_x < 3.42$ (eq. 11-7)
$M_x = 17.03$	\implies	$K_o L = 1.000$

$e = (1 - 0.3k)[1 + (L_o)/A]$
$A = (A_r)/(R/R_r)^2 = 0.304196$ sq.in.
$A_r = 0.328$ sq.in.
$R_r = 25.377$ in.
$L_o = [1.56(R/t)^{.5}] + t_w = 2.935797$ in.
$k = N_x/N_o = 0$
$e = 0.449$

$y = 1.0$	if	$M_x \leq 1.26$
$y = 1.58 - 0.46 M_x = -6.25$	if	$1.26 < M_x \leq 3.42$
$y = 0.0$	if	$M_x \geq 3.42$
$M_x = 17.03$	\implies	$y = 0.000$

$P_{eL} = [1.27 (E) (t/R)^2] / [A^{1.18} + 0.5] = 36.5161$	if $M_x > 1.5$ & $A < 2.5$	$A = M_x - 1.17 + 1.068k = 15.859$
$P_{eL} = [0.92 (E) (t/R)^2] / A = 44.3368$	if $2.5 < A < 0.208 R/t$	$M_x = 17.03$
$P_{eL} = 0.836(C_p)^{-1.061} (E)(t/R)^3 = 46.9145$	if $0.208 < C_p < 2.85$	$k = 0$
$P_{eL} = 0.275 (E)(t/R)^3 = 1.0922$	if $C_p > 2.85$	$0.208 R/t = 40.02526$
$P_{eL} = 44.3368$		$C_p = A/(R/t) = 0.082413$

b. Imperfection Factors

$a_o L = 0.8$

F_{reL} or $F_{heL} = (a_o L)(P_{eL})(R)/(K_o L)^2 = 6,825$ psi (eq. 4-8)

c. Inelastic Buckling Stresses

$F_{rcL} = n (F_{reL})$ (eq. 4-10)

$F_{hcL} = n (F_{heL})$ (eq. 4-11)

$n = 1.0$	if $d \leq 0.55$	$d = F_{reL}/F_y = 0.2730$
$n = 0.45/d + 0.18 = 1.83$	if $0.55 < d \leq 1.6$	$n = 1.000$ (eq. 5-3)
$n = 1.31/(1 + 1.15d) = 1.00$	if $1.6 < d < 6.25$	
$n = 1/d = 3.66$	if $d \geq 6.25$	

$F_{rcL} = n (F_{reL}) = 6,825$ psi (eq. 4-10)

d. Failure Pressures

$p_{oL} = (n)(a_o L)(p_{eL}) = 35,469$ psf (eq. 4-12)

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum, No Seismic Support Settlement of 0.579"	OFFICE: NOE	REVISION	REFERENCE NO. 930212	
	MADE BY WJC	CHKD RJW	MADE BY CHKD BY	
	DATE 3/9/94	DATE 18 MAR 94	DATE DATE	SHT 2 OF 6 7.42

Section 4.2 General Instability of Ring Stiffened Cylinders

Section 4.2.1 Axial Compression or Bending (No = 0)

a. Elastic Buckling Stresses

$F_{xeG} = (axG)(0.605)(E)(t/R)(1 + A'r)^{0.5}$ (eq. 4-13)

$A'r = Ar/[Lr] t] = 0.0861$ $Ar = \text{ring area} = 0.328 \text{ sq.in.}$
 $Lr = \text{ring spacing} = 30 \text{ in.}$

b. Imperfection Factors

$axG = 0.72$		if $A'r \geq 0.2$	
$axG = [3.6 - 5.0(axL)] (A'r) + axL = 0.558$		if $0.06 < A'r < 0.2$	(eq. 4-14)
$axG = axL = 0.436209$		if $A'r \leq 0.06$	
$axG = 0.558$		axL from eq. 4-4 with $c = 1.0$	

$F_{xeG} = (axG)(0.605)(E)(t/R)(1 + A'r)^{0.5} = 51,775 \text{ psi}$ (eq. 4-13)

c. Inelastic Buckling Stresses

$F_{xcG} = n (F_{xeG})$ (eq. 4-15)

$n = 1.0$		if $d \leq 0.55$	$d = F_{xeG}/F_y = 2.0710$	(eq. 5-3)
$n = 0.45/d + 0.18 = 0.40$		if $0.55 < d \leq 1.6$		
$n = 1.31/(1 + 1.15d) = 0.39$		if $1.6 < d < 6.25$		
$n = 1/d = 0.48$		if $d \geq 6.25$		

$F_{xcG} = n (F_{xeG}) = 20,057 \text{ psi}$ (eq. 4-15)

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum, No Seismic Support Settlement of 0.579"	OFFICE: NOE	REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RWN	MADE BY	CHKD BY
	DATE 3/9/94	DATE 18 MAR 94	DATE	DATE
				SHT 3 OF 6 7.43

Section 4.2.2 External Pressure (Nx/No = 0 or 0.5)

Use Nx/No = 0
 Nx/No = 0 for radial pressure
 Nx/No = 0.5 for hydrostatic pressure

a. Buckling Stresses With or Without End Pressure

$FreG$ or $FheG = (acG)(PeG)(Ro)(KoG)/t$ (eq. 4-16)

$KoG = (1 - 0.3k)(Le)(t)/[(Ar + (Le)(t))] =$

0.531994 (eq. 11-9)

Ar = 0.328 sq.in.
 Le = [1.56(Rt)^.5] + tw = 2.935797 in.
 k = 0

$PeG = \frac{(E)(tR)(Lg)^4}{[N^2 + k(Lg)^2 - 1][N^2 + (Lg)^2]^2}$

$+ \frac{(E)(I_r)(N^2 - 1)}{(L_r)(R_c)^2(R)}$ (eq. 4-17)

$\lambda G (Lg) = (\pi)(R)/Lb = 2.559194$

Lb = 30 in.

k = 0

$I_r = I_r + (Ar)(Z_r)^2 (L_e)(t)/[(Ar) + (L_e)(t)] + (L_e)(t)^3/12 = 0.233 \text{ in.}^4$ (eq. 4-18)

- Ir = 0.08374 in.^4
- Ar = 0.328 sq.in.
- Zr = 0.9385 in.
- t = 0.127 in.
- Rr = 25.3767 in.

Le = 1.1(Dt)^0.5 = 2.741 in. if Mx > 1.56
 Le = Lr = 30.000 in. if Mx < 1.56
 Mx = 17.03 ==> Le = 2.741 in.

N = 4 use N that gives PeG min. 2 < N

PeG = 1036.55 psi (eq. 4-17)

b. Imperfection Factors

aoG = 0.8

$FreG$ or $FheG = (aoG)(PeG)(Ro)(KoG)/t = 85.111 \text{ psi}$ (eq. 4-16)

c. Inelastic Buckling Stresses

$FrcG = n (FreG)$ (eq. 4-19)

$FhcG = n (FheG)$ (eq. 4-20)

n = 1.0	if d <= 0.55	d = FreG/Fy = 3.4044	(eq. 5-3)
n = 0.45/d + 0.18 = 0.312	if 0.55 < d <= 1.6	n = 0.267	
n = 1.31/(1 + 1.15d) = 0.267	if 1.6 < d < 6.25		
n = 1/d = 0.294	if d >= 6.25		

$FrcG = n (FreG) = 22.884 \text{ psi}$ (eq. 4-10)

d. Failure Pressures

$poG = (n)(aoG)(peG) = 221.014 \text{ psi}$ (eq. 4-12)

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum, No Seismic Support Settlement of 0.579"	OFFICE: NOE	REVISION	REFERENCE NO. 930212
	MADE BY WJC	CHKD RJN	MADE BY CHKD BY
	DATE 3/9/94	DATE 10 MAR 94	DATE DATE
			SHT 4 OF 6 7.44

SECTION 6 PREDICTED SHELL BUCKLING STRESSES FOR COMBINED LOADS

Section 6.3 Axial Compression, Bending and Hoop Compression

$(Ra)^2 - c (Ra)(Rh) + (Rh)^2 = 1.0$ (eq. 6-3)

$Ra = Fc_j / Fxc_j$ $Rh = Foc_j / Frc_j$

a. Unstiffened and Ring Stiffened Cylinders (all buckling modes)

$c = (Fxc_j + Frc_j) / Fy - 1.0$ (eq. 6-4)

Local Buckling

$c = [FxcL + FrcL] / Fy - 1 = -0.07693$

Let $FicL = (FocL) (k) (KxL/KoL) = 1.5303 (FocL)$

$Ra = FicL / FxcL = 9.42E-05 (FocL)$

$Rh = FocL / FrcL = 0.000147 (FocL)$

$KxL = 1.0$

$KoL = 1.0$ if $Mx \geq 3.42$
 $KoL = 1 - e^y = 1.000$ if $Mx < 3.42$ (eq. 11-7)
 $Mx = 17.03 \implies KoL = 1.000$

$e = (1 - 0.3k) / [1 + (Le) / VA]$
 $A = (Ar)(R/Rr)^2 = 0.304196$ sq.in.
 $Ar = 0.328$ sq.in.
 $Rr = 25.377$ in.
 $Le = [1.56(Rt)^{.5}] + tw = 2.748297$ in.
 $k = Nx/No = 1.530$
 $e = 0.252$

Solve eq. 6-3 for $FocL$

$FocL = 5,644$ psi (eq. 6-3)

$FicL = (FocL) (k) (KxL/KoL) = 8,637$ psi

General Instability

$c = [FxcG + FrcG] / Fy - 1 = 0.709645$

$KxG = 1.0$

$KoG = (1 - 0.3k)(Le)(t) / [Ar + (Le)(t)] = 0.2789$ (eq. 11-9)
 (If $KoG < 0$ then ring is in tension and the interaction equation is not applicable. Then $FicG = FxcG$)

$k = Nx / No = 1.530$
 $Ar = 0.328$ sq.in.
 $Le = [1.56(Rt)^{.5}] + tw = 2.748297$ in.

Let $FicG = (FocG) (k) (KxG/KoG) = 5.487705 (FocG)$

$Ra = FicG / FxcG = 0.000274 (FocG)$

$Rh = FocG / FrcG = 4.41E-05 (FocG)$

Solve eq. 6-3 for $FocG$

$FocG = 3,028$ psi (eq. 6-3)

$FicG = (FocG) (k) (KxG/KoG) = 21,007$ psi

SUBJECT

LIGO Beam Tube -- 49.00" OD x 0.127" Wall
 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing
 100F Operating + Vacuum, No Seismic
 Support Settlement of 0.579"

OFFICE: NOE		REVISION		REFERENCE NO. 930212
MADE BY WJC	CHKD RJA	MADE BY	CHKD BY	SHT 5 OF 6
DATE 3/9/94	DATE 10 MAR 94	DATE	DATE	7.45

SECTION 9 ALLOWABLE STRESSES

Section 9.1 Allowable Stresses For Shell Buckling Mode

Section 9.1.5 Axial Compression and Hoop Compression and Axial Compression, Bending and Hoop Compression

$F_a = F_b = F_{iCL} / FS$
(eq. 9-7)

$F_o = F_{oCL} / FS$

Factor of Safety (FS) = 1.99

$F_a = F_b = 4,340 \text{ psi} = 551.18 \text{ \#/in.}$

$N_x = 551.18 \text{ \#/in.}$

$F_o = 2,636 \text{ psi} = 360.18 \text{ \#/in.}$

$N_o = 360.18 \text{ \#/in.}$

SECTION 11 STRESS CALCULATIONS

Section 11.3 Hoop Stresses

Section 11.3(b)2 Stress in a Ring Stiffener

Ring Stiffener Stress = $(p \cdot R_o / t) \cdot (1.00 - 0.30 \cdot (N_x / N_o)) \cdot (L_e \cdot t) / (A_r + L_e \cdot t)$ (Eq 11-8)

Solving Eq 11-8,

\Rightarrow Ring Stiffener Stress = 1,744 psi

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum, No Seismic Support Settlement of 0.579"	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD <i>RSW</i>	MADE BY	CHKD BY	SHT 6 OF 6
	DATE 3/9/94	DATE <i>18 MAR 94</i>	DATE	DATE	<i>7.4b</i>

API BULLETIN 2U, FIRST EDITION, MAY 1, 1987
BULLETIN ON STABILITY DESIGN OF CYLINDRICAL SHELLS

SECTION 4

**PREDICTED SHELL BUCKLING STRESSES FOR AXIAL
LOAD, BENDING AND EXTERNAL PRESSURE**

Ring Stiffener

Lr =	30.00 in.	b =	0.00 in.	tw =	0.1875 in.
R =	24.439 in.	Fy =	25,000 psi	Ar =	0.3280 sq.in.
t =	0.127 in.	E =	28,300,000 psi	Zr =	0.9385 in.
(Rt) ^{.5} =	1.762 in.	R/t =	192.43	lr =	0.08374 in. ^{.4}
Mx = Lr/(Rt) ^{.5} =	17.03 (eq. 4.1)			Rr =	25.377 in.
Mc = b/(Rt) ^{.5} =	0.00 (eq. 4.1)			Le = [1.56(Rt) ^{.5} + tw =	2.936 in.

Nx =	617.35 #/in.	No =	360.18 #/in.
	= 4,861 psi (Axial)		= 2,836 psi (Hoop)

Section 4.1 Local Buckling of Unstiffened or Ring Stiffened Cylinders

Section 4.1.1 Axial Compression or Bending (No = 0)

a. Elastic Buckling Stresses

$F_{xeL} = (axL)(Cx)(E)(t/R)$ (eq. 4-2)

Cx = 0.630	if Mx ≤ 1.5	(eq. 4-3)
Cx = [0.904/(Mx) ²] + 0.1013(Mx) ² = 2.941	if 1.5 < Mx < 1.73	
Cx = 0.605	if Mx > 1.73	

Mx = 17.03 → Cx = 0.605

axL = 0.207	if R/t ≥ 610	(eq. 4-4)
axL = 169c/(195 + R/t) < 0.9 = 0.436	if R/t < 610	

R/t = 192 → axL = 0.436

c = 2.64	if Mx ≤ 1.5	(eq. 4-5)
c = 3.13/(Mx) ^{0.42} = 0.952	if 1.5 < Mx ≤ 15	
c = 1.0	if Mx > 15	

Mx = 17.03 → c = 1.000

$F_{xeL} = (axL)(Cx)(E)(t/R) = 38,812 \text{ psi}$ (eq. 4-2)

b. Inelastic Buckling Stresses

Smaller of:

$F_{xcL} = F_{xeL} = 38,812 \text{ psi}$ eq. 4-6

FxcL = 233(Fy)/(166 + R/t) ≤ Fy = 16,251 psi	if R/t < 300	(eq. 4-7)
FxcL = 0.5(Fy) = 12,500 psi	if R/t ≥ 300	

R/t = 192 → FxcL = 16,251 psi

SUBJECT

LIGO Beam Tube -- 49.00" OD x 0.127" Wall
1.75" x 0.1875" Vacuum Stiffener, 30" Spacing
100F Operating + Vacuum + Axial Seismic
Support Settlement of 0.579"

OFFICE: NOE

REVISION

REFERENCE NO.

930212

MADE BY
WJC

CHKD
RJA

MADE BY
CHKD BY

SHT 1 OF 6

DATE
3/10/94

DATE
10/11/94

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DATE

DATE
DATE

7.47

Section 4.1.2 External Pressure (Nx/No = 0 or 0.5)

Use Nx/No = 0
 Nx/No = 0 for radial pressure
 Nx/No = 0.5 for hydrostatic pressure

a. Elastic Buckling Stresses

F_{reL} or $F_{heL} = (acL)(P_{eL})(R)(K_{oL})/t$ (eq. 4-8)

$K_{oL} = 1.0$	if	$Mx \geq 3.42$
$K_{oL} = 1 - e^*y = 1.000$	if	$Mx < 3.42$ (eq. 11-7)
$Mx = 17.03$	\implies	$K_{oL} = 1.000$

$e = (1 - 0.3k)[1 + (L_e)/VA]$
$A = (Ar)(R/Rr)^2 = 0.304196$ sq.in.
$Ar = 0.328$ sq.in.
$Rr = 25.377$ in.
$L_e = [1.56(Rt)^{.5}] + tw = 2.935797$ in.
$k = Nx/No = 0$
$e = 0.449$

$y = 1.0$	if	$Mx \leq 1.26$
$y = 1.58 - 0.46 Mx = -6.25$	if	$1.26 < Mx \leq 3.42$
$y = 0.0$	if	$Mx \geq 3.42$
$Mx = 17.03$	\implies	$y = 0.000$

$P_{eL} = [1.27 (E) (V/R)^2] / [A^{1.18} + 0.5] = 36.5161$	if	$Mx > 1.5$ & $A < 2.5$	$A = Mx - 1.17 + 1.068k = 15.859$
$P_{eL} = [0.92 (E) (V/R)^2] / A = 44.3368$	if	$2.5 < A < 0.208 R/t$	$Mx = 17.03$
$P_{eL} = 0.836(Cp)^{-1.061} (E)(V/R)^3 = 46.9145$	if	$0.208 < Cp < 2.85$	$k = 0$
$P_{eL} = 0.275 (E)(V/R)^3 = 1.0922$	if	$Cp > 2.85$	$0.208 R/t = 40.02526$
$P_{eL} = 44.3368$			$Cp = A/(R/t) = 0.082413$

b. Imperfection Factors

$a_{oL} = 0.8$

F_{reL} or $F_{heL} = (a_{oL})(P_{eL})(R)(K_{oL})/t = 5.825$ psi (eq. 4-8)

c. Inelastic Buckling Stresses

$F_{rcL} = n (F_{reL})$ (eq. 4-10)

$F_{hcL} = n (F_{heL})$ (eq. 4-11)

$n = 1.0$	if	$d \leq 0.55$	$d = F_{reL}/F_y = 0.2730$
$n = 0.45/d + 0.18 = 1.83$	if	$0.55 < d \leq 1.6$	$n = 1.000$ (eq. 5-3)
$n = 1.31/(1 + 1.15d) = 1.00$	if	$1.6 < d < 6.25$	
$n = 1/d = 3.66$	if	$d \geq 6.25$	

$F_{rcL} = n (F_{reL}) = 6.825$ psi (eq. 4-10)

d. Failure Pressures

$p_{eL} = (n)(a_{oL})(P_{eL}) = 35.469$ psi (eq. 4-12)

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum + Axial Seismic Support Settlement of 0.579"	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RjN	MADE BY	CHKD BY	SHT 2 OF 6
	DATE 3/10/94	DATE 18MARCH	DATE	DATE	7.48

Section 4.2 General Instability of Ring Stiffened Cylinders

Section 4.2.1 Axial Compression or Bending (No = 0)

a. Elastic Buckling Stresses

$F_{xeG} = (axG)(0.605)(E)(t/R)(1 + A'r)^{0.5}$ (eq. 4-13)

$A'r = Ar/[Lr] t] = 0.0861$ $Ar = \text{ring area} = 0.328 \text{ sq.in.}$
 $Lr = \text{ring spacing} = 30 \text{ in.}$

b. Imperfection Factors

$axG = 0.72$	if $A'r \geq 0.2$	(eq. 4-14)
$axG = [3.6 - 5.0(axL)] (A'r) + axL = 0.558$	if $0.06 < A'r < 0.2$	
$axG = axL = 0.436209$	if $A'r \leq 0.06$	
$axG = 0.558$	axL from eq. 4-4 with $c = 1.0$	

$F_{xeG} = (axG)(0.605)(E)(t/R)(1 + A'r)^{0.5} = 51,775 \text{ psi}$ (eq. 4-13)

c. Inelastic Buckling Stresses

$F_{xcG} = n (F_{xeG})$ (eq. 4-15)

$n = 1.0$	if $d \leq 0.55$	(eq. 5-3)	
$n = 0.45/d + 0.18 = 0.40$	if $0.55 < d \leq 1.6$		
$n = 1.31/(1 + 1.15d) = 0.39$	if $1.6 < d < 6.25$		
$n = 1/d = 0.48$	if $d \geq 6.25$		
		$d = F_{xeG}/F_y = 2.0710$	$n = 0.387$

$F_{xcG} = n (F_{xeG}) = 20,057 \text{ psi}$ (eq. 4-15)

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum + Axial Seismic Support Settlement of 0.579"	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD <i>RW</i>	MADE BY	CHKD BY	SHT 3 OF 6
	DATE 3/10/94	DATE <i>10 MAR 94</i>	DATE	DATE	7.49

Section 4.2.2 External Pressure (Nx/No = 0 or 0.5)

Use Nx/No = 0
 Nx/No = 0 for radial pressure
 Nx/No = 0.5 for hydrostatic pressure

a. Buckling Stresses With or Without End Pressure

$FreG$ or $FheG = (acG)(PeG)(Ro)(KoG)/t$ (eq. 4-16)

$KoG = (1 - 0.3k)(Le)(t)/[(Ar + (Le)(t))] = 0.531994$ (eq. 11-9)

$Ar = 0.328$ sq.in.
 $Le = [1.56(Rt)^{.5}] + tw = 2.935797$ in.
 $k = 0$

$PeG = \frac{(E)(tR)(Lg)^4}{[N^2 + k(Lg)^2 - 1][N^2 + (Lg)^2]^2} + \frac{(E)(ler)(N^2 - 1)}{(Lr)(Rc)^2(R)}$ (eq. 4-17)

$\lambda G (Lg) = (\pi)(R)/Lb = 2.559194$ $Lb = 30$ in.

$k = 0$

$ler = lr + (Ar)(Zr)^2 (Le)(t)/[(Ar) + (Le)(t)] + (Le)(t)^3/12 = 0.233$ in.^4 (eq. 4-18)

- $lr = 0.08374$ in.^4
- $Ar = 0.328$ sq.in.
- $Zr = 0.9385$ in.
- $t = 0.127$ in.
- $Rr = 25.3767$ in.

$Le = 1.1(Dt)^{0.5} = 2.741$ in.	if $Mx > 1.56$
$Le = Lr = 30.000$ in.	if $Mx < 1.56$
$Mx = 17.03 \implies Le = 2.741$ in.	

$N = 4$ use N that gives PeG min. $2 < N$

$PeG = 1036.55$ psi (eq. 4-17)

b. Imperfection Factors

$aoG = 0.8$

$FreG$ or $FheG = (aoG)(PeG)(Ro)(KoG)/t = 35,111$ psi (eq. 4-16)

c. Inelastic Buckling Stresses

$FrcG = n (FreG)$ (eq. 4-19)

$FhcG = n (FheG)$ (eq. 4-20)

$n = 1.0$	if $d \leq 0.55$	$d = FreG/Fy = 3.4044$	(eq. 5-3)
$n = 0.45/d + 0.18 = 0.312$	if $0.55 < d \leq 1.6$		
$n = 1.31/(1 + 1.15d) = 0.267$	if $1.6 < d < 6.25$	$n = 0.267$	
$n = 1/d = 0.294$	if $d \geq 6.25$		

$FrcG = n(FreG) = 22,684$ psi (eq. 4-10)

d. Failure Pressures

$poG = (n)(aoG)(peG) = 221,014$ psi (eq. 4-12)

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum + Axial Seismic Support Settlement of 0.579"	OFFICE: NOE	REVISION	REFERENCE NO. 930212	
	MADE BY WJC	CHKD RJW	MADE BY CHKD BY	SHT 4 OF 6
	DATE 3/10/94	DATE 10 MAR 94	DATE DATE	DATE 7.50

SECTION 6 PREDICTED SHELL BUCKLING STRESSES FOR COMBINED LOADS

Section 6.3 Axial Compression, Bending and Hoop Compression

$(Ra)^2 - c (Ra)(Rh) + (Rh)^2 = 1.0$ (eq. 6-3)

$Ra = Fc_j / Fxc_j$ $Rh = Foc_j / Frc_j$

a. Unstiffened and Ring Stiffened Cylinders (all buckling modes)

$c = (Fxc_j + Frc_j) / Fy - 1.0$ (eq. 6-4)

Local Buckling

$c = [FxcL + FrcL] / Fy - 1 = -0.07693$

Let $FicL = (FocL) (k) (KxL/KoL) = 1.7140 (FocL)$

$Ra = FicL / FxcL = 0.000105 (FocL)$

$KxL = 1.0$

$Rh = FocL / FrcL = 0.000147 (FocL)$

$KoL = 1.0$ if $Mx \geq 3.42$
 $KoL = 1 - e^*y = 1.000$ if $Mx < 3.42$ (eq. 11-7)
 $Mx = 17.03 \implies KoL = 1.000$

$e = (1 - 0.3k) / [1 + (Le) / A]$
 $A = (Ar)(R/Rr)^2 = 0.304196$ sq.in.
 $Ar = 0.328$ sq.in.
 $Rr = 25.377$ in.
 $Le = [1.56(Rt)^{.5}] + tw = 2.748297$ in.
 $k = Nx/No = 1.714$
 $e = 0.226$

Solve eq. 6-3 for $FocL$

$FocL = 5,441$ psi (eq. 6-3)

$FicL = (FocL) (k) (KxL/KoL) = 9,326$ psi

General Instability

$c = [FxcG + FrcG] / Fy - 1 = 0.709645$

$KxG = 1.0$

$KoG = (1 - 0.3k)(Le)(t) / [(Ar) + (Le)(t)] = 0.2504$ (eq. 11-9)
 (if $KoG < 0$ then ring is in tension and the interaction equation is not applicable. Then $FicG = FxcG$)

$k = Nx / No = 1.714$
 $Ar = 0.328$ sq.in.
 $Le = [1.56(Rt)^{.5}] + tw = 2.748297$ in.

Let $FicG = (FocG) (k) (KxG/KoG) = 6.843771 (FocG)$

$Ra = FicG / FxcG = 0.000341 (FocG)$

$Rh = FocG / FrcG = 4.41E-05 (FocG)$

Solve eq. 6-3 for $FocG$

$FocG = 3,047$ psi (eq. 6-3)

$FicG = (FocG) (k) (KxG/KoG) = 20,854$ psi

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum + Axial Seismic Support Settlement of 0.579"	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RJW	MADE BY	CHKD BY	SHT 5 OF 6
	DATE 3/10/94	DATE 10 MAR 94	DATE	DATE	7.51

API BULLETIN 2U, FIRST EDITION, MAY 1, 1987
BULLETIN ON STABILITY DESIGN OF CYLINDRICAL SHELLS

SECTION 4

**PREDICTED SHELL BUCKLING STRESSES FOR AXIAL
LOAD, BENDING AND EXTERNAL PRESSURE**

Ring Stiffener

Lr =	30.00 in.	b =	0.00 in.	tw =	0.1875 in.
R =	24.439 in.	Fy =	25,000 psi	Ar =	0.3280 sq.in.
t =	0.127 in.	E =	28,300,000 psi	Zr =	0.9385 in.
(Rt) ^{.5} =	1.762 in.	R/t =	192.43	kr =	0.08374 in. ^{.4}
Mx = Lr/(Rt) ^{.5} =	17.03 (eq. 4.1)			Rr =	25.377 in.
Mc = b/(Rt) ^{.5} =	0.00 (eq. 4.1)			Le = [1.56(Rt) ^{.5}] + tw =	2.936 in.
Nx =	748.41 #/in.	No =	360.18 #/in.		
	= 5,893 psi (Axial)		= 2,836 psi (Hoop)		

Section 4.1 Local Buckling of Unstiffened or Ring Stiffened Cylinders

Section 4.1.1 Axial Compression or Bending (No = 0)

a. Elastic Buckling Stresses

$F_{xeL} = (axL)(Cx)(E)/(tR)$ (eq. 4-2)

Cx = 0.630	if Mx <= 1.5	(eq. 4-3)
Cx = [0.904/(Mx) ²] + 0.1013(Mx) ² = 2.941	if 1.5 < Mx < 1.73	
Cx = 0.605	if Mx > 1.73	

Mx = 17.03 → Cx = 0.605

axL = 0.207	if R/t >= 610	(eq. 4-4)
axL = 169c/(195 + R/t) < 0.9 = 0.436	if R/t < 610	

R/t = 192 → axL = 0.436

c = 2.64	if Mx <= 1.5	(eq. 4-5)
c = 3.13/(Mx) ^{0.42} = 0.952	if 1.5 < Mx <= 15	
c = 1.0	if Mx > 15	

Mx = 17.03 → c = 1.000

$F_{xeL} = (axL)(Cx)(E)/(tR) = 38,812 \text{ psi}$ (eq. 4-2)

b. Inelastic Buckling Stresses

Smaller of:

$F_{xcL} = F_{xeL} = 38,812 \text{ psi}$ eq. 4-6

$F_{xcL} = 233(Fy)/(166 + R/t) <= Fy = 16,251 \text{ psi}$	if R/t < 300	(eq. 4-7)
$F_{xcL} = 0.5(Fy) = 12,500 \text{ psi}$	if R/t >= 300	

R/t = 192 → $F_{xcL} = 16,251 \text{ psi}$

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum, No Seismic Support Misalignment of 1.000" during Adjustment	OFFICE: NOE	REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RJA	MADE BY	CHKD BY
	DATE 3/16/94	DATE 10/12/94	DATE	DATE
				SHT 1 OF 6 7.53

Section 4.1.2 External Pressure (Nx/No = 0 or 0.5)

Use Nx/No = 0
 Nx/No = 0 for radial pressure
 Nx/No = 0.5 for hydrostatic pressure

a. Elastic Buckling Stresses

F_{reL} or $F_{heL} = (a_c L)(P_e L)(R)(K_o L)/t$ (eq. 4-8)

$K_o L = 1.0$	if	$M_x \geq 3.42$
$K_o L = 1 - e^y = 1.000$	if	$M_x < 3.42$ (eq. 11-7)
$M_x = 17.03$	→	$K_o L = 1.000$

$e = (1 - 0.3k)/(1 + (L_e)/VA)$
$A = (A_r)(R/R_r)^2 = 0.304196$ sq.in.
$A_r = 0.328$ sq.in.
$R_r = 25.377$ in.
$L_e = [1.56(R_t)^{.5}] + t_w = 2.935797$ in.
$k = N_x/No = 0$
$e = 0.449$

$y = 1.0$	if	$M_x \leq 1.26$
$y = 1.58 - 0.46 M_x = -6.25$	if	$1.26 < M_x \leq 3.42$
$y = 0.0$	if	$M_x \geq 3.42$
$M_x = 17.03$	→	$y = 0.000$

$P_e L = [1.27 (E) (V/R)^2] / [A^{1.18 + 0.5}] = 36.5161$	if	$M_x > 1.5$ & $A < 2.5$	$A = M_x - 1.17 + 1.068k = 15.859$
$P_e L = [0.92 (E) (V/R)^2] / A = 44.3368$	if	$2.5 < A < 0.208 R/t$	$M_x = 17.03$
$P_e L = 0.836(C_p)^{-1.061} (E)(V/R)^3 = 46.9145$	if	$0.208 < C_p < 2.85$	$k = 0$
$P_e L = 0.275 (E)(V/R)^3 = 1.0922$	if	$C_p > 2.85$	$0.208 R/t = 40.02526$
$P_e L = 44.3368$			$C_p = A/(R/t) = 0.082413$

b. Imperfection Factors

$a_o L = 0.8$

F_{reL} or $F_{heL} = (a_o L)(P_e L)(R)(K_o L)/t = 6.525$ psi (eq. 4-8)

c. Inelastic Buckling Stresses

$F_{rcL} = n (F_{reL})$ (eq. 4-10)

$F_{hcL} = n (F_{heL})$ (eq. 4-11)

$n = 1.0$	if	$d \leq 0.55$	$d = F_{reL}/F_y = 0.2730$
$n = 0.45/d + 0.18 = 1.83$	if	$0.55 < d \leq 1.6$	$n = 1.000$ (eq. 5-3)
$n = 1.31/(1 + 1.15d) = 1.00$	if	$1.6 < d < 6.25$	
$n = 1/d = 3.66$	if	$d \geq 6.25$	

$F_{rcL} = n (F_{reL}) = 6.525$ psi (eq. 4-10)

d. Failure Pressures

$p_o L = (n)(a_o L)(p_e L) = 35.459$ psi (eq. 4-12)

SUBJECT LIGO Beam Tube – 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum, No Seismic Support Misalignment of 1.000" during Adjustment	OFFICE: NOE	REVISION	REFERENCE NO. 930212
	MADE BY WJC	CHKD RJV	MADE BY CHKD BY
	DATE 3/16/94	DATE 1/8/11/294	DATE DATE
			SHT 2 OF 6 7.54

Section 4.2 General Instability of Ring Stiffened Cylinders

Section 4.2.1 Axial Compression or Bending (No = 0)

a. Elastic Buckling Stresses

$F_{xeG} = (axG)(0.605)(E)(t/R)(1 + Ar)^{0.5}$ (eq. 4-13)

$A'r = Ar/(Lr) t = 0.0861$ $Ar = \text{ring area} = 0.328 \text{ sq.in.}$
 $Lr = \text{ring spacing} = 30 \text{ in.}$

b. Imperfection Factors

$axG = 0.72$	if $A'r \geq 0.2$	(eq. 4-14)
$axG = [3.6 - 5.0(axL)] (A'r) + axL = 0.558$	if $0.06 < A'r < 0.2$	
$axG = axL = 0.436209$	if $A'r \leq 0.06$	
$axG = 0.558$	axL from eq. 4-4 with $c = 1.0$	

$F_{xeG} = (axG)(0.605)(E)(t/R)(1 + Ar)^{0.5} = 61,775 \text{ psi}$ (eq. 4-13)

c. Inelastic Buckling Stresses

$F_{xcG} = n (F_{xeG})$ (eq. 4-15)

$n = 1.0$	if $d \leq 0.55$	(eq. 5-3)	
$n = 0.45/d + 0.18 = 0.40$	if $0.55 < d \leq 1.6$		
$n = 1.31/(1 + 1.15d) = 0.39$	if $1.6 < d < 6.25$		
$n = 1/d = 0.48$	if $d \geq 6.25$		
		$d = F_{xeG}/F_y = 2.0710$	$n = 0.387$

$F_{xcG} = n (F_{xeG}) = 20,057 \text{ psi}$ (eq. 4-15)

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	MADE BY WJC	CHKD Rjw	MADE BY	CHKD BY	SHT 3 OF 6
	DATE 3/16/94	DATE 18 MAR 94	DATE	DATE	7.55

Section 4.2.2 External Pressure (Nx/No = 0 or 0.5)

Use Nx/No = 0
 Nx/No = 0 for radial pressure
 Nx/No = 0.5 for hydrostatic pressure

a. Buckling Stresses With or Without End Pressure

$FreG$ or $FheG = (acG)(PeG)(Ro)(KoG)/t$ (eq. 4-16)

$KoG = (1 - 0.3k)(Le)(t)/[(Ar + (Le)(t))] = 0.531994$ (eq. 11-9)

$Ar = 0.328$ sq.in.
 $Le = [1.56(Rt)^{0.5}] + tw = 2.935797$ in.
 $k = 0$

$PeG = \frac{(E)(t/R)(Lg)^4}{[N^2 + k(Lg)^2 - 1][N^2 + (Lg)^2]^2} + \frac{(E)(ler)(N^2 - 1)}{(Lr)(Rc)^2(R)}$ (eq. 4-17)

$lamda G (Lg) = (pi)(R)/Lb = 2.559194$ $Lb = 30$ in.

$k = 0$

$ler = lr + (Ar)(Zr)^2(Lt)/[(Ar + (Le)(t)) + (Le)(t)^3/12] = 0.233$ in.^4 (eq. 4-18)

$lr = 0.08374$ in.^4
 $Ar = 0.328$ sq.in.
 $Zr = 0.9385$ in.
 $t = 0.127$ in.
 $Rr = 25.3767$ in.

$Le = 1.1(Dt)^{0.5} = 2.741$ in.	if $Mx > 1.56$
$Le = Lr = 30.000$ in.	if $Mx < 1.56$
$Mx = 17.03$	\longrightarrow $Le = 2.741$ in.

$N = 4$ use N that gives PeG min. $2 < N$

$PeG = 1036.55$ psi (eq. 4-17)

b. Imperfection Factors

$aoG = 0.8$

$FreG$ or $FheG = (aoG)(PeG)(Ro)(KoG)/t = 85.111$ psi (eq. 4-16)

c. Inelastic Buckling Stresses

$FrcG = n (FreG)$ (eq. 4-19)

$FhcG = n (FheG)$ (eq. 4-20)

$n = 1.0$	if $d <= 0.55$	$d = FreG/Fy = 3.4044$	(eq. 5-3)
$n = 0.45/d + 0.18 = 0.312$	if $0.55 < d <= 1.6$	$n = 0.267$	
$n = 1.31/(1 + 1.15d) = 0.267$	if $1.6 < d < 6.25$		
$n = 1/d = 0.294$	if $d >= 6.25$		

$FrcG = n (FreG) = 22.654$ psi (eq. 4-10)

d. Failure Pressures

$poG = (n)(aoG)(PeG) = 221.914$ psi (eq. 4-12)

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum, No Seismic Support Misalignment of 1.000" during Adjustment	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD Rim	MADE BY	CHKD BY	SHT 4 OF 6
	DATE 3/16/94	DATE 12/22/94	DATE	DATE	7.56

SECTION 6 PREDICTED SHELL BUCKLING STRESSES FOR COMBINED LOADS

Section 6.3 Axial Compression, Bending and Hoop Compression

$(Ra)^2 - c (Ra)(Rh) + (Rh)^2 = 1.0$ (eq. 6-3)

$Ra = F_{icj} / F_{xcj}$ $Rh = F_{ocj} / F_{rcj}$

a. Unstiffened and Ring Stiffened Cylinders (all buckling modes)

$c = (F_{xcj} + F_{rcj}) / F_y - 1.0$ (eq. 6-4)

Local Buckling

$c = [F_{xcL} + F_{rcL}] / F_y - 1 = -0.07693$

Let $F_{icL} = (F_{ocL}) (k) (K_{xL} / K_{oL}) = 2.0779 (F_{ocL})$

$Ra = F_{icL} / F_{xcL} = 0.000128 (F_{ocL})$

$K_{xL} = 1.0$

$Rh = F_{ocL} / F_{rcL} = 0.000147 (F_{ocL})$

$K_{oL} = 1.0$ if $M_x \geq 3.42$
 $K_{oL} = 1 - e^*y = 1.000$ if $M_x < 3.42$ (eq. 11-7)
 $M_x = 17.03 \implies K_{oL} = 1.000$

$e = (1 - 0.3k) / [1 + (L_e) / A]$
 $A = (A_r) (R / R_r)^2 = 0.304196 \text{ sq.in.}$
 $A_r = 0.328 \text{ sq.in.}$
 $R_r = 25.377 \text{ in.}$
 $L_e = [1.56(R_r)^{1.5}] + t_w = 2.748297 \text{ in.}$
 $k = N_x / N_o = 2.078$
 $e = 0.175$

Solve eq. 6-3 for F_{ocL}

$F_{ocL} = 5.047 \text{ psi}$ (eq. 6-3)

$F_{icL} = (F_{ocL}) (k) (K_{xL} / K_{oL}) = 10.488 \text{ psi}$

General Instability

$c = [F_{xcG} + F_{rcG}] / F_y - 1 = 0.709645$

$K_{xG} = 1.0$

$K_{oG} = (1 - 0.3k)(L_e) / t_w [(A_r) + (L_e) / t_w] = 0.1942$ (eq. 11-9)
 (If $K_{oG} < 0$ then ring is in tension and the interaction equation is not applicable. Then $F_{icG} = F_{xcG}$)

$k = N_x / N_o = 2.078$
 $A_r = 0.328 \text{ sq.in.}$
 $L_e = [1.56(R_r)^{1.5}] + t_w = 2.748297 \text{ in.}$

Let $F_{icG} = (F_{ocG}) (k) (K_{xG} / K_{oG}) = 10.70148 (F_{ocG})$

$Ra = F_{icG} / F_{xcG} = 0.000534 (F_{ocG})$

$Rh = F_{ocG} / F_{rcG} = 4.41E-05 (F_{ocG})$

Solve eq. 6-3 for F_{ocG}

$F_{ocG} = 1.925 \text{ psi}$ (eq. 6-3)

$F_{icG} = (F_{ocG}) (k) (K_{xG} / K_{oG}) = 20.598 \text{ psi}$

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum, No Seismic Support Misalignment of 1.000" during Adjustment	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RW	MADE BY	CHKD BY	SHT 5 OF 6
	DATE 3/16/94	DATE 19 MAR 1994	DATE	DATE	7.57

SECTION 9 ALLOWABLE STRESSES

Section 9.1 Allowable Stresses For Shell Buckling Mode

Section 9.1.5 Axial Compression and Hoop Compression and Axial Compression, Bending and Hoop Compression

$F_a = F_b = F_{cL} / FS$
(eq. 9-7)

$F_o = F_{ocL} / FS$

Factor of Safety (FS) = 1.77

$F_a = F_b = 5,925 \text{ psi} = 752.50 \text{ #/in.}$	$N_x = 748.41 \text{ #/in.}$
$F_o = 2,862 \text{ psi} = 362.15 \text{ #/in.}$	$N_o = 360.18 \text{ #/in.}$

SECTION 11 STRESS CALCULATIONS

Section 11.3 Hoop Stresses

Section 11.3(b)2 Stress in a Ring Stiffener

Ring Stiffener Stress = $(p \cdot R_o / t) \cdot (1.00 - 0.30 \cdot (N_x / N_o)) \cdot (L_e \cdot t) / (A_r + L_e \cdot t)$ (Eq 11-8)

Solving Eq 11-8,

⇒ Ring Stiffener Stress = 1,214 psi

SUBJECT LIGO Beam Tube -- 49.00" OD x 0.127" Wall 1.75" x 0.1875" Vacuum Stiffener, 30" Spacing 100F Operating + Vacuum, No Seismic Support Misalignment of 1.000" during Adjustment	OFFICE: NOE		REVISION		REFERENCE NO. 930212
	MADE BY WJC	CHKD RSW	MADE BY	CHKD BY	SHT 6 OF 6
	DATE 3/16/94	DATE 12 MAR 1994	DATE	DATE	
					7.58

TUBE OVALING FREQUENCY

THE FREQUENCY OF ANY IN-PLANE MODE OF VIBRATION OF A CIRCULAR RING.

$$f_n = \frac{1}{2\pi} \sqrt{\frac{Eg}{\gamma} \frac{I}{Ar^4} \frac{n^2(1-n^2)^2}{1+n^2}}$$

REF: "VIBRATION PROBLEMS IN ENGINEERING", TIMOSHENKO, 2ND EDITION
Pg. 410, EQ. 202

E = MODULUS OF ELASTICITY = 28,300,000 PSI @ 70°F
(STAINLESS STEEL) = 27,000,000 PSI @ 300°F

γ = UNIT WEIGHT OF MATERIAL = 500 PCF = 0.2894 #/IN³

g = ACCELERATION DUE TO GRAVITY = 386 1/IN/SEC²

I = RING MOMENT OF INERTIA (IN⁴)

A = AREA OF RING (IN²)

r = RADIUS OF RING (IN)

n = NUMBER OF WAVES AROUND CIRCUMFERENCE

f = FREQUENCY

SUBJECT TUBE OVALING FREQUENCY LIGO BEAM TUBE	OFFICE GEB NoE C		REVISION		REFERENCE NO. 97021V
	MADE BY MRS	CHKD BY WJC	MADE BY	CHKD BY	SHT 1 OF 18
	DATE 2/17/94	DATE 4-5-94	DATE	DATE	8.1

FOR UNSTIFFENED CYLINDER

THE FUNDAMENTAL MODE OF VIBRATION $n = 2$

$I = \frac{t^3}{12}$ $A = t$ $r = 24.4385$

$$f = \frac{1}{2\pi} \sqrt{\frac{(28,300,000)(386)}{0.2894} \cdot \frac{t^3/12}{t r^4} \cdot \frac{n^2(1-n^2)^2}{1+n^2}}$$

$$f = 8,926.21 \frac{t}{r^2} \sqrt{\frac{n^2(1-n^2)^2}{1+n^2}}$$

$t = 0.127''$ $r = 24.4385''$ $n = 2$

$f = 23,952 \frac{t}{r^2}$

$f = 5.1 \text{ CYCLES/SEC}$

PERIOD (T) = 0.196 SEC.

SUBJECT TUBE DVALING FREQUENCY W/GO BEAM TUBE	OFFICE CBI NOEC		REVISION		REFERENCE NO. 93021
	MADE BY MRS	CHKD BY WJC	MADE BY	CHKD BY	SHT 2 OF 18
	DATE 2/17/94	DATE 4.5.94	DATE	DATE	0.2

TUBE OVALING FREQUENCY

The frequency of any in-plane mode of vibration of a circular ring can be found using the following formula.

$$f_i = 1/[2(\pi)] \times \{ [Eg/d][I/Ar^4][n^2(1-n^2)^2]/[1+n^2] \}$$

Ref: "Vibration Problems in Engineering", Timoshenko, 2nd Edition

- E = modulus of elasticity = 28300 ksi
- d = unit weight of material = 0.2894 lbs/cu.in.
- g = acceleration due to gravity = 386 in/sec²
- I = ring moment of inertia = 0.0002 in⁴
- A = area of ring = 0.127 sq.in.
- r = ring radius = 24.439 in.
- n = number of waves around circumference
- f_i = frequency (Hz)

n	f _i (Hz)	T (sec.)
2	5.0932	0.1963
3	14.406	0.0694
4	27.622	0.0362
5	44.67	0.0224
6	65.53	0.0153
7	90.194	0.0111
8	118.66	0.0084
9	150.92	0.0066
10	186.98	0.0053

SUBJECT TUBE OVALING FREQUENCY LIGO BEAM TUBE	OFFICE: NOE		REVISION:		REFERENCE NO. 930212
	MADE MRS	CHKD WJC	MADE	CHKD	SHT 3 OF 18
	DATE 2/17/94	DATE 4-5-94	DATE	DATE	8.3

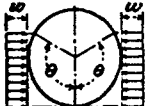
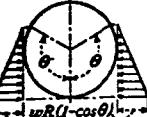
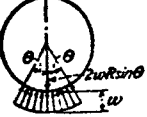


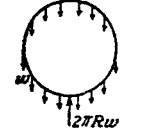
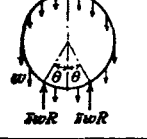
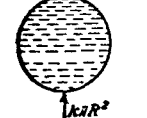
<p>13.</p> 	$M_1 = wR^2(0.3183\{s - \theta c + \frac{1}{2}sc^2 + \frac{1}{2}c^3 - \frac{1}{2}sc - \frac{1}{2}c^2 - \frac{1}{2}c - \frac{1}{2}c^2\}) \quad T_1 = wR(0.3183\{s + \frac{1}{2}sc^2 - \theta c\} + c - 1)$ $(x = 0 \text{ to } z = \theta) \quad M = M_1 - T_1R(1 - u) - \frac{1}{2}wR^2 \quad (x = \theta \text{ to } z = \pi) \quad M = M_1 - T_1R(1 - u) - \frac{1}{2}wR^2(1 - c)(1 + c - 2u)$ $T = T_1u + wR(1 - u)u \quad T = T_1u + wR(1 - c)u$ $V = -T_1s - wR(1 - u)s \quad V = -T_1s - wR(1 - c)s$
<p>14.</p> 	$M_1 = wR^2(0.3183\{s + \frac{1}{2}sc - \frac{1}{2}sc^2 + \frac{1}{2}sc^2 - \frac{1}{2}sc^2 - \frac{1}{2}sc^2 - \frac{1}{2}sc^2 - \frac{1}{2}sc^2 - \frac{1}{2}(1 - c)^2\}) \quad T_1 = wR(0.3183\{s + \frac{1}{2}sc^2 - \frac{1}{2}sc - \frac{1}{2}sc^2 - \frac{1}{2}(1 - c)^2\})$ $(x = 0 \text{ to } z = \theta) \quad M = M_1 - T_1R(1 - u) + wR^2(1 - u^2) - \frac{1}{2}wR^2(1 - c)(1 - u)^2 \quad (x = \theta \text{ to } z = \pi) \quad M = M_1 - T_1R(1 - u) - \frac{1}{2}wR^2(1 - c)^2(1 + \frac{1}{2}c - u)$ $T = T_1u + wR^2(1 - 2c + u)(1 - u) \quad T = T_1u + wR^2(1 - c)^2(1 - c)(1 + \frac{1}{2}c - u)$ $V = -T_1s - wR^2(1 - 2c + u)(1 - u)s \quad V = -T_1s - wR^2(1 - c)^2(1 - c)s$
<p>15.</p> 	$M_1 = wR^2(c - 0.3183(\theta c - \theta) - 1) \quad T_1 = wR(0.3183(s - \theta c) + c - 1)$ $(x = 0 \text{ to } z = \theta) \quad M = M_1 - T_1R(1 - u) - wR^2(1 - u - \pi s) \quad (x = \theta \text{ to } z = \pi) \quad M = M_1 - T_1R(1 - u) - wR^2(\pi u - u)$ $T = T_1u + wR(\pi s + u - 1) \quad T = T_1u + wR(\pi u - \pi)$ $V = -T_1s + wR(\pi u - s) \quad V = -T_1s + wR(\pi s - s)$ $D_x = \frac{2wR^2}{EI} \left[\frac{1}{4}\theta c + 0.3183\theta - 0.5683s \right] \quad w \leq \theta < \frac{\pi}{2}$ $D_y = \frac{2wR^2}{EI} \left[\frac{1}{4}\theta s + 0.3183\theta + \frac{1}{2}c - 0.3183s - \frac{1}{2} \right]$
<p>16.</p> 	$(x = 0 \text{ to } z = \theta) \quad M = WR(0.15915\theta^2 + c - \pi c - c - \frac{1}{2}\pi^2 - \pi^2) - \pi(ac + \theta + \pi s + \phi) - \pi(s + \pi) - \frac{1}{2}(s - \pi) + z]$ $(x = \theta \text{ to } z = 2\pi - \phi) \quad M = WR(0.15915\theta^2 - c - \pi c - c - \frac{1}{2}\pi^2 - \pi^2) - \pi(ac + \theta + \pi s + \phi) - \pi(s + \pi) - \frac{1}{2}(s - \pi)$ $(x = 2\pi - \phi \text{ to } 2\pi) \quad M = WR(0.15915\theta^2 + c - \pi c - c - \frac{1}{2}\pi^2 - \pi^2) - \pi(ac + \theta + \pi s + \phi) - \pi(s + \pi) - \frac{1}{2}(s - \pi) - z]$ $(x = 0 \text{ to } z = \theta \text{ and } z = 2\pi - \phi \text{ to } z = 2\pi) \quad T = W(0.15915\theta^2 - \pi^2 - \pi^2 - \pi c - \pi s - \pi s - \pi s + \pi) + z]$ $(x = \theta \text{ to } z = 2\pi - \phi) \quad T = W(0.15915(\theta^2 - \pi^2 - \pi^2 - \pi c - \pi s - \pi s - \pi s - \pi s) + z]$ $V = W(0.15915(-s - \pi - \pi - \pi s^2 + \pi^2 - \pi c - \pi s - \pi s - \pi s) + u)$ $V = W(0.15915(-s - \pi - \pi - \pi s^2 + \pi^2 - \pi c - \pi s - \pi s - \pi s) + u)$
<p>17. Ring under localized couple M_0 and uniform tangential shear of $(M_0 + 2\pi R^2)$ lb. per linear in.</p> 	$(x = 0 \text{ to } z = \theta) \quad M = M_0(0.3183(\pi s - \pi c + \frac{1}{2}\pi - \frac{1}{2}\pi) - \frac{1}{2}) \quad (x = \theta \text{ to } z = 2\pi) \quad M = M_0(0.3183(\pi s - \pi c - \frac{1}{2}\pi - \frac{1}{2}\pi) + \frac{1}{2})$ $T = -\frac{M_0}{R}(0.318(\pi c - \pi s)) \quad T = -\frac{M_0}{R}(0.3183(\pi c - \pi s))$ $V = -\frac{M_0}{R} \left[0.3183 \left(\pi s + \frac{1}{2} \right) \right] \quad V = -\frac{M_0}{R} \left[0.3183 \left(\pi s + \frac{1}{2} \right) \right]$

TABLE VIII.—FORMULAS FOR CIRCULAR RINGS AND ARCHES.—(Continued)

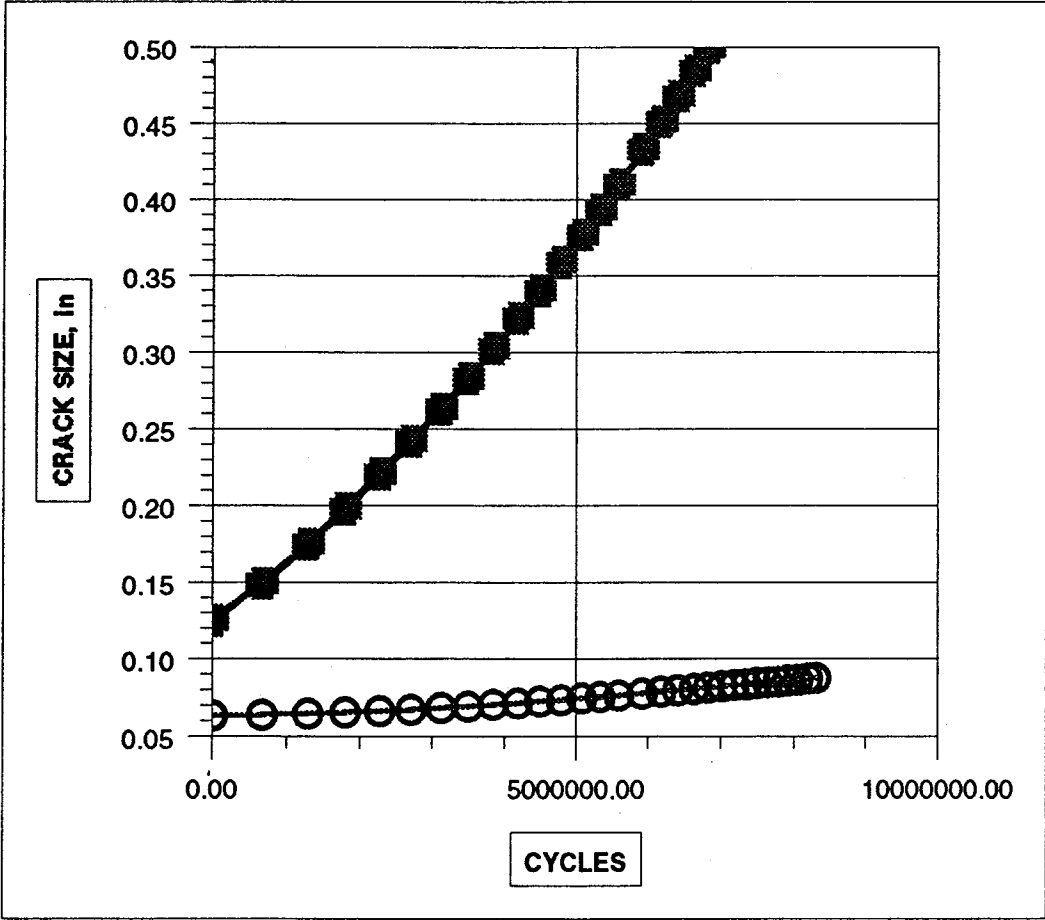
Loading, support and case number	Formulas for bending moment M , circumferential tension T , radial shear V at angular distance z from bottom of ring and for D_x , change in horizontal diameter, and D_y , change in vertical diameter
<p>18. Ring supported at base and loaded by own weight of w lb. per linear in.</p> 	$M = wR^2(1 + \frac{1}{2}u - \pi z + \pi s) \quad \text{Max } +M = M_1 = \frac{1}{2}wR^2 \quad \text{Max } -M = -0.642wR^2 \text{ at } z = 1.3 \text{ rad } (74.6^\circ)$ $T = wR(\pi z - \frac{1}{2}u - \pi s) \quad T = wR(\pi z + \frac{1}{2}z - \pi u)$ $V = wR(\pi z + \frac{1}{2}z - \pi u)$ $D_x = \frac{wR^2}{EI}(0.4292)$ $D_y = -\frac{wR^2}{EI}(0.4674)$ $\Delta R = -\frac{wR^2}{EI}(0.18765)$
<p>19. Ring symmetrically supported and loaded by own weight of w lb. per linear in.</p> 	$M_1 = wR^2(\frac{1}{2} + c + \theta s - \pi s + s^2) \quad T_1 = wR(s^2 - \frac{1}{2})$ $(x = 0 \text{ to } z = \theta) \quad M = M_1 - T_1R(1 - u) + wR^2(\pi z + u - 1) \quad (x = \theta \text{ to } z = \pi) \quad M = M_1 - T_1R(1 - u) - wR^2(\pi z + u - 1 - \pi z + \pi s)$ $T = T_1u + wR\pi z \quad T = T_1u + wR(\pi z - \pi s)$ $V = -T_1s + wR\pi z \quad V = -T_1s + wR(\pi z - \pi s)$ $D_x = \frac{2wR^2}{EI} \left[c + \theta s - \frac{1}{2}\pi(1 + s^2) \right]$ $D_y = \frac{wR^2}{EI} \left[-2.4674 + \frac{1}{2}\pi(\pi c + \theta - 2s) + 2(\theta s + c) \right]$ $\Delta R = \frac{wR^2}{EI} \left[0.38315 - 0.5708(c + \theta s) + \frac{1}{2}s^2 \right]$
<p>20. 1-in. segment of pipe filled with liquid of specific weight k lb. per cu. in. and supported at base</p> 	$M_1 = \frac{1}{2}kR^2 \quad T_1 = \frac{1}{2}kR^2 \quad \text{Max } +M = M_1 \quad \text{Max } -M = -0.321kR^2 \text{ at } z = 1.3 \text{ rad } (74.6^\circ)$ $M = kR^2(\frac{1}{2} + \frac{1}{2}u - \frac{1}{2}\pi z + \frac{1}{2}\pi s)$ $T = kR^2(\frac{1}{2} + \frac{1}{2}u - \frac{1}{2}\pi z + \frac{1}{2}\pi s)$ $V = kR^2(\frac{1}{2}\pi z + \frac{1}{2}z - \frac{1}{2}\pi u)$ $D_x = \frac{kR^2}{EI}(0.2146)$ $D_y = -\frac{kR^2}{EI}(0.2337)$ $\Delta R = -\frac{kR^2}{EI}(0.093825)$

	A	B	C	D	E	F
1	... FRACTURE MECHANISMS					
2						
3						
4						
5						
6						
7						
8						
9	da/dN = C*deltaK^m					
10	C	3.00E-10				
11	m	3.25				
12	DIMENSIONS					
13	Flaw type	Surface				
14	Initial flaw depth (a)	0.06 in	Dist. to free edge (W)	100.00 in		
15	Initial flaw length (2c)	0.13 in				
16	Wall thickness	0.13 in				
17						
18	Temperature	0.00 Deg. F				
19						
20	STRESSES					
21	Prim. memb. stress (Pm)	0.00 ksi	Sec. memb. stress (Sm)	25.00 ksi**		
22	Prim. bend stress (Pb)	12.25 ksi	Sec. bend stress (Sb)	0.00 ksi**		
23	Cyclic memb. stress	0.00 ksi				
24	Cyclic bend stress	12.25 ksi				
25	MATERIAL PROP.					
26	Parent metal yield str.	25.00 ksi*	Fracture toughness (Kc)	120.00 ksiVin		
27	Parent metal tensile str.	70.00 ksi*				
28	Weld metal yield str.	25.00 ksi*				
29	Weld metal tensile str.	70.00 ksi*				
30	*Specified minimum values		**These values may be reduced by			
31			mechanical stress relief			
32						
33						
34						
35						
36						
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	A	B	C	D	E	F
42		[REDACTED]				
43		[REDACTED]				
44		[REDACTED]				
45		[REDACTED]				
46						
47	Cycles	Length (2c)		Crack Depth, inches	Type	
48	0.000E+00	0.125 in		0.064	Surface	
49	6.633E+05	0.149 in		0.064	Surface	
50	1.289E+06	0.175 in		0.065	Surface	
51	1.798E+06	0.198 in		0.065	Surface	
52	2.279E+06	0.221 in		0.066	Surface	
53	2.708E+06	0.242 in		0.067	Surface	
54	3.119E+06	0.263 in		0.068	Surface	
55	3.487E+06	0.283 in		0.069	Surface	
56	3.843E+06	0.302 in		0.070	Surface	
57	4.187E+06	0.322 in		0.071	Surface	
58	4.495E+06	0.340 in		0.072	Surface	
59	4.793E+06	0.359 in		0.073	Surface	
60	5.082E+06	0.377 in		0.075	Surface	
61	5.340E+06	0.394 in		0.075	Surface	
62	5.590E+06	0.410 in		0.076	Surface	
63	5.920E+06	0.433 in		0.078	Surface	
64	6.176E+06	0.451 in		0.079	Surface	
65	6.404E+06	0.468 in		0.080	Surface	
66	6.626E+06	0.485 in		0.081	Surface	
67	6.823E+06	0.500 in		0.081	Surface	
68	7.016E+06	0.515 in		0.082	Surface	
69	7.185E+06	0.529 in		0.083	Surface	
70	7.352E+06	0.542 in		0.084	Surface	
71	7.497E+06	0.554 in		0.084	Surface	
72	7.639E+06	0.566 in		0.085	Surface	
73	7.779E+06	0.579 in		0.085	Surface	
74	7.917E+06	0.591 in		0.086	Surface	
75	8.053E+06	0.603 in		0.086	Surface	
76	8.186E+06	0.615 in		0.087	Surface	
77	8.301E+06	0.626 in		0.088	Surface	

	A	B	C	D	E	F
68	8414161.20	0.64 in			0.09 Surface	
79	8525895.36	0.65 in			0.09 Surface	
80	8636112.87	0.66 in			0.09 Surface	
81	8744851.02	0.67 in			0.09 Surface	
82	8852145.66	0.68 in			0.09 Surface	
83	8958031.29	0.69 in			0.09 Surface	
84	9062541.04	0.70 in			0.09 Surface	
85	9151049.94	0.71 in			0.09 Surface	
86	9238590.77	0.72 in			0.09 Surface	
87	9325182.22	0.73 in			0.09 Surface	
88	9410842.35	0.74 in			0.09 Surface	
89	9495588.61	0.75 in			0.09 Surface	
90	9579437.87	0.75 in			0.09 Surface	
91	9662406.41	0.76 in			0.09 Surface	
92	9744509.98	0.77 in			0.09 Surface	
93	9825763.79	0.78 in			0.09 Surface	
94	9906182.53	0.79 in			0.09 Surface	
95	9985780.40	0.80 in			0.09 Surface	
96	10064571.13	0.81 in			0.10 Surface	
97	10129623.08	0.82 in			0.10 Surface	
98	10194131.17	0.82 in			0.10 Surface	
99	10258102.62	0.83 in			0.10 Surface	
100	10321544.46	0.84 in			0.10 Surface	
101	10384463.52	0.85 in			0.10 Surface	
102	10407478.28	0.85 in			0.10 Surface	
103						
104						
105						
106						
107	Estimated life = 10407478.28 cycles					
108						
109	Limiting crack length = 0.85 in					
110						
111	Limiting crack depth = 0.097 in					
112						
113						
114						
115						
116						
117						
118						

	A	B	C	D	E	F
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139						
140						
141						
142						
143						



TOTAL STRESS = 24,989 PSI \approx F_y

o o VERTICAL PIPE STU MUST BE USED AT EACH TUBE SUPPORT

FRACTURE MECHANICS METHODS WERE USED TO EVALUATE BEAM TUBE FOR OVALING ASSUMING PEAK STRESSES VARY FROM 12,250 PSI TO 24,500 PSI.

DL TO DL + 1/2 LOAD ON TUBE

ASSUMED FLAW SIZE $L = 1/2 t$ $D = 1/4 t$

ESTIMATED LIFE = 10,000,000 CYCLES

TUBE SHIPPING DISTANCE = 3000 MILES

AUG. SHIPPING SPEED \approx 40 MPH

TRAVEL TIME = $\frac{3000 \text{ MILES}}{40 \text{ MPH}} = 75 \text{ HRS} = 270,000 \text{ SEC.}$

<u>N</u>	<u>FREQ.</u>	<u>CYCLES</u>
2	5.09	1,374,300
3	14.41	3,890,700
4	27.62	7,457,400
5	44.67	14,060,900

THE FUNDAMENTAL MODE OF VIBRATION FOR UNSTIFFENED CYLINDER IS $N=2$. FOR $N=2$ CYCLES = 1,374,000 $<$ 10,000,000
o o OK

SUBJECT	OFFICE CBI NOE C		REVISION		REFERENCE NO. 930212
	MADE BY mbs	CHKD BY WJC	MADE BY	CHKD BY	SHT 10 OF 18
	DATE 2/17/94	DATE 4-6-94	DATE	DATE	8.10

BEAM BENDING MODE

REF: TIMOSHENKO

$$f_n = \frac{\pi n^2}{2L^2} \sqrt{\frac{EI g}{A \gamma}}$$

$$g = 386 \text{ #/SEC}^2$$

$$E = 28,300,000 \text{ PSI}$$

$$\gamma = 0.2894 \text{ #/IN}^3$$

$$A = 2\pi r t = 2\pi (24.4385)(.127) = 19.5 \text{ IN}^2$$

$$I = \pi r^3 t = \pi (24.4385)^3 (.127) = 5823.4 \text{ IN}^4$$

$$L = 65' = 780''$$

LET $n=1$

$$f = \frac{\pi(1)^2}{2(780)^2} \sqrt{\frac{(28,300,000)(5823.4)(386)}{(19.5)(0.2894)}}$$

$$f = 8.67 \text{ CYCLES/SEC}$$

$$T = 0.115 \text{ SEC.}$$

SUBJECT	OFFICE CBI NOE C		REVISION	REFERENCE NO. 930212
	MADE BY MRS	CHKD BY WJC	MADE BY	CHKD BY
	DATE 2/17/94	DATE 4-5-94	DATE	DATE
				SHT 11 OF 18
				8.11

REF ASME SECTION VIII, DIV. 2

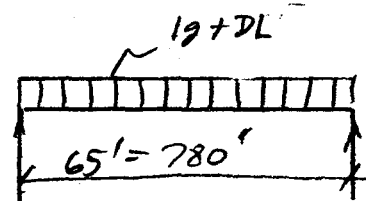
APP. 5, FIG. 5-110.2.2
TABLE 5-10.2

ENDURANCE LIMIT $\sigma = 13,600 \text{ PSI}$

$$w = [2.9 \pi r t \gamma]^2$$

$$w = [2.9 (24.4385) (.127) (0.2894)]^2$$

$$w = 11.29 \text{ #/IN}$$



$$M = \frac{w \cdot l^2}{8} = \frac{(11.29)(780)^2}{8} = 859,000 \text{ IN-LBS}$$

$$S = \frac{M}{\sigma}$$

$$S = \pi r^2 t = \pi (24.4385)^2 (.127) = 238.3$$

$$\sigma = \frac{859,000}{238.3} = 3,600 \text{ PSI} < 13,600 \text{ PSI} \quad \text{NO FATIGUE PROBLEM}$$

SUBJECT	OFFICE CBI NOE C		REVISION		REFERENCE NO. 930212
	MADE BY MRS	CHKD BY WJC	MADE BY	CHKD BY	SHT 12 OF 18
	DATE 2/17/94	DATE 4-6-94	DATE	DATE	8.12

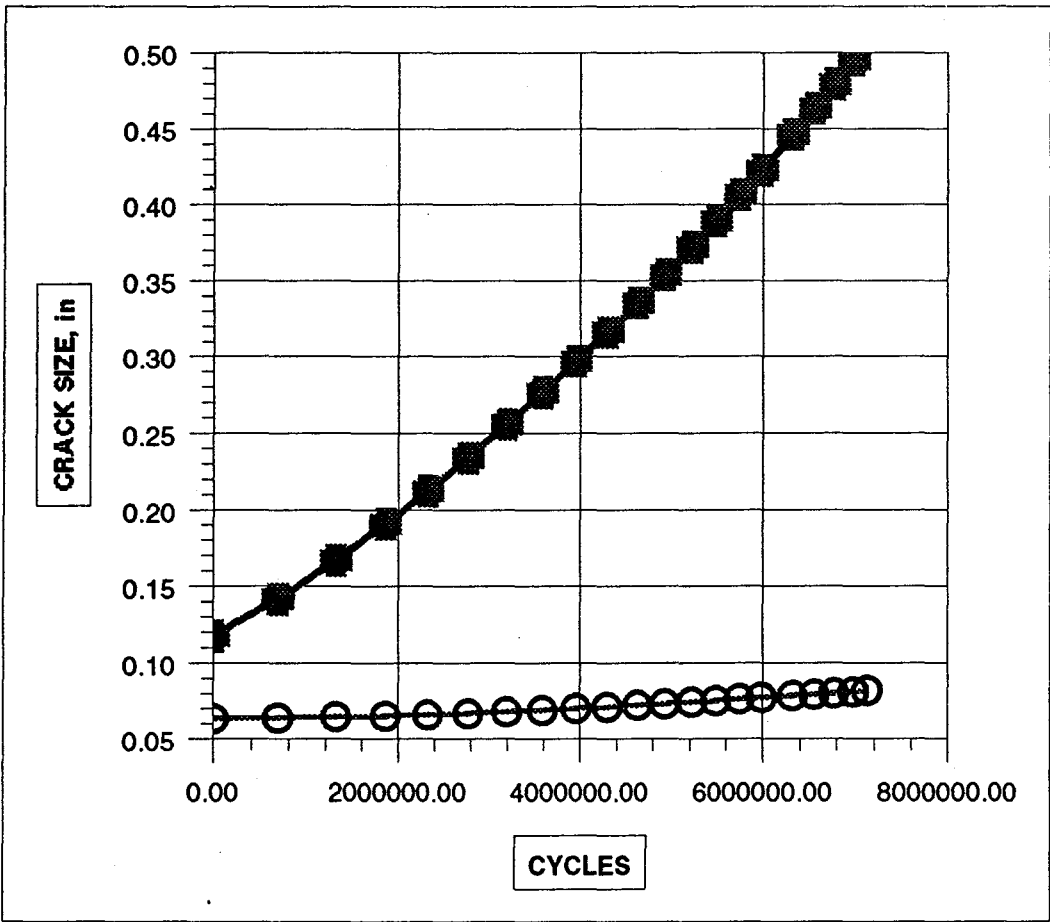
	A	B	C	D	E	F
1	CYCLE LIFE FOR BEAM BENDING MODE DURING SHIPPING OF TUBES					
2	0.125" LONG X 0.064" DEPTH CRACK					
3	CYCLE LIFE					
4						
5	Previous		Calculate		Next	
6	Previous		Calculate		Next	
7	Previous		Calculate		Next	
8						
9	da/dN = C*deltaK^m					
10	C	3.00E-10				
11	m	3.25				
12	DIMENSIONS					
13	Flaw type	Surface				
14	Initial flaw depth (a)	0.06 in		Dist. to free edge (W)	100.00 in	
15	Initial flaw length (2c)	0.13 in				
16	Wall thickness	0.13 in				
17						
18	Temperature	0.00 Deg. F				
19						
20	STRESSES					
21	Prim. memb. stress (Pm)	1.80 ksi		Sec. memb. stress (Sm)	25.00 ksi**	
22	Prim. bend stress (Pb)	0.00 ksi		Sec. bend stress (Sb)	0.00 ksi**	
23	Cyclic memb. stress	1.80 ksi				
24	Cyclic bend stress	0.00 ksi				
25	MATERIAL PROP.					
26	Parent metal yield str.	25.00 ksi*		Fracture toughness (Kc)	120.00 ksiVin	
27	Parent metal tensile str.	70.00 ksi*				
28	Weld metal yield str.	25.00 ksi*				
29	Weld metal tensile str.	70.00 ksi*				
30	*Specified minimum values			**These values may be reduced by		
31				mechanical stress relief		
32						
33						
34						
35						
36						
37						
38						
39						
40						
41						

	A	B	C	D	E	F
42		TIME-DEPENDENT CRACK GROWTH				
43						
44						
45						
46						
47	Cycles	Length (2c)		Crack Depth, inches	Type	
48	0.000E+00	0.125 in		0.064	Surface	
49	2.826E+08	0.182 in		0.080	Surface	
50	4.283E+08	0.240 in		0.097	Surface	
51	5.077E+08	0.297 in		0.119	Surface	
52	6.520E+08	0.584 in		0.127	Through-wall	
53	7.108E+08	0.871 in		0.127	Through-wall	
54	7.497E+08	1.215 in		0.127	Through-wall	
55	7.739E+08	1.559 in		0.127	Through-wall	
56	7.931E+08	1.960 in		0.127	Through-wall	
57	8.084E+08	2.419 in		0.127	Through-wall	
58	8.208E+08	2.935 in		0.127	Through-wall	
59	8.301E+08	3.452 in		0.127	Through-wall	
60	8.381E+08	4.025 in		0.127	Through-wall	
61	8.450E+08	4.656 in		0.127	Through-wall	
62	8.510E+08	5.344 in		0.127	Through-wall	
63	8.558E+08	6.033 in		0.127	Through-wall	
64	8.602E+08	6.778 in		0.127	Through-wall	
65	8.640E+08	7.581 in		0.127	Through-wall	
66	8.673E+08	8.384 in		0.127	Through-wall	
67	8.702E+08	9.245 in		0.127	Through-wall	
68	8.729E+08	10.162 in		0.127	Through-wall	
69	8.735E+08	10.375 in		0.127	Through-wall	
70						
71						
72						
73						
74						
75						
76						
77						

	A	B	C	D	E	F
1	... FRACTURE MECHANISMS/DEFLECTION					
2						
3						
4						
5						
6	Fracture		Calculation		Heat	
7						
8						
9	da/dN = C*deltaK^m					
10	C	3.00E-10				
11	m	3.25				
12	DIMENSIONS					
13	Flaw type	Surface				
14	Initial flaw depth (a)	0.06 in		Dist. to free edge (W)	100.00 in	
15	Initial flaw length (2c)	0.12 in				
16	Wall thickness	0.13 in				
17						
18	Temperature	0.00 Deg. F				
19						
20	STRESSES					
21	Prim. memb. stress (Pm)	0.00 ksi		Sec. memb. stress (Sm)	25.00 ksi**	
22	Prim. bend stress (Pb)	12.25 ksi		Sec. bend stress (Sb)	0.00 ksi**	
23	Cyclic memb. stress	0.00 ksi				
24	Cyclic bend stress	12.25 ksi				
25	MATERIAL PROP.					
26	Parent metal yield str.	25.00 ksi*		Fracture toughness (Kc)	120.00 ksiVin	
27	Parent metal tensile str.	70.00 ksi*				
28	Weld metal yield str.	25.00 ksi*				
29	Weld metal tensile str.	25.00 ksi*				
30	*Specified minimum values			**These values may be reduced by		
31				mechanical stress relief		
32						
33	$\sqrt{2}/\sqrt{1} = 0.144$					
34	$\beta = 30^\circ$					
35	PARALLEL TO WELD					
36						
37						
38						
39						
40						
41						

	A	B	C	D	E	F
42		TIME-DEPENDENT CRACK GROWTH				
43						
44						
45						
46						
47	Cycles	Length (2c)		Crack Depth, inches	Type	
48	0.000E+00	0.118 in		0.064	Surface	
49	6.832E+05	0.142 in		0.064	Surface	
50	1.312E+06	0.167 in		0.064	Surface	
51	1.860E+06	0.191 in		0.065	Surface	
52	2.326E+06	0.212 in		0.066	Surface	
53	2.770E+06	0.234 in		0.067	Surface	
54	3.195E+06	0.256 in		0.068	Surface	
55	3.583E+06	0.276 in		0.069	Surface	
56	3.957E+06	0.297 in		0.070	Surface	
57	4.299E+06	0.316 in		0.071	Surface	
58	4.629E+06	0.336 in		0.072	Surface	
59	4.930E+06	0.354 in		0.073	Surface	
60	5.222E+06	0.372 in		0.074	Surface	
61	5.488E+06	0.389 in		0.075	Surface	
62	5.747E+06	0.406 in		0.076	Surface	
63	5.981E+06	0.422 in		0.077	Surface	
64	6.322E+06	0.446 in		0.078	Surface	
65	6.557E+06	0.463 in		0.079	Surface	
66	6.772E+06	0.479 in		0.080	Surface	
67	6.981E+06	0.495 in		0.081	Surface	
68	7.124E+06	0.506 in		0.082	Surface	
69						
70						
71						
72						
73						
74						
75						
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77						

	A	B	C	D	E	F
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103					
104					
105					
106					

107	Estimated life = 7124168.76 cycles
108	
109	Limiting crack length = 0.506 in
110	
111	Limiting crack depth = 0.082 in
112	
113	
114	

115					
116					
117					
118					

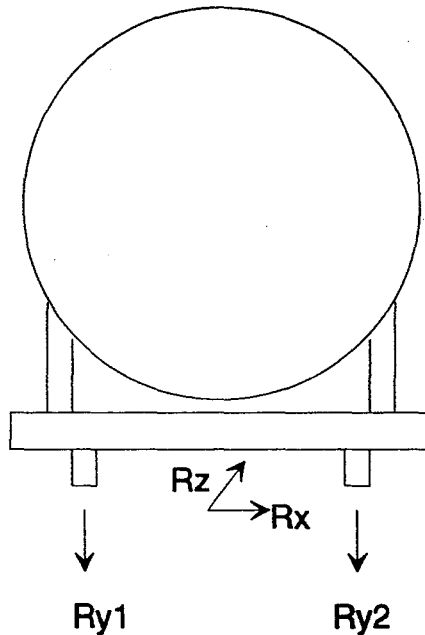
	A	B	C	D	E	F		
78								
79								
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82								
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86								
87								
88								
89								
90								
91								
92								
93								
94								
95								
96								
97								
98								
99								
100								
101								
102								
103								
104								
105								
106								
107	Estimated life = 873496918.15 cycles							
108								
109	Limiting crack length = 10.374 in							
110								
111	Limiting crack depth = 0.127 in (Through-wall)							
112								
113								
114								
115								
116								
117								
118								

	A	B	C	D	E	F
1	... T TREATMENT/REPAIR					
2						
3						
4						
5						
6	Previous		Calculate		Next	
7						
8						
9	da/dN = C*deltaK^m					
10	C	3.00E-10				
11	m	3.25				
12	DIMENSIONS					
13	Flaw type	Surface				
14	Initial flaw depth (a)	0.06 in		Dist. to free edge (W)	100.00 in	
15	Initial flaw length (2c)	0.12 in				
16	Wall thickness	0.13 in				
17						
18	Temperature	0.00 Deg. F				
19						
20	STRESSES					
21	Prim. memb. stress (Pm)	0.00 ksi		Sec. memb. stress (Sm)	25.00 ksi**	
22	Prim. bend stress (Pb)	12.25 ksi		Sec. bend stress (Sb)	0.00 ksi**	
23	Cyclic memb. stress	0.00 ksi				
24	Cyclic bend stress	12.25 ksi				
25	MATERIAL PROP.					
26	Parent metal yield str.	25.00 ksi*		Fracture toughness (Kc)	120.00 ksiVin	
27	Parent metal tensile str.	70.00 ksi*				
28	Weld metal yield str.	25.00 ksi*				
29	Weld metal tensile str.	25.00 ksi*				
30	*Specified minimum values			**These values may be reduced by		
31				mechanical stress relief		
32						
33	$\sqrt{2}/\sqrt{1} = 0.144$					
34	$\beta = 30^\circ$					
35	PARALLEL TO WELD					
36						
37						
38						
39						
40						
41						

	A	B	C	D	E	F
42		TIME-DEPENDENT CRACK GROWTH				
43						
44						
45						
46						
47	Cycles	Length (2c)		Crack Depth, inches	Type	
48	0.000E+00	0.118 in		0.064	Surface	
49	6.832E+05	0.142 in		0.064	Surface	
50	1.312E+06	0.167 in		0.064	Surface	
51	1.860E+06	0.191 in		0.065	Surface	
52	2.326E+06	0.212 in		0.066	Surface	
53	2.770E+06	0.234 in		0.067	Surface	
54	3.195E+06	0.256 in		0.068	Surface	
55	3.583E+06	0.276 in		0.069	Surface	
56	3.957E+06	0.297 in		0.070	Surface	
57	4.299E+06	0.316 in		0.071	Surface	
58	4.629E+06	0.336 in		0.072	Surface	
59	4.930E+06	0.354 in		0.073	Surface	
60	5.222E+06	0.372 in		0.074	Surface	
61	5.488E+06	0.389 in		0.075	Surface	
62	5.747E+06	0.406 in		0.076	Surface	
63	5.981E+06	0.422 in		0.077	Surface	
64	6.322E+06	0.446 in		0.078	Surface	
65	6.557E+06	0.463 in		0.079	Surface	
66	6.772E+06	0.479 in		0.080	Surface	
67	6.981E+06	0.495 in		0.081	Surface	
68	7.124E+06	0.506 in		0.082	Surface	
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	A	B	C	D	E	F
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99						
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101						
102						
103						
104						
105						
106						
107	Estimated life = 7124168.76 cycles					
108	Limiting crack length = 0.506 in					
109	Limiting crack depth = 0.082 in					
110						
111						
112						
113						
114						
115						
116						
117						
118						

FIXED SUPPORT FOUNDATION LOADS



FIXED SUPPORT FOUNDATION LOADS		Lateral		Vertical		Axial
Load Case	Support Weight = 500 lbs	Rx	Ry1	Ry2	Ry total	Rz
1. Operating		85	3870	3648	7518	771
2. Bake Out @ 302 F		656	860	-860	0	5959
3. 0.579" Differential Settlement @ Fixed Support		0	+/- 715	+/- 715	+/- 1430	0
4. 1" Differential Settlement @ Fixed Support		0	+/- 1235	+/- 1235	+/- 2470	0
5. 0.5" Horizontal Misalignment @ Fixed Support		1235	1621	-1621	0	0
6. 0.579" Differential Settlement @ Guided Support		0	+/- 356	+/- 356	+/- 712	0
7. 1" Differential Settlement @ Guided Support		0	+/- 615	+/- 615	+/- 1230	0
8. 0.5" Misalignment @ Guided Support		615	807	-807	0	0
9. Lateral Seismic (x direction)		1287	1689	-1689	0	0
10. Axial Seismic (z direction)		0	0	0	0	2169
11. Wind Load		1275	1673	-1673	0	0

Maximum Reactions - Normal Operating Conditions

Max Rx = 1320 lbs, 1+3+5
 Max Ry1= 6206 lbs, 1+3+5
 Min Ry2= 1312 lbs, 1-3+5
 Max Ry= 9988 lbs, 1+4
 Max Rz = 6730 lbs, 1+2+3

Maximum Reactions - Seismic or Wind Conditions

Max Rx = 2607 lbs, 1+3+5+9
 Max Ry1= 7895 lbs, 1+3+5+9
 Min Ry2= -377 lbs, 1-3+5+9
 Max Ry= 9988 lbs, 1+4+9
 Max Rz = 8899 lbs, 1+2+6+10

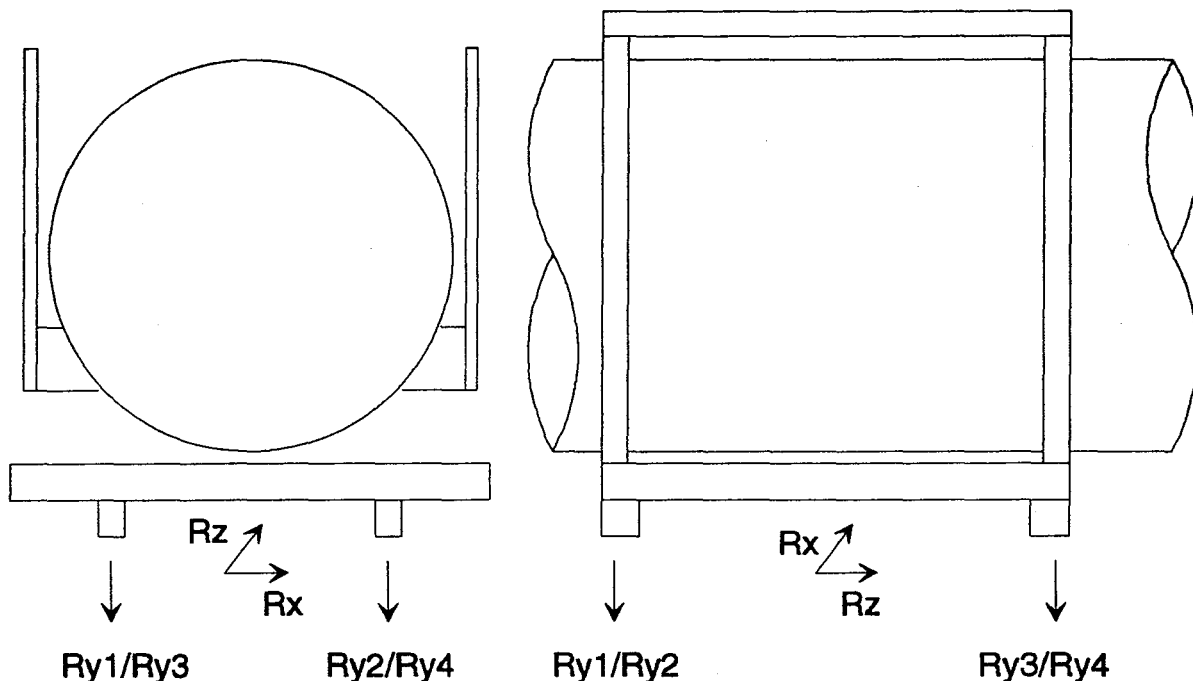
SUBJECT LIGO Beam Tube Foundation Load Summary	OFFICE:	NOE-C	REVISION:	REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY
	DATE 3/22/94	DATE 4-4-94	DATE	DATE
				SHT 1 OF 4 9.1

RESULTS OF LOAD COMBINATIONS

FIXED SUPPORT FOUNDATION LOADS		Lateral	Vertical			Axial
Load Combination	Support Weight = 500 lbs	Rx	Ry1	Ry2	Ry total	Rz
W/O Increased allowable						
1+2+3		740	5446	3502	8948	6730
1+4		85	5105	4883	9988	771
1+3+5		1320	6206	2742	8948	771
1+7		85	4485	4263	8748	771
1+6+8		700	5034	3197	8230	771
1+2-3		740	4016	2072	6088	6730
1-4		85	2635	2413	5048	771
1-3+5		1320	4776	1312	6088	771
1-7		85	3255	3033	6288	771
1-6+8		700	4321	2484	6806	771
MAXIMUM		1320	6206	4883	9988	6730
MINIMUM		85	2635	1312	5048	771
With Increased allowable						
1+2+3+9		2027	7135	1813	8948	6730
1+2+3+10		740	5446	3502	8948	8899
1+2+6+9		2027	6776	1454	8230	6730
1+2+6+10		740	5087	3143	8230	8899
1+4+9		1372	6794	3194	9988	771
1+4+10		85	5105	4883	9988	2940
1+7+9		1372	6174	2574	8748	771
1+7+10		85	4485	4263	8748	2940
1+3+5+9		2607	7895	1053	8948	771
1+3+5+10		1320	6206	2742	8948	2940
1+6+8+9		1987	6723	1507	8230	771
1+6+8+10		700	5034	3197	8230	2940
1+2-3+9		2027	5705	383	6088	6730
1+2-3+10		740	4016	2072	6088	8899
1+2-6+9		2027	6064	742	6806	6730
1+2-6+10		740	4375	2431	6806	8899
1-4+9		1372	4324	724	5048	771
1-4+10		85	2635	2413	5048	2940
1-7+9		1372	4944	1344	6288	771
1-7+10		85	3255	3033	6288	2940
1-3+5+9		2607	6465	-377	6088	771
1-3+5+10		1320	4776	1312	6088	2940
1-6+8+9		1987	6011	795	6806	771
1-6+8+10		700	4321	2484	6806	2940
MAXIMUM		2607	7895	4883	9988	8899
MINIMUM		85	2635	-377	5048	771

SUBJECT LIGO Beam Tube Foundation Load Summary	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 2 OF 4
	DATE 3/22/94	DATE 4-4-94	DATE	DATE	9.2

GUIDED SUPPORT FOUNDATION LOADS



GUIDED SUPPORT FOUNDATION LOADS Load Case	Support Weight = 1600 lbs		Vertical			
	Lateral Rx	Ry1	Ry2	Ry3	Ry4	Ry total
1. Operating	85	1664	1553	1664	1553	6434
2a. Bake Out @ 302 F	656	598	-262	598	-262	672
2b. Bake Out @ 302 F	656	262	-598	262	-598	-672
3. 0.579" Differential Settlement @ Fixed Support	0	+/- 715	+/- 715	-/+ 365	-/+ 365	+/- 701
4. 1" Differential Settlement @ Fixed Support	0	+/- 1235	+/- 1235	-/+ 630	-/+ 630	+/- 1210
5. 0.5" Horizontal Misalignment @ Fixed Support	605	397	-397	397	-397	0
6. 0.579" Differential Settlement @ Guided Support	0	+/- 162	+/- 162	+/- 155	+/- 155	+/- 635
7. 1" Differential Settlement @ Guided Support	0	+/- 280	+/- 280	+/- 269	+/- 269	+/- 1097
8. 0.5" Misalignment @ Guided Support	549	360	-360	360	-360	0
9. Lateral Seismic (x direction)	882	579	-579	579	-579	0
10. Axial Seismic (z direction)	0	0	0	0	0	0
11. Wind Load	874	574	-574	574	-574	0

Maximum Reactions - Normal Operating Conditions

- Max Rx = 740 lbs, 1+2a+3
- Max Ry1= 2977 lbs, 1+2a+3
- Min Ry2= 240 lbs, 1+2b-3
- Max Ry3= 2627 lbs, 1+2a+3
- Min Ry4= 590 lbs, 1+2b-3
- Max Ry= 7807 lbs, 1+2a+3

Maximum Reactions - Seismic or Wind Conditions

- Max Rx = 1622 lbs, 1+2a+3+9
- Max Ry1= 3556 lbs, 1+2a+3+9
- Min Ry2= -339 lbs, 1+2b-3+9
- Max Ry1= 3206 lbs, 1+2a+3+9
- Min Ry2= 11 lbs, 1+2b-3+9
- Max Ry= 7807 lbs, 1+2a+3+9

SUBJECT LIGO Beam Tube Foundation Load Summary	OFFICE:	NOE-C	REVISION:	REFERENCE NO.	930212
	MADE BY	RJW	CHKD BY	WJC	SHT 3 OF 4
	DATE	3/22/94	DATE	4-4-94	9.3

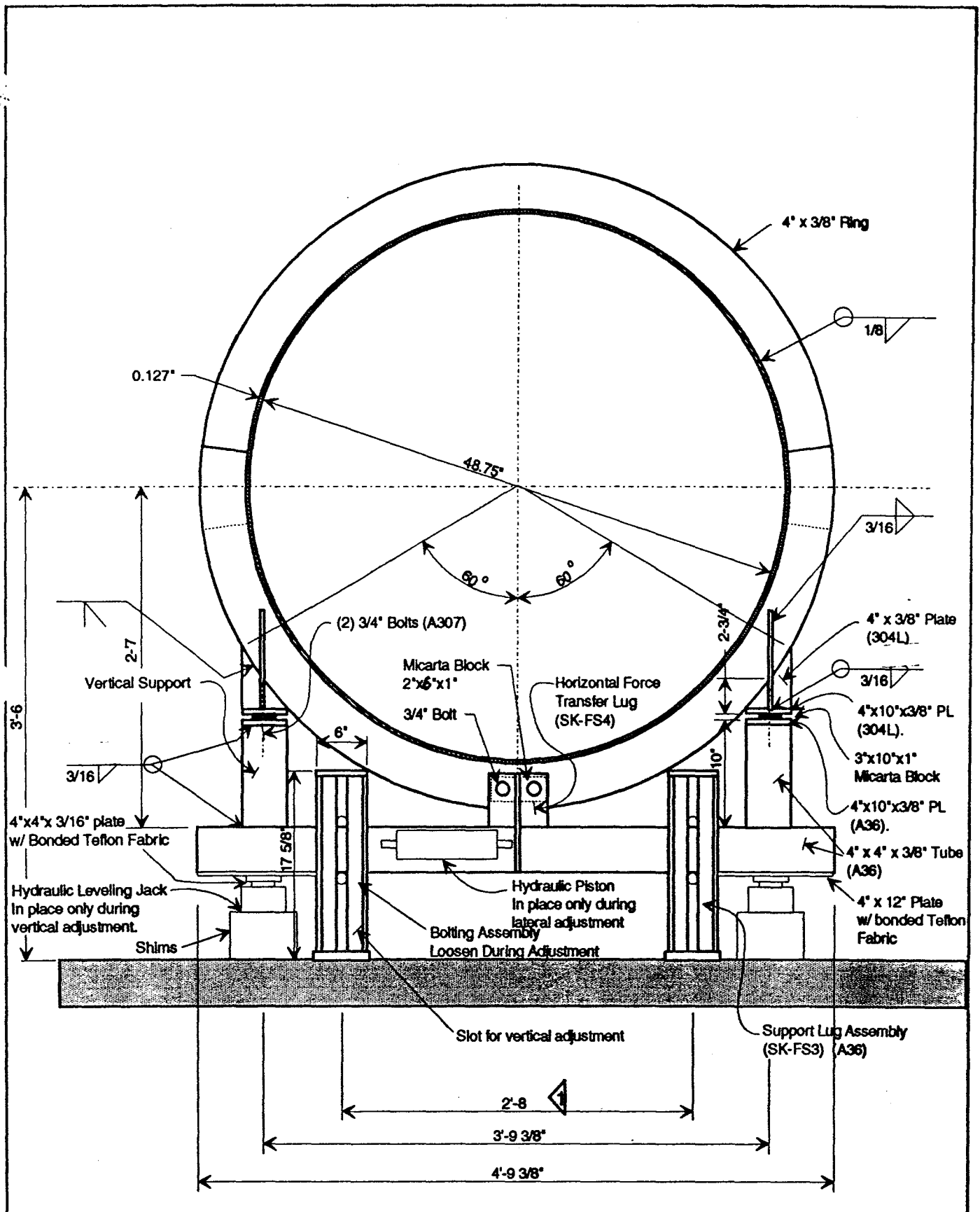
RESULTS OF LOAD COMBINATIONS

GUIDED SUPPORT FOUNDATION LOADS		Lateral	Vertical				
Load Case	Support Weight = 1600 lbs	Rx	Ry1	Ry2	Ry3	Ry4	Ry total
W/O Increased allowable							
1+2a+3		740	2977	2006	2627	1655	7807
1+4		85	2899	2788	2294	2183	7644
1+3+5		690	2776	1871	2426	1521	7135
1+7		85	1944	1833	1933	1821	7531
1+6+8		633	2186	1355	2180	1348	7069
1+2b-3		740	1211	240	1562	590	5061
1-4		85	429	318	1034	923	5224
1-3+5		690	1346	441	1696	791	5733
1-7		85	1384	1273	1396	1284	5337
1-6+8		633	1862	1031	1869	1037	5799
MAXIMUM		740	2977	2788	2627	2183	7807
MINIMUM		85	429	240	1034	590	5061

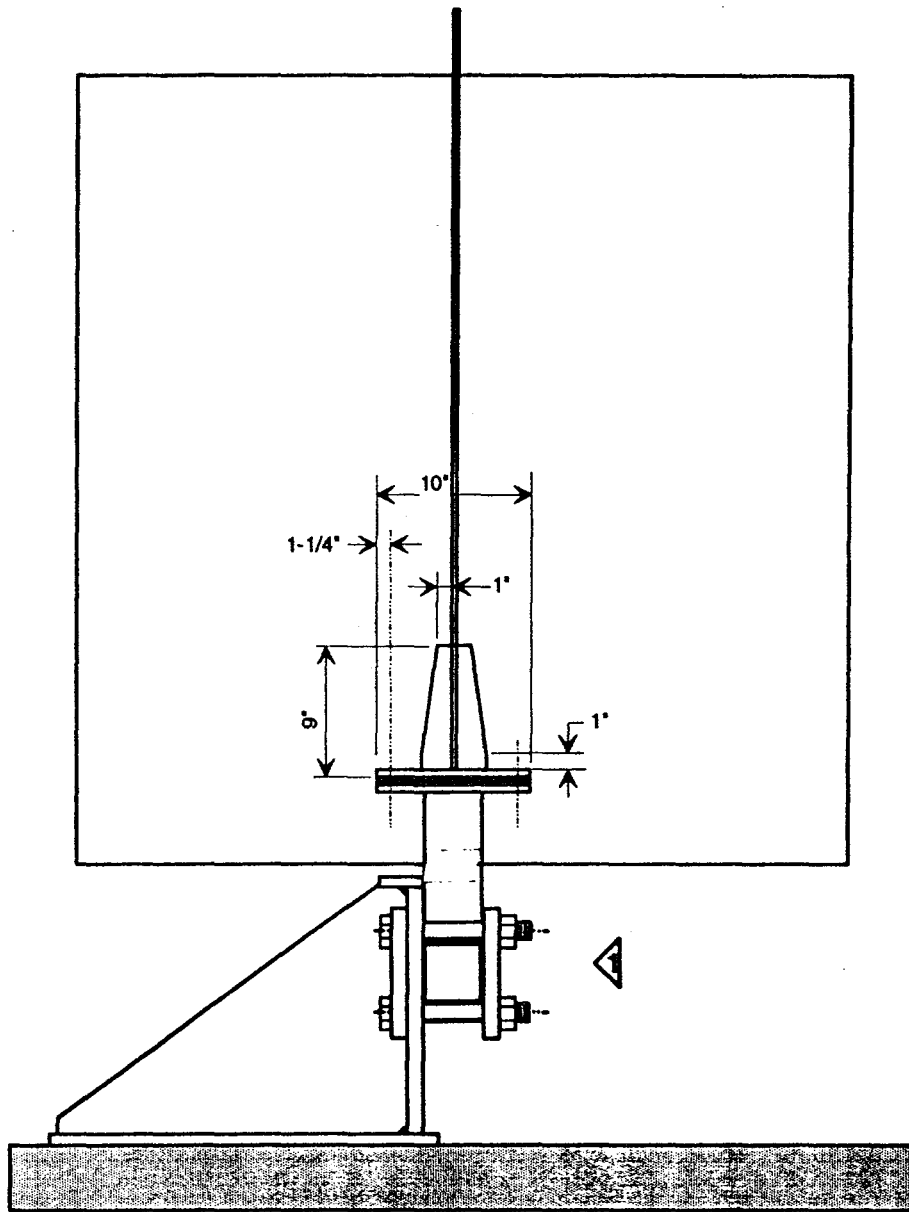
With Increased allowable

1+2a+3+9		1622	3556	1427	3206	1077	7807
1+2a+3+10		740	2977	2006	2627	1655	7807
1+2a+6+9		1622	3003	874	2997	867	7741
1+2a+6+10		740	2424	1453	2418	1446	7741
1+4+9		967	3478	2209	2873	1604	7644
1+4+10		85	2899	2788	2294	2183	7644
1+7+9		967	2523	1254	2511	1243	7531
1+7+10		85	1944	1833	1933	1821	7531
1+3+5+9		1572	3355	1292	3005	942	7135
1+3+5+10		690	2776	1871	2426	1521	7135
1+6+8+9		1515	2765	776	2758	770	7069
1+6+8+10		633	2186	1355	2180	1348	7069
1+2b-3+9		1622	1790	-339	2140	11	5061
1+2b-3+10		740	1211	240	1562	590	5061
1+2b-6+9		1622	2343	214	2350	220	5127
1+2b-6+10		740	1764	793	1771	799	5127
1-4+9		967	1008	-261	1613	344	5224
1-4+10		85	429	318	1034	923	5224
1-7+9		967	1963	694	1974	706	5337
1-7+10		85	1384	1273	1396	1284	5337
1-3+5+9		1572	1925	-138	2275	212	5733
1-3+5+10		690	1346	441	1696	791	5733
1-6+8+9		1515	2441	452	2447	459	5799
1-6+8+10		633	1862	1031	1869	1037	5799
MAXIMUM		1622	3556	2788	3206	2183	7807
MINIMUM		85	429	-339	1034	11	5061

SUBJECT LIGO Beam Tube Foundation Load Summary	OFFICE: NOE-C		REVISION:		REFERENCE NO. 930212
	MADE BY RJW	CHKD BY WJC	MADE BY	CHKD BY	SHT 4 OF 4
	DATE 3/22/94	DATE 4-4-94	DATE	DATE	9.4

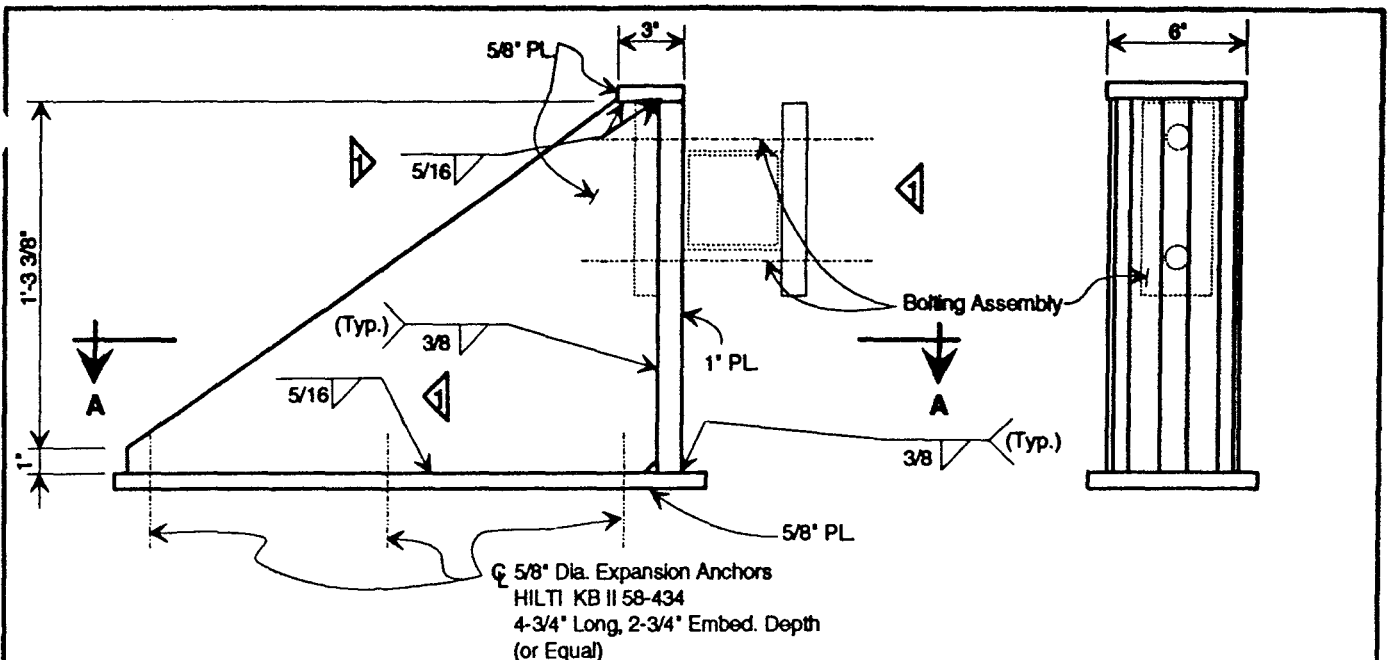


SUBJECT LIGO BEAM TUBE FIXED SUPPORT	CBI OFFICE		REVISION: 1		REFERENCE NO. 930212
	MADE MRS	CHK'D DDG	MADE MRS	CHK'D DDG	SHT 1 OF 48
	DATE 2/22/94	DATE 4/6/94	DATE 3/7/94	DATE 4/6/94	SK-FS1

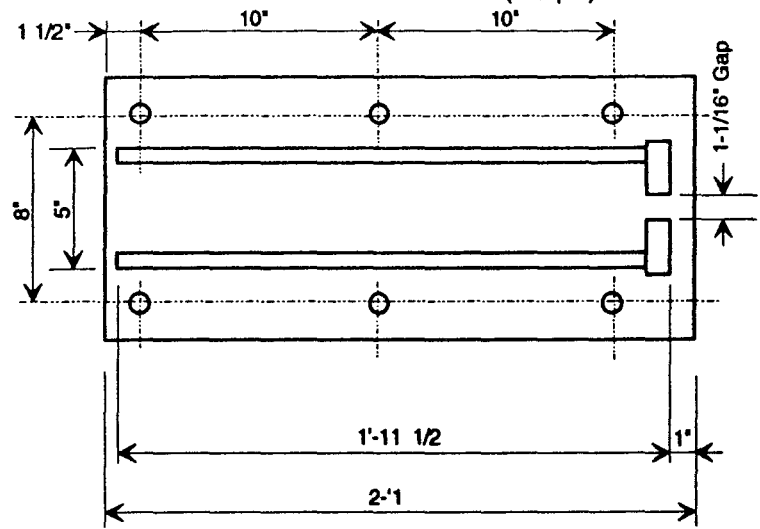


SIDE VIEW

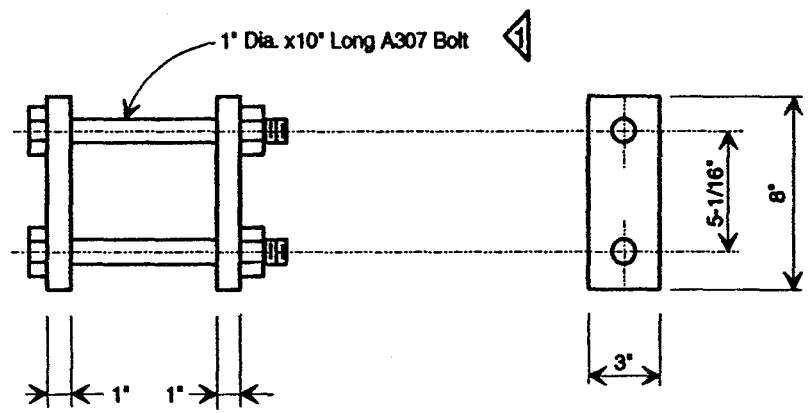
SUBJECT LIGO BEAM TUBE FIXED SUPPORT	OFFICE CBI		REVISION: 1		REFERENCE NO. 930212
	MADE MRS	CHK'D DDG	MADE MRS	CHK'D DDG	SHT <u>2</u> OF <u>48</u>
	DATE 2/19/94	DATE 4/6/94	DATE 3/7/94	DATE 4/6/94	SK - FS2



⌀ 5/8" Dia. Expansion Anchors
 HILTI KB II 58-434
 4-3/4" Long, 2-3/4" Embed. Depth
 (or Equal)

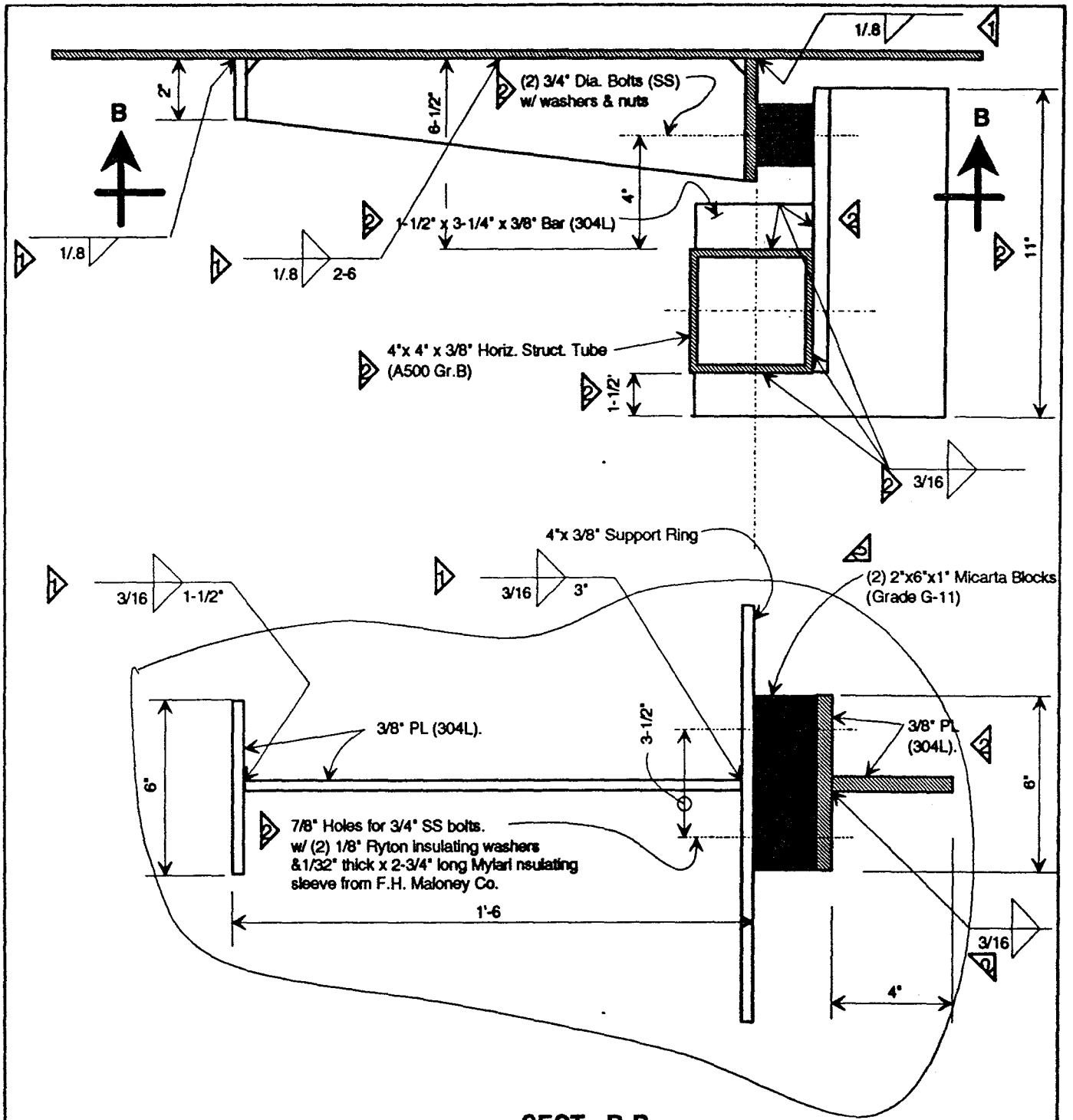


SECTION A-A



BOLTING ASSEMBLY

SUBJECT SUPPORT LUG ASSEMBLY BEAM TUBE FIXED SUPPORT LIGO	CBI OFFICE		REVISION: 1		REFERENCE NO. 931212
	MADE MRS	CHK'D DDG	MADE MRS	CHK'D DDG	SHT 3 OF 48
	DATE 2/23/94	DATE 4/6/94	DATE 3/7/94	DATE 4/6/94	SK - FS3



SECT. B-B

SUBJECT HORIZONTAL RESTRAINT BEAM TUBE FIXED SUPPORT LIGO	CBI OFFICE		REVISION: 2		REFERENCE NO. 930212
	MADE MRS	CHK'D DDG	MADE MRS	CHK'D DDG	SHT 4 OF 48
	DATE 2/24/94	DATE 4/6/94	DATE 3/7/94	DATE 4/6/94	SK - FS4

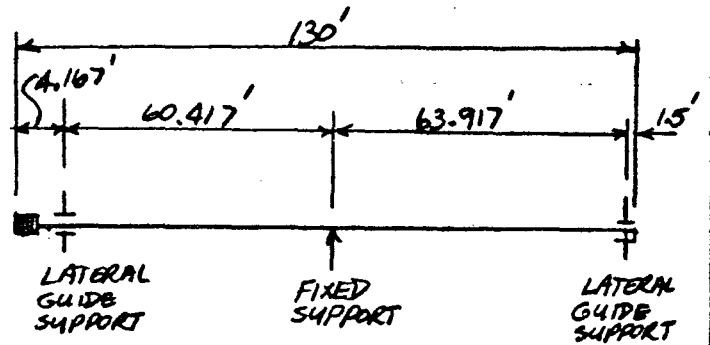
LOADS.XLS

Load Case	Fixed Support Loads	Magnitude		Direction	Comments				
1	DL (Metal Wt + Insul'n)	7,000	lbs	down	(per RISA-2D analysis for continuous beam system)				
2	Axial Seismic Load	2,260	lbs	axial	(~ 18% * 90 plf * 130 ft), assumes guided support does not take axial loads				
3	Lateral Seismic Load	1,130	lbs	lateral	(~ 18% * 90 plf * 65 ft), assumes guided support takes lateral loads				
4	Axial BakeOut Load	5,300	lbs	axial	(~ 3.2 inches of growth * 8300 lb/in bellows axial spring rate * 20% variation in bellows)				
5	Axial Contraction Load	830	lbs	axial	(Due to daily Delta T above assumed ambient)				
6	Axial Extension Load	2,158	lbs	axial	(Due to daily Delta T below assumed ambient)				
7	Vertical Stability Load	600	lbs	down	(Assumed)				
8	Lateral Stability Load	600	lbs	lateral	(Assumed)				
Comb'n	Load Combination					Vert Load	Axial Load	Lateral Load	Stress Increase
1	DL + Vertical Stability					7,600	0	0	1.000
2	DL + Axial EQ + Axial Ext'n + Vert Stability + Lat Stability					7,600	4,418	600	1.333
3	DL + Lat EQ + Axial Ext'n + Vert Stability + Lat Stability					7,600	2,158	1,730	1.333
4	DL + Axial BakeOut + Vert Stability + Lat Stability					7,600	5,300	600	1.000

10.5

Handwritten note: 10.5

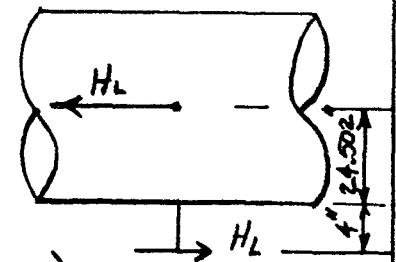
LONGITUDINAL TUBE LOADS



AXIAL TUBE LOAD FROM EXP. JTS. = 5300* (BAKE OUT)

AXIAL TUBE SEISMIC LOAD = 2260*

THE LONGITUDINAL TUBE FORCES WILL BE RESISTED AT THE FIXED SUPPORT AT 28.502" BELOW THE TUBE CENTERLINE. THE RESULTING MOMENT ON THE TUBE CROSS SECTION IS:



$$M = 28.502' (H_L) = 151,061 \text{ IN-LBS (BAKE OUT)}$$

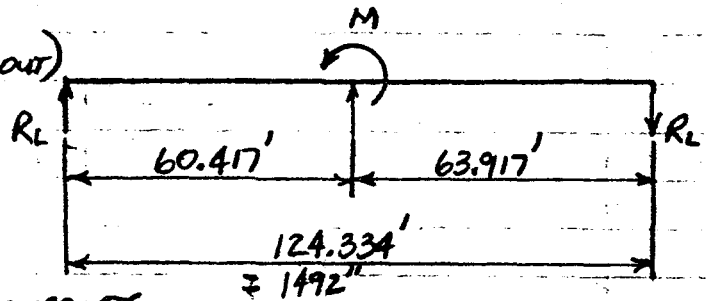
$$= 64,415 \text{ IN-LBS (SEISMIC)}$$

THE MOMENT WILL BE RESISTED BY THE LATERAL GUIDE SUPPORTS AS A COUPLE

$$R_L = \frac{M}{L}$$

$$R_L = \frac{151,061 \text{ IN-LBS}}{1492"} = \pm 102* \text{ (BAKE OUT)}$$

$$R_L = \frac{64,415 \text{ IN-LBS}}{1492"} = \pm 44* \text{ (SEISMIC)}$$



THESE LOADS ARE ADDED TO OTHER VERTICAL LOADS AT GUIDE SUPPORTS.

SUBJECT L40 BEAM TUBE FIXED SUPPORT DESIGN	OFFICE CBI NOE		REVISION		REFERENCE NO. 930212
	MADE BY MRS	CHKD BY DDG	MADE BY	CHKD BY	SHT. 6 OF 48
	DATE 2/18/94	DATE 4/5/94	DATE	DATE	10.6

ADDITIONAL STRESS IN TUBE WALL

$$M = 767''(102'') = 78,234 \text{ IN-LBS}$$

$$\text{TUBE SECTION MODULUS} = \pi r^2 t$$

$$r = \frac{48.75 + 0.127}{2} = 24.4385''$$

$$t = 0.127''$$

$$S = \pi r^2 t = \pi (24.4385'')^2 (0.127'')$$

$$S = 238.2 \text{ IN}^3$$

$$\sigma = \frac{M}{S} = \frac{78,234 \text{ IN-LBS}}{238.2 \text{ IN}^3} = 329 \text{ PSI}$$

SUBJECT	OFFICE CBI NOE		REVISION		REFERENCE NO. 930212
	MADE BY MRS	CHKD BY DDG	MADE BY	CHKD BY	SHT 7 OF 48
	DATE 2/22/94	DATE 4/5/94	DATE	DATE	10.7

SHELL RING DESIGN

LOAD COMBINATION 1

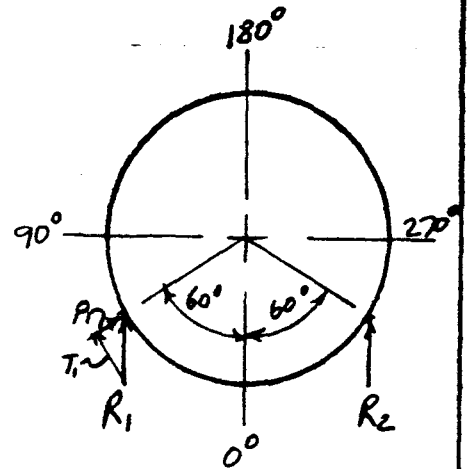
VERTICAL LOAD = 7600#

ASSUME 2 VERTICAL SUPPORTS @ 60° FROM INVERT.

$$R_1 = R_2 = \frac{7600}{2} = 3800\#$$

RADIAL COMPONENT (P) = $R \cos 60^\circ = 3800(.5) = 1900\#$

TANGENTIAL COMPONENT (T) = $R \sin 60^\circ = 3800(.866) = 3,291\#$



THESE LOADS WILL BE RESISTED BY A SINGLE RING COMPOSED OF A STIFFENER PLATE AND A PORTION OF TUBE WALL PARTICIPATION.

USE 4" x 3/8" STIFFENER

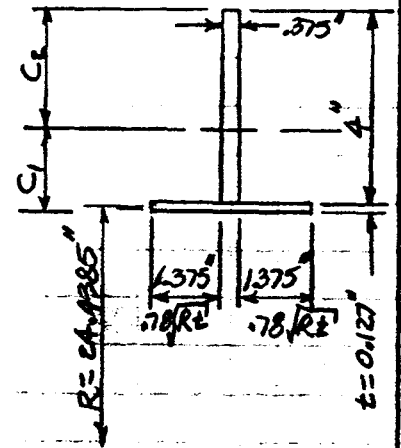
RING PROPERTIES CALCULATED ON NEXT SHEET.

$$A = 1.897 \text{ IN}^2$$

$$C_1 = 1.695" \quad C_2 = 2.432"$$

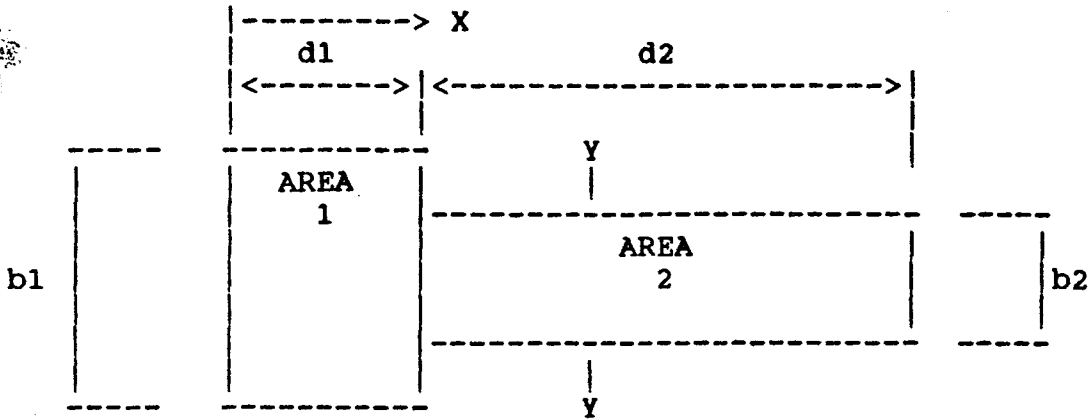
$$I = 3.34 \text{ IN}^4$$

$$S_1 = \frac{I}{C_1} = 1.97 \text{ IN}^3 \quad S_2 = \frac{I}{C_2} = 1.37 \text{ IN}^3$$



SUBJECT	OFFICE		REVISION		REFERENCE NO.
	CBI NOE				930212
	MADE BY	CHKD BY	MADE BY	CHKD BY	SHT 8 OF 48
	mrs	DDG			
DATE	DATE	DATE	DATE	10.8	
	2/18/94	4/5/94			

MOMENT OF INERTIA CALCULATION



	1	2	3	4	5	6	7
d	0.127	4					
b	3.125	0.375					
X	0.0635	2.1270	4.1270	4.1270	4.1270	4.1270	4.1270
THETA	0	0	0	0	0	0	0

	AREA	X	AX	AX ²	Io
1	0.397	0.064	0.03	0.00	0.00
2	1.500	2.127	3.19	6.79	2.00
3	0.000	4.127	0.00	0.00	0.00
4	0.000	4.127	0.00	0.00	0.00
5	0.000	4.127	0.00	0.00	0.00
6	0.000	4.127	0.00	0.00	0.00
7	0.000	4.127	0.00	0.00	0.00
	1.897		3.22		8.79

TOTAL DEPTH = 4.1270 in.

CENTROID (\bar{X}) = $\text{SUM}(AX) / \text{SUM}(AREA) = 1.695$ in.

$I(\text{total}) = [\text{SUM}(AX^2) + \text{SUM}(Io)] - (AREA)(\bar{X})^2 = 3.34$ in.⁴

$C1 = \bar{X} = 1.695$ in. $C2 = \text{DEPTH} - \bar{X} = 2.432$ in.

$Sy1 = I/C1 = 1.97$ in.³ $Sy2 = I/C2 = 1.37$ in.³

Radius of gyration (r) = $(I/A)^{1/2} = 1.326$ in.

Torsional constant (J) = in.⁴

SUBJECT	!MADE BY: MRS ! R	!MADE BY:	!REFERENCE NO.!
SECTION PROPERTIES	!-----!	E !-----!	930212 !
4" X 3/8" STIFFENER	!CHKD BY: DDG ! V	!CHKD BY:	!-----!
!FIXED SUPPORT SHELL RING	!-----!	!-----!	!-----!
!LIGO BEAM TUBE	!DATE: 2/18/94!	!DATE:	!SHT 9 OF 48 !

CBI COMPUTER PROGRAM EO405A

"ANALYSIS OF CIRCULAR RINGS WITH CONCENTRATED LOADS IN PLANE OF RING"

IS USED TO EVALUATE THE RING FOR THE APPLIED LOADS.

EO405A INPUT

2 RADIAL LOADS $1900^{\#}$ @ 60° & 300°

2 TANGENTIAL LOADS $-3291^{\#}$ @ 60°
 $+3291^{\#}$ @ 300°

$$R = 24.375'' + 1.695'' = 26.07''$$

$E = 28,000,000$ PSI @ AMBIENT

$I = 3.34$ IN⁴ $A = 1.897$ IN²

$C_1 = 1.695''$ $C_2 = 2.432''$

FROM EO405A OUTPUT (NEXT 2 SHEETS)

MAXIMUM INSIDE STRESS = $3,829$ PSI @ 60° & 300°

MAXIMUM OUTSIDE STRESS = $-6,404$ PSI @ 60° & 300°

$$V_{max} = (806 \text{ PSI})(1.897 \text{ IN}^2) = 1,529^{\#}$$

SUBJECT	OFFICE CBI NOE		REVISION		REFERENCE NO. 930212
	MADE BY MRS	CHKD BY DDG	MADE BY	CHKD BY	SHT 10 OF 48
	DATE 2/18/74	DATE 4/5/74	DATE	DATE	10.10

PROGRAM E0405A (STD 9110-1-2) [IBM RS/6000]
 STRESS AND STRAIN COEFFICIENTS FOR CIRCULAR RINGS
 REVISION 11 - JANUARY 7, 1992

STANDARDS 9110-1-1 AND 9110-1-2

FIXED SUPPORT DEAD LOAD - 2 PT. AT 60 DEG.

N(R) N(T) N(M) DATA SHEET NO. PLOT CODE
 2 2 0 1 0

RADIUS MOD-ELAS MOM-INER AREA C(1) C(2)
 26.07 28000000.00 3.34 1.90 1.70 2.43

EQUILIBRIUM LOADS FV FH M/R
 INPUT 7600.000 .000 .000
 COMPUTED 7600.179 .000 .000

INPUT LOADS AND THEIR LOCATIONS

RADIAL LOADS AND ANGLES

LOADS 1900.00 1900.00
 ANGLES 60.00 300.00

TANGENTIAL LOADS AND ANGLES

LOADS -3291.00 3291.00
 ANGLES 60.00 300.00

/0.11

NO MOMENT/R LOADS

UNIT STRESS VALUES FOR CIRCULAR RINGS

PRINTOUT AT EQUAL ANGLE INCREMENTS

X	V/A	T/A	M(FORCE-LG)	THETA	DR	DC	X
.00	0.0000E+00	-4.7821E+02	-2.0116E+03	0.0000E+00	-2.1789E-03	0.0000E+00	.
5.00	4.1538E+01	-4.7154E+02	-1.9218E+03	-4.8207E-05	-2.2258E-03	1.9151E-04	5.
10.00	8.1914E+01	-4.5162E+02	-1.6548E+03	-9.2063E-05	-2.3612E-03	3.9106E-04	10.
15.00	1.1998E+02	-4.1871E+02	-1.2181E+03	-1.2734E-04	-2.5695E-03	6.0575E-04	15.
20.00	1.5463E+02	-3.7324E+02	-6.2406E+02	-1.5005E-04	-2.8248E-03	8.4090E-04	20.
25.00	1.8478E+02	-3.1582E+02	1.1013E+02	-1.5656E-04	-3.0925E-03	1.0991E-03	25.
30.00	2.0942E+02	-2.4721E+02	9.6292E+02	-1.4373E-04	-3.3300E-03	1.3798E-03	30.
35.00	2.2762E+02	-1.6831E+02	1.9085E+03	-1.0896E-04	-3.4885E-03	1.6780E-03	35.
40.00	2.3853E+02	-8.0188E+01	2.9172E+03	-5.0360E-05	-3.5145E-03	1.9848E-03	40.
45.00	2.4139E+02	1.5976E+01	3.9558E+03	3.3212E-05	-3.3524E-03	2.2860E-03	45.
50.00	2.3555E+02	1.1888E+02	4.9882E+03	1.4205E-04	-2.9460E-03	2.5628E-03	50.
55.00	2.2048E+02	2.2711E+02	5.9756E+03	2.7554E-04	-2.2414E-03	2.7915E-03	55.
60.00	-8.0579E+02	-1.3957E+03	6.8774E+03	4.3209E-04	-1.1897E-03	2.9439E-03	60.
65.00	-6.8535E+02	-1.3620E+03	3.6608E+03	5.5922E-04	2.1246E-04	2.9886E-03	65.
70.00	-5.6849E+02	-1.3139E+03	9.5666E+02	6.1436E-04	1.8179E-03	2.9009E-03	70.
75.00	-4.5641E+02	-1.2525E+03	-1.2530E+03	6.0977E-04	3.4647E-03	2.6702E-03	75.
80.00	-3.5024E+02	-1.1790E+03	-2.9914E+03	5.5722E-04	5.0180E-03	2.2989E-03	80.
85.00	-2.5094E+02	-1.0949E+03	-4.2860E+03	4.6783E-04	6.3697E-03	1.8003E-03	85.
90.00	-1.5941E+02	-1.0016E+03	-5.1685E+03	3.5203E-04	7.4377E-03	1.1955E-03	90.
95.00	-7.6353E+01	-9.0068E+02	-5.6741E+03	2.1942E-04	8.1651E-03	5.1206E-04	95.
100.00	-2.3817E+00	-7.9374E+02	-5.8406E+03	7.8717E-05	8.5180E-03	-2.1867E-04	100.
105.00	6.2055E+01	-6.8245E+02	-5.7084E+03	-6.2322E-05	8.4845E-03	-9.6335E-04	105.
110.00	4.4445E+02	-5.4848E+02	-5.3192E+03	-1.9693E-04	8.0718E-03	-1.6884E-03	110.

20.00	1.9582E+02	-3.3916E+02	-3.9418E+03	-4.2477E-04	0.2213E-03	2.7071E-03	125
25.00	2.2050E+02	-2.2709E+02	-3.0399E+03	-5.1013E-04	4.8738E-03	-3.4400E-03	125
30.00	2.3557E+02	-1.1886E+02	-2.0524E+03	-5.7220E-04	3.3212E-03	-3.7988E-03	130
35.00	2.4140E+02	-1.5957E+01	-1.0199E+03	-6.0962E-04	1.6301E-03	-4.0156E-03	135
40.00	2.3855E+02	8.0209E+01	1.8778E+01	-6.2177E-04	-1.2987E-04	-4.0813E-03	140
45.00	2.2764E+02	1.6833E+02	1.0275E+03	-6.0895E-04	-1.8879E-03	-3.9930E-03	145
50.00	2.0944E+02	2.4723E+02	1.9731E+03	-5.7229E-04	-3.5751E-03	-3.7538E-03	150
55.00	1.8479E+02	3.1585E+02	2.8260E+03	-5.1370E-04	-5.1263E-03	-3.3730E-03	155
60.00	1.5464E+02	3.7327E+02	3.5602E+03	-4.3576E-04	-6.4828E-03	-2.8648E-03	160
65.00	1.1999E+02	4.1874E+02	4.1543E+03	-3.4163E-04	-7.5936E-03	-2.2487E-03	165
70.00	8.1918E+01	4.5165E+02	4.5910E+03	-2.3492E-04	-8.4176E-03	-1.5478E-03	170
75.00	4.1540E+01	4.7157E+02	4.8580E+03	-1.1964E-04	-8.9245E-03	-7.8875E-04	175
80.00	0.0000E+00	4.7824E+02	4.9479E+03	0.0000E+00	-9.0955E-03	0.0000E+00	180
80.00	0.0000E+00	4.7824E+02	4.9479E+03	0.0000E+00	-9.0955E-03	0.0000E+00	180

PRINTOUT AT LOAD AND/OR MOMENT POINTS

60.00	-8.0579E+02	-1.3957E+03	6.8774E+03	4.3209E-04	-1.1897E-03	2.9439E-03	60
59.99	1.9586E+02	3.3895E+02	6.8757E+03	4.3176E-04	-1.1922E-03	2.9437E-03	60
300.00	-1.9580E+02	3.3917E+02	6.8774E+03	-4.3209E-04	-1.1897E-03	-2.9439E-03	300
299.99	8.0554E+02	-1.3956E+03	6.8704E+03	-4.3243E-04	-1.1873E-03	-2.9441E-03	300

PRINTOUT OF MAXIMUM AND MINIMUM VALUES OF SHEAR, THRUST, AND MOMENT VALUES TAKEN FROM BOTH OF THE TABLES ABOVE

MAXIMUM V/A=	8.0554E+02	AT	299.99	DEGREES
MINIMUM V/A=	-8.0579E+02	AT	60.00	DEGREES
MAXIMUM T/A=	4.7824E+02	AT	180.00	DEGREES
MINIMUM T/A=	-1.3957E+03	AT	60.00	DEGREES
MAXIMUM M(FORCE-LG)=	6.8774E+03	AT	60.00	DEGREES
MINIMUM M(FORCE-LG)=	-5.8406E+03	AT	100.00	DEGREES

PRINTOUT OF MAXIMUM AND MINIMUM (AXIAL + BENDING) STRESSES FOR MOMENT ARMS OF C1 AND C2 - VALUES TAKEN FROM BOTH OF THE TABLES ABOVE

MAXIMUM STRESS (C1)=	3.8293E+03	AT	300.00	DEGREES
MINIMUM STRESS (C1)=	-3.7802E+03	AT	95.00	DEGREES
MAXIMUM STRESS (C2)=	3.4741E+03	AT	105.00	DEGREES
MINIMUM STRESS (C2)=	-6.4034E+03	AT	60.00	DEGREES

TOP [SYSIN]

DDG 4/5/94

10.12

LOAD COMBINATION 3

VERTICAL LOAD = 7600 #

LATERAL SEISMIC LOAD = 1,730 #

$H = 1730 \#$

VERTICAL LOADS DUE TO SEISMIC

$\Sigma M_{@H} = 0$

$(1730 \#)(28.502'') - 22.577'(R_1) - 22.577'(R_2) = 0$

$R_2 = -R_1$

$R = 1092 \#$

VERTICAL LOAD DUE TO DL

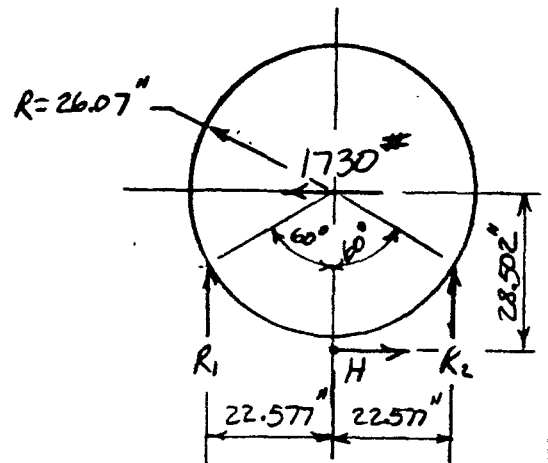
$R_1 = R_2 = \frac{7600 \#}{2} = 3800 \#$

$R_1 = 3800 \# + 1092 \# = 4892 \#$

$R_2 = 3800 \# - 1092 \# = 2708 \#$

MOMENT @ INVERT = $(4.127' - 1.695')(1730 \#) = 4,208 \text{ IN-LBS}$

USING SAME RING AS LOAD #1



SUBJECT	OFFICE CBI NOE		REVISION		REFERENCE NO. 930212
	MADE BY MRS	CHKD BY DDG	MADE BY	CHKD BY	SHT 12 OF 48
	DATE 2/18/94	DATE 4/5/94	DATE	DATE	10.13

INPUT TO CBI COMPUTER PROGRAM E0405A

2 RADIAL LOADS @ 60° & 300°

$$P_{60} = (4892^{\#}) \cos 60^{\circ} = 2446^{\#} @ 60^{\circ}$$

$$P_{300} = (2708^{\#}) \cos 60^{\circ} = 1354^{\#} @ 300^{\circ}$$

3 TANGENTIAL LOADS @ 0°, 60° & 300°

$$T_0 = 1730^{\#}$$

$$T_{60} = (4892^{\#})(\sin 60^{\circ}) = -4237^{\#} @ 60^{\circ}$$

$$T_{300} = (2708^{\#})(\sin 60^{\circ}) = -2345^{\#} @ 300^{\circ}$$

1 MOMENT LOAD @ 0°

$$M/R = \frac{4208 \text{ IN-LBS}}{26.07^{\#}} = 161.4^{\#} @ 0^{\circ}$$

$$R = 26.07^{\#} \quad E = 28,000,000 \text{ PSI} \quad I = 3.34 \text{ IN}^4$$

$$A = 1.897 \text{ IN}^2 \quad C_1 = 1.695^{\#} \quad C_2 = 2.432^{\#}$$

SUBJECT	OFFICE		REVISION		REFERENCE NO.
	CBI	NOE			930212
	MADE BY	CHKD BY	MADE BY	CHKD BY	SHT 14 OF 48
	MPS	DDG			
	DATE	DATE	DATE	DATE	10.14
	2/18/94	4/5/94			

FROM E0405A OUTPUT (NEXT 2 SHEETS)

MAXIMUM INSIDE STRESS = 7,445 PSI @ 60°

MINIMUM INSIDE STRESS = -5,810 PSI @ 105°

MAXIMUM OUTSIDE STRESS = 6,025 PSI @ 110°

MINIMUM INSIDE STRESS = -11,175 PSI @ 60°

$$V_{MAX} = (1155 \text{ PSI})(1.897 \text{ IN}^2) = 2,191 \text{ #} @ 60^\circ$$

STIFFENER TO SHELL WELD

$$f = \frac{VQ}{I_n}$$

$$V = 2191 \text{ #}$$

$$I = 3.34 \text{ IN}^4$$

$$Q = A' \bar{y} \quad A' = [(2)(1.375) + .375](.127) = 0.397 \text{ IN}^2$$

$$\bar{y} = C_1 - t/2 = 1.695 - .127/2 = 1.632 \text{ #}$$

$$f = \frac{2191(1.632)(0.397 \text{ IN}^2)}{3.34 \text{ IN}^4} = 425 \text{ #/IN}$$

SUBJECT	OFFICE		REVISION		REFERENCE NO.
	MADE BY	CHKD BY	MADE BY	CHKD BY	SHEET NO. OF
	CBI	NOE			930212
	MRS	DDG			15 OF 48
	DATE	DATE	DATE	DATE	10.15
	2/18/94	4/5/94			

PROGRAM E0405A (STD 9110-1-2) [IBM RS/6000]
 STRESS AND STRAIN COEFFICIENTS FOR CIRCULAR RINGS
 REVISION 11 - JANUARY 7, 1992

STANDARDS 9110-1-1 AND 9110-1-2

FIXED SUPPORT DEAD LOAD + LATERAL SEISMIC - 2 PT. AT 60 DEG.

N(R) N(T) N(M) DATA SHEET NO. PLOT CODE
 2 3 1 1 0

RADIUS MOD-ELAS MOM-INER AREA C(1) C(2)
 26.07 28000000.00 3.34 1.90 1.70 2.43

EQUILIBRIUM LOADS FV FH M/R
 INPUT 7600.000 1730.000 .000
 COMPUTED 7600.179 1729.700 .600

INPUT LOADS AND THEIR LOCATIONS

RADIAL LOADS AND ANGLES

LOADS 2446.00 1354.00
 ANGLES 60.00 300.00

TANGENTIAL LOADS AND ANGLES

LOADS 1730.00 -4237.00 2345.00
 ANGLES .00 60.00 300.00

MOMENT LOADS/R AND ANGLES

LOADS -161.40
 ANGLES .00

10.16

UNIT STRESS VALUES FOR CIRCULAR RINGS

PRINTOUT AT EQUAL ANGLE INCREMENTS

X	V/A	T/A	M(FORCE-LG)	THETA	DR	DC	
.00	4.0410E+02	-2.2226E+01	-4.1155E+03	-7.4310E-04	-2.1789E-03	-6.9789E-03	
5.00	4.0547E+02	-7.3311E+00	-2.3680E+03	-8.2198E-04	-4.5650E-03	-6.6858E-03	5
10.00	4.0510E+02	1.7475E+01	-6.1818E+02	-8.5829E-04	-7.0473E-03	-6.1794E-03	10
15.00	4.0213E+02	5.2083E+01	1.1248E+03	-8.5211E-04	-9.5102E-03	-5.4563E-03	15
20.00	3.9573E+02	9.6235E+01	2.8479E+03	-8.0373E-04	-1.1839E-02	-4.5234E-03	20
25.00	3.8507E+02	1.4952E+02	4.5344E+03	-7.1384E-04	-1.3920E-02	-3.3973E-03	25
30.00	3.6938E+02	2.1140E+02	6.1643E+03	-5.8357E-04	-1.5644E-02	-2.1044E-03	30
35.00	3.4794E+02	2.8117E+02	7.7144E+03	-4.1457E-04	-1.6909E-02	-6.8027E-04	35
40.00	3.2010E+02	3.5801E+02	9.1584E+03	-2.0910E-04	-1.7619E-02	8.3066E-04	40
45.00	2.8528E+02	4.4098E+02	1.0467E+04	2.9919E-05	-1.7689E-02	2.3762E-03	45
50.00	2.4299E+02	5.2901E+02	1.1610E+04	2.9882E-04	-1.7046E-02	3.8972E-03	50
55.00	1.9284E+02	6.2094E+02	1.2553E+04	5.9317E-04	-1.5632E-02	5.3289E-03	55
60.00	-1.1549E+03	-1.5180E+03	1.3263E+04	9.0769E-04	-1.3406E-02	6.6019E-03	60
65.00	-1.0219E+03	-1.5260E+03	8.5653E+03	1.1720E-03	-1.0395E-02	7.6455E-03	65
70.00	-8.8904E+02	-1.5163E+03	4.4420E+03	1.3291E-03	-6.8290E-03	8.4002E-03	70
75.00	-7.5777E+02	-1.4896E+03	8.8923E+02	1.3928E-03	-2.9622E-03	8.8287E-03	75
80.00	-6.2952E+02	-1.4470E+03	-2.1030E+03	1.3769E-03	9.7884E-04	8.9150E-03	80
85.00	-5.0565E+02	-1.3895E+03	-4.5508E+03	1.2949E-03	4.7986E-03	8.6614E-03	85
90.00	-3.8741E+02	-1.3184E+03	-6.4757E+03	1.1597E-03	8.3327E-03	8.0859E-03	90
95.00	-2.7504E+02	-1.2350E+03	-7.9044E+03	9.8384E-04	1.1447E-02	7.2193E-03	95

105.00	-7.7062E+01	-1.0376E+03	-9.4029E+03	3.3384E-04	1.8037E-02	4.7882E-03	105
110.00	8.6972E+00	-9.2676E+02	-9.5470E+03	3.2460E-04	1.7394E-02	3.3228E-03	110
115.00	8.4520E+01	-8.1014E+02	-9.3422E+03	9.4178E-05	1.8092E-02	1.7696E-03	115
120.00	1.4998E+02	-6.8945E+02	-8.8324E+03	-1.2746E-04	1.8137E-02	1.8414E-04	120
125.00	2.0479E+02	-5.6646E+02	-8.0630E+03	-3.3344E-04	1.7557E-02	-1.3777E-03	125
130.00	2.4882E+02	-4.4290E+02	-7.0803E+03	-5.1802E-04	1.6398E-02	-2.8633E-03	130
135.00	2.8212E+02	-3.2052E+02	-5.9307E+03	-6.7657E-04	1.4723E-02	-4.2247E-03	135
140.00	3.0485E+02	-2.0100E+02	-4.6604E+03	-8.0559E-04	1.2609E-02	-5.4202E-03	140
145.00	3.1733E+02	-8.5962E+01	-3.3142E+03	-9.0270E-04	1.0142E-02	-6.4151E-03	145
150.00	3.2003E+02	2.3053E+01	-1.9354E+03	-9.6657E-04	7.4143E-03	-7.1827E-03	150
155.00	3.1353E+02	1.2460E+02	-5.6508E+02	-9.9693E-04	4.5230E-03	-7.7044E-03	155
160.00	2.9854E+02	2.1733E+02	7.5859E+02	-9.9445E-04	1.5659E-03	-7.9702E-03	160
165.00	2.7589E+02	3.0006E+02	2.0007E+03	-9.6068E-04	-1.3616E-03	-7.9786E-03	165
170.00	2.4649E+02	3.7170E+02	3.1302E+03	-8.9802E-04	-4.1686E-03	-7.7361E-03	170
175.00	2.1136E+02	4.3134E+02	4.1201E+03	-8.0952E-04	-6.7714E-03	-7.2570E-03	175
180.00	1.7158E+02	4.7824E+02	4.9479E+03	-6.9888E-04	-9.0955E-03	-6.5624E-03	180
180.00	1.7158E+02	4.7824E+02	4.9479E+03	-6.9888E-04	-9.0955E-03	-6.5624E-03	180

PRINTOUT AT LOAD AND/OR MOMENT POINTS

.00	4.0410E+02	-2.2226E+01	-4.1155E+03	-7.4310E-04	-2.1789E-03	-6.9789E-03	
-.01	4.0394E+02	-9.3421E+02	8.8749E+01	-7.4311E-04	-2.1743E-03	-6.9793E-03	
60.00	-1.1549E+03	-1.5180E+03	1.3263E+04	9.0769E-04	-1.3406E-02	6.6019E-03	60
59.99	1.3466E+02	7.1532E+02	1.3262E+04	9.0705E-04	-1.3412E-02	6.5996E-03	60
300.00	-2.5705E+02	-3.7167E+01	4.9202E+02	4.3509E-05	1.1027E-02	7.1422E-04	300
299.99	4.5648E+02	-1.2732E+03	4.8808E+02	4.3485E-05	1.1027E-02	7.1615E-04	300

PRINTOUT OF MAXIMUM AND MINIMUM VALUES OF SHEAR, THRUST, AND MOMENT VALUES TAKEN FROM BOTH OF THE TABLES ABOVE

MAXIMUM V/A=	4.5648E+02	AT	299.99	DEGREES
MINIMUM V/A=	-1.1549E+03	AT	60.00	DEGREES
MAXIMUM T/A=	7.1532E+02	AT	59.99	DEGREES
MINIMUM T/A=	-1.5260E+03	AT	65.00	DEGREES
MAXIMUM M(FORCE-LG)=	1.3263E+04	AT	60.00	DEGREES
MINIMUM M(FORCE-LG)=	-9.5470E+03	AT	110.00	DEGREES

PRINTOUT OF MAXIMUM AND MINIMUM (AXIAL + BENDING) STRESSES FOR MOMENT ARMS OF C1 AND C2 - VALUES TAKEN FROM BOTH OF THE TABLES ABOVE

MAXIMUM STRESS (C1)=	7.4454E+03	AT	59.99	DEGREES
MINIMUM STRESS (C1)=	-5.8094E+03	AT	105.00	DEGREES
MAXIMUM STRESS (C2)=	6.0248E+03	AT	110.00	DEGREES
MINIMUM STRESS (C2)=	-1.1175E+04	AT	60.00	DEGREES

TOP [SYSIN]

DDG 4/5/94

10.17

LOAD COMBINATION #4

VERTICAL LOAD = 7600*

LATERAL STABILITY = 600*

H = 600*

VERTICAL LOADS DUE TO LATERAL FORCES

$\Sigma M @ H = 0$

$(600^*)(28.502'') - 2(22.577'')(R) = 0$

$R = \pm 379^*$

TOTAL VERTICAL LOADS

$R_1 = \frac{7600^*}{2} + 379^* = 4179^*$ $R_2 = \frac{7600^*}{2} - 379^* = 3421^*$

MOMENT @ INVERT (M) = $(4.127' - 1.695')(600^*) = 1459$ IN-LBS

INPUT INTO PROGRAM E0905A

$P_{60} = (4179^*) \cos 60^\circ = 2090^*$ $P_{300} = (3421^*) \cos 60^\circ = 1711^*$

$T_0 = 600^*$, $T_{60} = (-4179^*) \sin 60^\circ = -3619^*$, $T_{300} = (3421^*) \sin 60^\circ = 2963^*$

$M_y/R = \frac{-1459 \text{ IN-LBS}}{26.07'} = -56.0^*$

SUBJECT	OFFICE NOE		REVISION		REFERENCE NO.
	CBI				930212
	MADE BY MRS	CHKD BY DIG	MADE BY	CHKD BY	SHT 18 OF 48
	DATE 2/12/94	DATE 4/5/94	DATE	DATE	10.18

FROM E0905A OUTPUT (NEXT 2 SHEETS)

MAXIMUM INSIDE STRESS = 5,085 PSI @ 60°

MINIMUM INSIDE STRESS = -4,411 PSI @ 100°

MAXIMUM OUTSIDE STRESS = 4,284 PSI @ 105°

MINIMUM OUTSIDE STRESS = -8,061 PSI @ 60°

$$V_{max} = (927 \text{ PSI})(1.897 \text{ IN}^2) = 1,759 \text{ \#}$$

SUBJECT	OFFICE		REVISION		REFERENCE NO.
	CBI	NOE			930212
	MADE BY	CHKD BY	MADE BY	CHKD BY	SHT 19 OF 48
	MRS	DIG			
	DATE	DATE	DATE	DATE	10.99
	2/18/94	4/5/94			

PROGRAM E0405A (STD 9110-1-2) [IBM RS/6000]
 STRESS AND STRAIN COEFFICIENTS FOR CIRCULAR RINGS
 REVISION 11 - JANUARY 7, 1992

STANDARDS 9110-1-1 AND 9110-1-2

FIXED SUPPORT LOAD COMBINATION #4 - 2 PT. AT 60 DEG.

N(R) 2 N(T) 3 N(M) 1 DATA SHEET NO. 1 PLOT CODE 0

RADIUS 26.07 MOD-ELAS 28000000.00 MOM-INER 3.34 AREA 1.90 C(1) 1.70 C(2) 2.43

EQUILIBRIUM LOADS FV FH M/R
 INPUT 7600.000 600.000 .000
 COMPUTED 7600.679 600.224 .000

INPUT LOADS AND THEIR LOCATIONS

RADIAL LOADS AND ANGLES

LOADS 2090.00 1711.00
 ANGLES 60.00 300.00

TANGENTIAL LOADS AND ANGLES

LOADS 600.00 -3619.00 2963.00
 ANGLES .00 60.00 300.00

MOMENT LOADS/R AND ANGLES

LOADS -56.00
 ANGLES .00

10.20

INIT STRESS VALUES FOR CIRCULAR RINGS

PRINTOUT AT EQUAL ANGLE INCREMENTS

X	V/A	T/A	M(FORCE-LG)	THETA	DR	DC	
.00	1.4016E+02	-3.2028E+02	-2.7439E+03	-2.5774E-04	-2.1745E-03	-2.4209E-03	
5.00	1.6779E+02	-3.1076E+02	-2.0791E+03	-3.1665E-04	-3.0329E-03	-2.1945E-03	5.
10.00	1.9405E+02	-2.8915E+02	-1.2976E+03	-3.5795E-04	-3.9827E-03	-1.8889E-03	10.
15.00	2.1791E+02	-2.5565E+02	-4.0763E+02	-3.7890E-04	-4.9734E-03	-1.4982E-03	15.
20.00	2.3833E+02	-2.1064E+02	5.7828E+02	-3.7701E-04	-5.9483E-03	-1.0213E-03	20.
25.00	2.5435E+02	-1.5464E+02	1.6432E+03	-3.5013E-04	-6.8457E-03	-4.6226E-04	25.
30.00	2.6502E+02	-8.8358E+01	2.7660E+03	-2.9659E-04	-7.5998E-03	1.6933E-04	30.
35.00	2.6950E+02	-1.2616E+01	3.9218E+03	-2.1528E-04	-8.1429E-03	8.5802E-04	35.
40.00	2.6698E+02	7.1606E+01	5.0821E+03	-1.0574E-04	-8.4071E-03	1.5824E-03	40.
45.00	2.5678E+02	1.6321E+02	6.2152E+03	3.1761E-05	-8.3263E-03	2.3153E-03	45.
50.00	2.3831E+02	2.6097E+02	7.2866E+03	1.9615E-04	-7.8388E-03	3.0238E-03	50.
55.00	2.1109E+02	3.6356E+02	8.2596E+03	3.8548E-04	-6.8890E-03	3.6700E-03	55.
60.00	-9.2698E+02	-1.4382E+03	9.0956E+03	5.9690E-04	-5.4305E-03	4.2113E-03	60.
65.00	-8.0220E+02	-1.4190E+03	5.3648E+03	7.7169E-04	-3.4707E-03	4.6029E-03	65.
70.00	-6.7977E+02	-1.3842E+03	2.1680E+03	8.6224E-04	-1.1853E-03	4.8077E-03	70.
75.00	-5.6103E+02	-1.3348E+03	-5.0794E+02	8.8140E-04	1.2315E-03	4.8059E-03	75.
80.00	-4.4719E+02	-1.2721E+03	-2.6816E+03	8.4161E-04	3.6131E-03	4.5938E-03	80.
85.00	-3.3937E+02	-1.1972E+03	-4.3766E+03	7.5481E-04	5.8211E-03	4.1804E-03	85.
90.00	-2.3856E+02	-1.1116E+03	-5.6210E+03	6.3263E-04	7.7449E-03	3.5861E-03	90.
95.00	-1.4574E+02	-1.0147E+03	-6.4471E+03	4.8472E-04	9.3007E-03	2.8394E-03	95.

05.00	1.3750E+01	-8.0571E+02	-6.9897E+03	1.5227E-04	1.1102E-02	1.0522E-03	100.0
10.00	7.9162E+01	-6.9283E+02	-6.7857E+03	-1.5856E-05	1.1304E-02	5.1176E-05	110.0
15.00	1.3459E+02	-5.7727E+02	-6.3208E+03	-1.7576E-04	1.1045E-02	-9.2726E-04	115.0
20.00	1.7989E+02	-4.6073E+02	-5.6385E+03	-3.2162E-04	1.0354E-02	-1.8640E-03	120.0
25.00	2.1503E+02	-3.4487E+02	-4.7827E+03	-4.4869E-04	9.2732E-03	-2.7231E-03	125.0
30.00	2.4014E+02	-2.3132E+02	-3.7970E+03	-5.5326E-04	7.8577E-03	-3.4727E-03	130.0
35.00	2.5551E+02	-1.2165E+02	-2.7240E+03	-6.3272E-04	6.1727E-03	-4.0866E-03	135.0
40.00	2.6153E+02	-1.7382E+01	-1.6050E+03	-6.8542E-04	4.2904E-03	-4.5443E-03	140.0
45.00	2.5874E+02	8.0083E+01	-4.7920E+02	-7.1075E-04	2.2866E-03	-4.8319E-03	145.0
50.00	2.4779E+02	1.6943E+02	6.1664E+02	-7.0898E-04	2.3881E-04	-4.9421E-03	150.0
55.00	2.2944E+02	2.4948E+02	1.6490E+03	-6.8126E-04	-1.7770E-03	-4.8745E-03	155.0
60.00	2.0455E+02	3.1915E+02	2.5877E+03	-6.2951E-04	-3.6885E-03	-4.6350E-03	160.0
65.00	1.7407E+02	3.7755E+02	3.4065E+03	-5.5634E-04	-5.4294E-03	-4.2357E-03	165.0
70.00	1.3901E+02	4.2390E+02	4.0836E+03	-4.6492E-04	-6.9412E-03	-3.6941E-03	170.0
75.00	1.0045E+02	4.5760E+02	4.6013E+03	-3.5895E-04	-8.1750E-03	-3.0324E-03	175.0
80.00	5.9524E+01	4.7822E+02	4.9472E+03	-2.4245E-04	-9.0929E-03	-2.2765E-03	180.0
80.00	5.9524E+01	4.7822E+02	4.9472E+03	-2.4245E-04	-9.0929E-03	-2.2765E-03	180.0

RINTOUT AT LOAD AND/OR MOMENT POINTS

.00	1.4016E+02	-3.2028E+02	-2.7439E+03	-2.5774E-04	-2.1745E-03	-2.4209E-03	.0
-.01	1.4005E+02	-6.3658E+02	-1.2852E+03	-2.5768E-04	-2.1729E-03	-2.4213E-03	.0
60.00	-9.2698E+02	-1.4382E+03	9.0956E+03	5.9690E-04	-5.4305E-03	4.2113E-03	60.0
59.99	1.7484E+02	4.6937E+02	9.0940E+03	5.9645E-04	-5.4340E-03	4.2104E-03	60.0
00.00	-2.1717E+02	2.0862E+02	4.6648E+03	-2.6697E-04	3.0455E-03	-1.6735E-03	300.0
99.99	6.8454E+02	-1.3532E+03	4.6589E+03	-2.6720E-04	3.0470E-03	-1.6730E-03	300.0

RINTOUT OF MAXIMUM AND MINIMUM VALUES OF SHEAR, THRUST, AND MOMENT ALUES TAKEN FROM BOTH OF THE TABLES ABOVE

MAXIMUM V/A=	6.8454E+02	AT	299.99	DEGREES
MINIMUM V/A=	-9.2698E+02	AT	60.00	DEGREES
MAXIMUM T/A=	4.7822E+02	AT	180.00	DEGREES
MINIMUM T/A=	-1.4382E+03	AT	60.00	DEGREES
MAXIMUM M(FORCE-LG)=	9.0956E+03	AT	60.00	DEGREES
MINIMUM M(FORCE-LG)=	-6.9897E+03	AT	105.00	DEGREES

RINTOUT OF MAXIMUM AND MINIMUM (AXIAL + BENDING) STRESSES FOR MOMENT RMS OF C1 AND C2 - VALUES TAKEN FROM BOTH OF THE TABLES ABOVE

MAXIMUM STRESS (C1)=	5.0845E+03	AT	59.99	DEGREES
MINIMUM STRESS (C1)=	-4.4110E+03	AT	100.00	DEGREES
MAXIMUM STRESS (C2)=	4.2838E+03	AT	105.00	DEGREES
MINIMUM STRESS (C2)=	-8.0610E+03	AT	60.00	DEGREES

OP [SYSIN]

DDG 4/5/94

DESIGN RMG SUPPORT LUGS

MAX - VERTICAL LOAD = 4892# (VERT. DL + VERT STAB. + LAT. EQ)

DESIGN LUG AS A CANTILEVER COLUMN.

$$L = 11''$$

TRY 4" x 3/8" PLATE

$$A = 4(.375) = 1.5 \text{ in}^2 \quad I_{\min} = \frac{(.375)^3(4)}{12} = 0.0176 \text{ in}^4$$

$$r = \sqrt{I/A} = 0.108'' \quad \text{LET } K = 2.1$$

$$KL/r = \frac{(2.1)(11'')}{.108''} = 214 \quad \text{TOO LARGE}$$

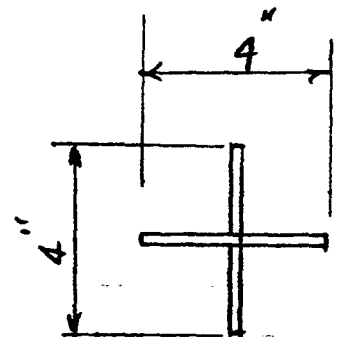
USE 3/8" STIFFENERS

$$A = [4 + (4 \cdot .375)] (.375'') = 2.86 \text{ in}^2$$

$$I = \frac{(4)^3(.375)}{12} + \frac{(.375)^3(3.625)}{12} = 2.016 \text{ in}^4$$

$$r = \sqrt{I/A} = 0.840''$$

$$KL/r = \frac{(2.1)(11'')}{.840} = 27.5$$



SUBJECT	OFFICE CBI NOE		REVISION		REFERENCE NO. 930212
	MADE BY MRS	CHKD BY DDG	MADE BY	CHKD BY	SHT 20 OF 48
	DATE 2/22/94	DATE 4/5/94	DATE	DATE	10.22

(AISC E2)

$$C_c = \sqrt{\frac{2\pi^2 E}{F_y}}$$

$$E = 27,000,000 \text{ PSI} \quad @ 300^\circ\text{F}$$

$$F_y = 19,200 \text{ PSI} \quad @ 300^\circ\text{F}$$

} ASME SEC. VIII, DIV. 2

$$C_c = 166.6$$

$$KL/r = 27.5$$

$$F_a = \frac{\left[1 - \frac{(KL/r)^2}{2C_c^2}\right] F_y}{\frac{5}{8} + \frac{3(KL/r)}{8C_c} - \frac{(KL/r)^3}{8C_c^3}}$$

$$F_a = 10,960 \text{ PSI}$$

$$f_a = \frac{4.892^{\text{th}}}{2.86 \text{ in}^2} = 1,710 \text{ PSI} < F_a \quad \text{OK}$$

SUBJECT	OFFICE CBI NOE		REVISION		REFERENCE NO. 930212
	MADE BY MRS	CHKD BY DDG	MADE BY	CHKD BY	SHT 23 OF 48
	DATE 2/22/94	DATE 4/5/94	DATE	DATE	10.23

DESIGN COLUMN SUPPORT

MAX. VERTICAL LOAD = 4,892[#] (VERT. DL + VERT. STABILITY + WIND BR)

$$L = 10'' \quad K = 2.1$$

TRY 2x2x³/₁₆ TUBE

$$A = 1.27 \text{ IN}^2 \quad I = 0.668'' \quad t = 0.726''$$

$$KL/r = \frac{2.1(10)}{.726} = 29$$

(AISC E2)

$$C_c = 166.6 \quad \frac{KL/r}{C_c} = 0.174$$

$$F_a = \frac{\left[1 - \frac{(0.174)^2}{2}\right] 19,200 \text{ PSI}}{\frac{5}{3} + \frac{3}{8}(0.174) - \frac{(0.174)^3}{8}} = 10,922 \text{ PSI}$$

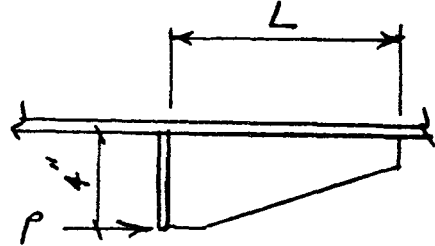
$$f_a = \frac{4892^{\#}}{1.27 \text{ IN}^2} = 3852 \text{ PSI} < F_a \quad \circ \circ \quad \text{OK}$$

SUBJECT	OFFICE CBI <u>NOE</u>		REVISION		REFERENCE NO. 93021✓
	MADE BY MRS	CHKD BY DDG	MADE BY	CHKD BY	SHT <u>24</u> OF <u>48</u>
	DATE 2/2/94	DATE 4/5/94	DATE	DATE	10.24

LATERAL RESTRAINT

CHECK SHELL FOR LONGITUDINAL BAKE OUT FORCE

$$P = 5,300 \#$$



TO PREVENT BENDING OF RING AND OVER STRESSING THE TUBE WALL A LONGITUDINAL GUSSET IS WELDED TO THE TUBE & SUPPORT RING.

DESIGN GUSSET AS A BRACKET ON A CYLINDRICAL SHELL

REF: CBI STD. 9108-7 "BRACKET SUPPORTS ON CYLINDERS"

CHECK TUBE WALL FOR LONGITUDINAL BENDING

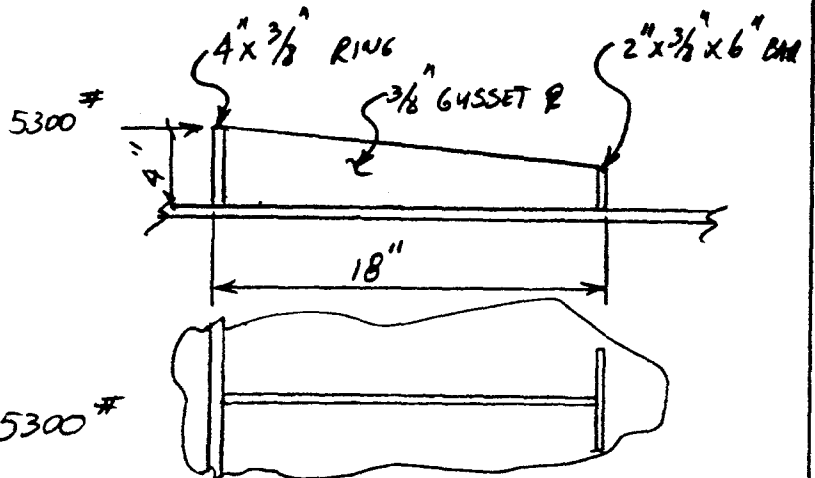
MAXIMUM CALCULATED LONGITUDINAL STRESS IN WALL

$$\sigma = 1533 \text{ PSI} + 329 \text{ PSI} = 1862 \text{ PSI}$$

↑ PER WALL CALCS. — ADDITIONAL BENDING DUE TO LONG. BAKE OUT FORCE

SUBJECT	CBI	OFFICE	REVISION	REFERENCE NO.
	NOE			930212
	MADE BY	CHKD BY	MADE BY	CHKD BY
MRS	DIG			SHT 25 OF 48
DATE	DATE	DATE	DATE	10.25
2/22/94	4/5/94			

USE AN 18" LONG LUG WITH A 6" WIDE PLATE AT THE END



LONGITUDINAL FORCE (P) = 5300 #

MOMENT ON TUBE WALL (M₁) = 4" (5300 #) = 21,200 IN-LBS

ALLOWABLE LONGITUDINAL COMPRESSIVE STRESS (σ_A) = 5800 PSI
(PER BEAM TUBE CALCS)

DESIGN LUG AS A BRACKET ON A CYLINDRICAL SHELL

REF. CBI STD. 9108-7 "BRACKET SUPPORTS ON CYLINDERS"

SUBJECT	OFFICE CBI NOE		REVISION		REFERENCE NO. 930212
	MADE BY MRS	CHKD BY DDG	MADE BY	CHKD BY	SHT 26 OF 48
	DATE 2/22/94	DATE 4/5/94	DATE	DATE	10.26

$t = 0.127$ in. Shell thickness
 $R = 24.4385$ in. Shell radius
 $L = 18$ in. Length of lug
 $t_l = 6$ in. Lug thickness
 $t_w = 0.125$ in. Weld size
 $W = 6.25$ in. $t_l + 2(t_w)$
 $P = 5300$ lbs. Vertical load
 $M_{act} = 21200$ in-lbs Longitudinal moment on shell from lug
 $Q_{act} = 0$ lbs. Radial load on shell from lug
 $SL = 0$ psi Longitudinal shell stress due to internal pressure.
 $SH = 0$ psi Circumferential shell stress due to internal pressure.
 $Slc = 1862$ psi Longitudinal shell compression stress.

ALLOWABLE STRESS (S_a) = 14400 psi = 1.25(.6F_y)

$$= 1.285/(Rt)^{.5} = 0.729397$$

$$M_1 = 1390(t^2) [6 + 6BL + (B^2)(L^2)] W = 20754 \text{ in-lbs}$$

$$M_2 = 2520(t^2) [6 + 6BL + (B^2)(L^2)] W = 37627 \text{ in-lbs}$$

$$Q_1 = 8333(t^2) WB (2 + BL) = 5339 \text{ lbs.}$$

$$Q_2 = 15150(t^2) WB (2 + BL) = 9707 \text{ lbs.}$$

$$M_a/M_1 + Q_a/Q_1 + SL/S_a = 1.0 \leq 1.0$$

$$M_a/M_2 + Q_a/Q_2 + SH/S_a = 0.56 \leq 1.0$$

ALLOWABLE SHELL COMPRESSION (S_{ca}) = 1,800,000 (t/R) = 9354 psi

$$S_c = \frac{P}{[1.154(L) + W](t)} + Slc = 3406 \text{ psi}$$

SUBJECT

MADE BY: *mrs*

R
E
V

MADE BY:

REFERENCE NO.

CHKD BY: *DDG*

CHKD BY:

DATE: *2/2/94*

DATE:

SHT *27* OF *48*

HORIZONTAL RESTRAINT DESIGN

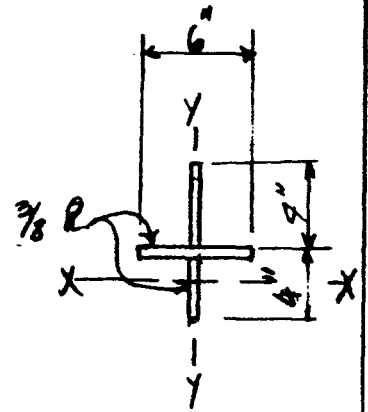
$F = 5300^{\#}$ LONGITUDINAL BAKE OUT

$F = 1730^{\#}$ LATERAL SEISMIC

MOMENT @ BASE OF RESTRAINT

$$M_{LONG.} = 4''(5300^{\#}) = 21,200^{\#-IN}$$

$$M_{LAT} = 4''(1730^{\#}) = 6,920^{\#-IN}$$



$$A = (6 + 8 \cdot .375)(.375) = 5.11 \text{ IN}^2$$

$$I_x = 16.02 \text{ IN}^4$$

$$I_y = 6.8 \text{ IN}^4$$

$$S_{MN} = 4.01 \text{ IN}^3$$

$$S_{MY} = 2.26 \text{ IN}^3$$

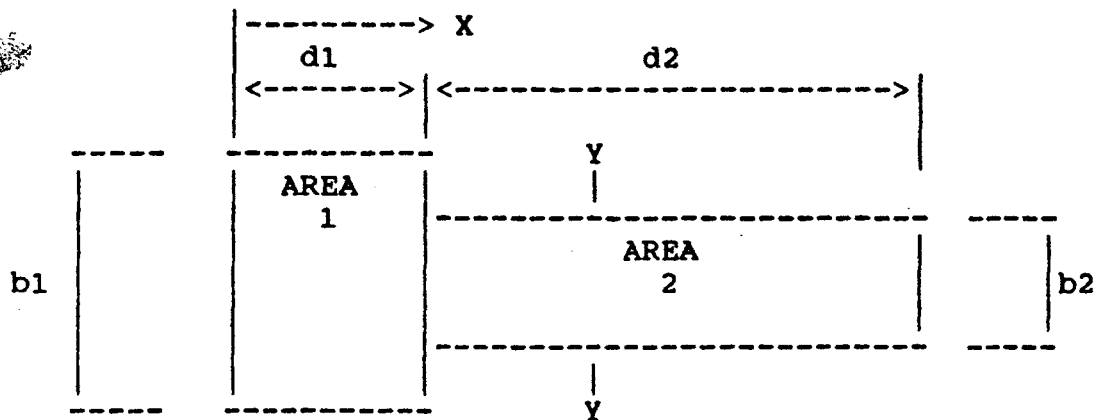
$$\sigma_{bx} = \frac{M}{S} = \frac{21,200^{\#}}{4.01 \text{ IN}^3} = 5,286 \text{ PSI} < \sigma_{ALLOW} \approx \text{OK}$$

$$\sigma_{ALLOW} = .6(F_y) = .6(19,200) = 11,520 \text{ PSI}$$

$$\sigma_{by} = \frac{M_{LAT}}{S} = \frac{6,920}{2.26} = 3,062 \text{ PSI} < \sigma_{ALLOW} \approx \text{OK}$$

SUBJECT	OFFICE CEI <u>LoE</u>		REVISION		REFERENCE NO. 930212
	MADE BY MRS	CHKD BY DIG	MADE BY	CHKD BY	SHT <u>28</u> OF <u>49</u>
	DATE 2/22/94	DATE 4/6/94	DATE	DATE	10.29

MOMENT OF INERTIA CALCULATION



	1	2	3	4	5	6	7
d	3.8125	0.375	3.8125				
b	0.375	6	0.375				
X	1.9063	4.0000	6.0938	8.0000	8.0000	8.0000	8.0000
THETA	0	0	0	0	0	0	0

	AREA	X	AX	AX ²	Io
1	1.430	1.906	2.73	5.20	1.73
2	2.250	4.000	9.00	36.00	0.03
3	1.430	6.094	8.71	53.09	1.73
4	0.000	8.000	0.00	0.00	0.00
5	0.000	8.000	0.00	0.00	0.00
6	0.000	8.000	0.00	0.00	0.00
7	0.000	8.000	0.00	0.00	0.00
	5.109		20.44		97.77

TOTAL DEPTH = 8.0000 in.

CENTROID (\bar{X}) = $\text{SUM}(AX)/\text{SUM}(AREA) = 4.000$ in.

$I(\text{total}) = [\text{SUM}(AX^2) + \text{SUM}(Io)] - (\text{AREA})(\bar{X})^2 = 16.02$ in.⁴

$C1 = \bar{X} = 4.000$ in. $C2 = \text{DEPTH} - \bar{X} = 4.000$ in.

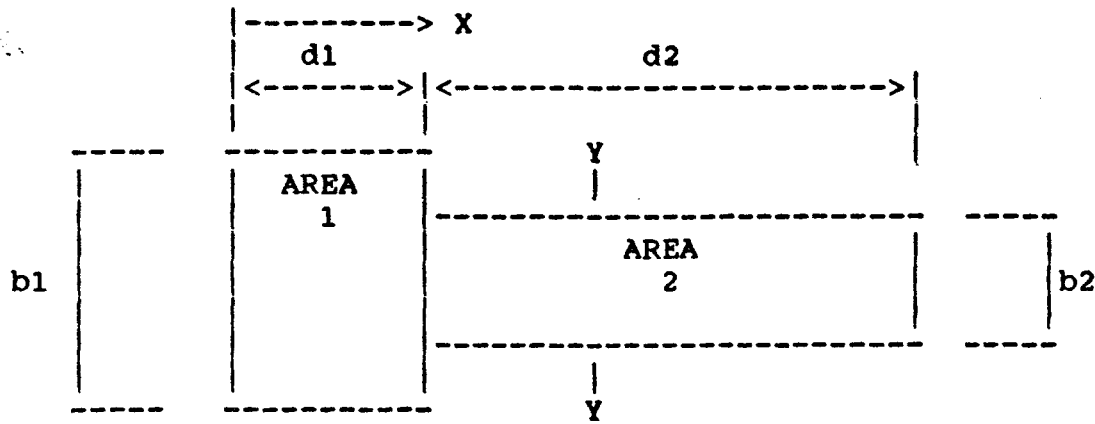
$Sy1 = I/C1 = 4.01$ in.³ $Sy2 = I/C2 = 4.01$ in.³

Radius of gyration (r) = $(I/A)^{1/2} = 1.771$ in.

Torsional constant (J) = in⁴

SUBJECT: LIGO BEAM TUBE
 MADE BY: MRS R
 HORIZONTAL RESTRAINT
 CHECKED BY: DDG
 DATE: 2/22/94
 MADE BY: E
 CHECKED BY: V
 DATE:
 REFERENCE NO.: 930212
 SHT 29 OF 48

MOMENT OF INERTIA CALCULATION



	1	2	3	4	5	6	7
d	2.8125	0.375	2.8125				
b	0.375	8	0.375				
X	1.4063	3.0000	4.5938	6.0000	6.0000	6.0000	6.0000
THETA	0	0	0	0	0	0	0

	AREA	X	AX	AX ²	Io
1	1.055	1.406	1.48	2.09	0.70
2	3.000	3.000	9.00	27.00	0.04
3	1.055	4.594	4.84	22.26	0.70
4	0.000	6.000	0.00	0.00	0.00
5	0.000	6.000	0.00	0.00	0.00
6	0.000	6.000	0.00	0.00	0.00
7	0.000	6.000	0.00	0.00	0.00
	5.109		15.33		52.77

TOTAL DEPTH = 6.0000 in.

CENTROID (\bar{X}) = SUM(AX)/SUM(AREA) = 3.000 in.

I(total) = [SUM(AX²) + SUM(Io)] - (AREA)(\bar{X})² = 6.78 in.⁴

C1 = \bar{X} = 3.000 in. C2 = DEPTH - \bar{X} = 3.000 in.

Sy1 = I/C1 = 2.26 in.³ Sy2 = I/C2 = 2.26 in.³

Radius of gyration (r) = (I/A)^{1/2} = 1.152 in.

Torsional constant (J) = in⁴

 SUBJECT: LIGO BEAM TUBE
 SECTION PROPERTIES ABOUT Y-Y
 HORIZONTAL RESTRAINT
 MADE BY: MRS R
 CHECKED BY: DDG V
 DATE: 2/22/94
 MADE BY:
 CHECKED BY:
 DATE:
 REFERENCE NO.: 93021
 SHT 30 OF 49

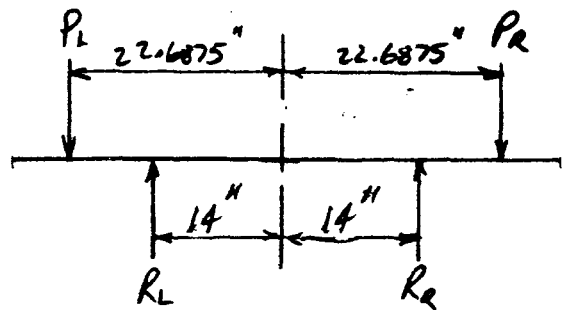
DESIGN HORIZONTAL SUPPORT BEAM

LOAD COMBINATION #1

DL + VERT. STABILITY

$$P_L = P_R = 7600 \# / 2 = 3800 \#$$

$$R_L = R_R = 7600 \# / 2 = 3800 \#$$



$$M_x = (3800 \#)(22.6875 - 14) = 33,013 \text{ IN-LBS}$$

$$T_{allow} = .6 F_y = .6(36,000 \text{ PSI}) = 21,600 \text{ PSI}$$

$$S_{req'd} = \frac{M}{T_{allow}} = \frac{33,013}{21,600} = 1.53 \text{ IN}^3$$

TRY STRUCTURAL TUBE 4x4x 3/16

SUBJECT	OFFICE CBI NOE		REVISION		REFERENCE NO. 930212
	MADE BY MRS	CHKD BY DDG	MADE BY	CHKD BY	SHT. 31 OF 48
	DATE 2/23/94	DATE 4/6/94	DATE	DATE	10.31

LOAD COMBINATION #3

$$P_L = 4892^{\#}$$

$$P_R = 2708^{\#}$$

$$\Sigma M_{R_L} = 0$$

$$8''(1730^{\#}) + (36.6875'')(4892^{\#}) - 8.6875''(2708^{\#}) - 28''(R_L) = 0$$

$$R_L = 6064^{\#}$$

$$R_R = 4892^{\#} + 2708^{\#} - 6064^{\#} = 1536^{\#}$$

$$M_{max} = (4892^{\#})(22.6875'' - 14'') = 42,500 \text{ IN-LBS}$$

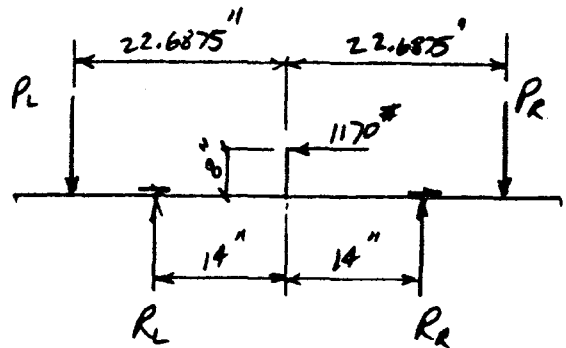
$$\text{AXIAL LOAD} = 1730^{\#}$$

$$S_{REQ'D} = \frac{42,500 \text{ IN-LBS}}{21,600 (1.33)} = 1.48 \text{ IN}^3$$

$$\text{TRY } 4 \times 4 \times 3/16 \text{ TUBE } \quad A = 2.77 \text{ IN}^2 \quad S = 3.30 \text{ IN}^3$$

$$f = \frac{P}{A} + \frac{M}{S} = \frac{1730^{\#}}{2.77 \text{ IN}^2} + \frac{42,500 \text{ IN-LBS}}{3.30 \text{ IN}^3} = 625 + 12,879 = 13,504 \text{ PSI}$$

$$< 1.33(21,600) = 28,800 \text{ PSI}$$

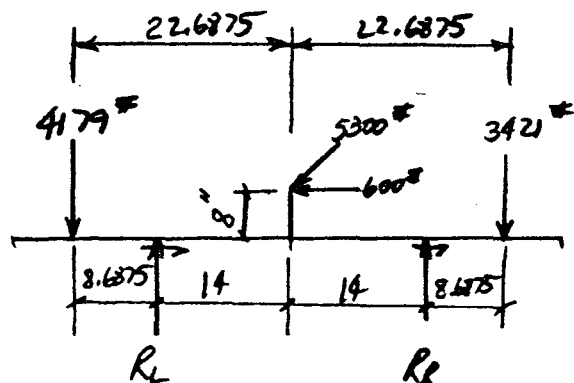


SUBJECT	OFFICE CBI NOE		REVISION		REFERENCE NO. 930212
	MADE BY MRS	CHKD BY DDG	MADE BY	CHKD BY	SHT ³² OF 48
	DATE 2/23/94	DATE 4/6/94	DATE	DATE	11.32

LOAD COMBINATION #4

MOMENTS ABOUT X-X AXIS

$$\Sigma M_{R_L} = 0$$



$$(4179^*)(8.6875'') - 28'' R_L + 8''(600^*) - (8.6875'')(3421^*) = 0$$

$$R_L = 4,586^*$$

$$R_R = 4179 + 3421 - 4586 = 3014^*$$

$$M_{MAX} = 4179^*(8.6875'') = 36,305 \text{ IN-LBS}$$

TRY 4x4x 3/8 TUBE A = 5.08 in² S = 5.35 in³

$$f_{bx} = \frac{36,305}{5.35 \text{ in}^3} = 6,786 \text{ PSI}$$

MOMENT ABOUT Y-Y AXIS

$$M_y = \frac{PL}{4} = \frac{(5300^*)(28'')}{4} = 37,100 \text{ IN-LBS}$$

$$f_{by} = \frac{37,100 \text{ IN-LBS}}{5.35 \text{ in}^3} = 6,935 \text{ PSI}$$

SUBJECT	OFFICE CBI NOE		REVISION		REFERENCE NO. 930212
	MADE BY MRS	CHKD BY DDG	MADE BY	CHKD BY	SHT 23 OF 48
	DATE 2/23/94	DATE 4/6/94	DATE	DATE	10.33

AXIAL LOAD

$$P = 600 \#$$

$$f_a = \frac{P}{A} = \frac{600 \#}{5.081 \text{ in}^2} = 118 \text{ PSI}$$

COMBINED STRESS

$$\frac{f_a}{F_a} + \frac{f_{bx}}{F_{bx}} + \frac{f_{by}}{F_{by}} \leq 1.0$$

$$\frac{118}{21,600} + \frac{6786}{21,600} + \frac{6935}{21,600} = 0.641 < 1.0 \quad \text{OK}$$

4 x 4 x 3/8 STRUCT. TUBE OK

SUBJECT	OFFICE CBI <u>NOE</u>		REVISION		REFERENCE NO. <u>930212</u>
	MADE BY <u>MRS</u>	CHKD BY <u>DDG</u>	MADE BY	CHKD BY	SHT <u>34</u> OF <u>48</u>
	DATE <u>2/23/94</u>	DATE <u>4/6/94</u>	DATE	DATE	<u>10.34</u>

CHECK SHEAR STRESSES DUE TO TORQUE

$$\text{TORQUE (T)} = \frac{8''(5300\#)}{2} = 21,200 \text{ IN-LBS}$$

$$\tau_{\text{TOT}} = \tau_T + \tau_V$$

$$\text{MAX HORIZONTAL SHEAR} = \frac{5300\#}{2} = 2650\#$$

$$\text{MAX VERTICAL SHEAR} = 4179\#$$

TRY 4" x 4" x 3/8" TUBE

$$\text{AREA} = 5.08 \text{ IN}^2 \quad A_V = 2(4'')(0.375'') = 3 \text{ IN}^2$$

$$\text{TORSIONAL RESISTANCE (R)} = \frac{2b^2d^2}{\frac{b}{E} + \frac{d}{E}}$$

$$b = d = 4 - 0.375 = 3.625''$$

$$R = 17.863 \text{ IN}^4$$

[A] AREA ENCLOSED BY MEAN DIMENSIONS

$$[A] = bd = 13.14 \text{ IN}^2$$

SUBJECT	OFFICE CBI DOE		REVISION		REFERENCE NO. 930212
	MADE BY MRS	CHKD BY DDG	MADE BY	CHKD BY	SHT 35 OF 48
	DATE 2/23/94	DATE 4/6/94	DATE	DATE	11.35

$$\tau_v = \frac{V}{A_v} = \frac{4179^{\#}}{3 \text{ IN}^2} = 1,393 \text{ PSI}$$

$$\tau_T = \frac{T}{2[A]t} = \frac{21,200 \text{ IN-LBS}}{2(13.14 \text{ IN}^2)(.375)} = 2,151 \text{ PSI}$$

$$\tau_{\text{TOT.}} = 2,151 \text{ PSI} + 1,393 \text{ PSI} = 3,544 \text{ PSI}$$

MAX. ALLOWABLE SHEAR STRESS = .4F_y = .4(36,000) = 14,400 PSI
 4x4x³/₈ TUBE OK

ROTATION

$$\theta = \frac{TL}{E_s R}$$

$$E_s = 12,000,000 \text{ PSI}$$

$$\theta = \frac{(21,200)(32^{\#})}{(12,000,000)(17.863 \text{ IN}^4)} = 0.00316 \text{ RAD} = 0.1813^{\circ}$$

DISPLACEMENT @ TOP OF LATERAL RESTRAINT.

$$8^{\#}(.00316) = 0.0253^{\#}$$

SUBJECT	OFFICE CBI NOE		REVISION		REFERENCE NO. 93022
	MADE BY MRS	CHKD BY DDG	MADE BY	CHKD BY	SHT 36 OF 48
	DATE 2/23/94	DATE 4/6/94	DATE	DATE	11.76

DESIGN SUPPORT BRACKETS

LOAD COMBINATION # 4 (DL + AXIAL BACKOUT + VERT STAB. + LAT. STAB.)

MAX. VERT. LOAD = 4586#

T = 21,200 IN-LBS

MAX TRANS. LOAD = 600#

MAX. LOAG. LOAD = $\frac{5300\#}{2} = 2650\#$

$F_u = \frac{2650\#}{2} + \frac{2"(4586\#)}{4.75"} + \frac{21,200\text{IN-LBS}}{4.75"}$

$F_u = 1325\# + 1931\# + 4463\# = 7,719\#$

$F_L = 1325 - 1931 - 4463\# = -5069\#$

SIZE BOLT

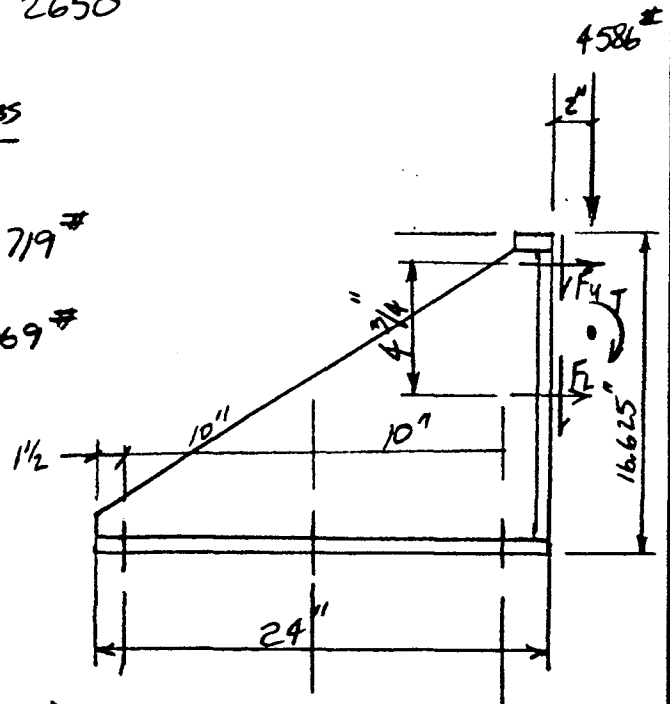
$F_u = 7,719\#$

FOR $\frac{3}{4}" \phi$ A307 BOLTS
ALLOWABLE TENSILE = 8,800#

(TABLE I-A, AISC, Pg 4-3)

BOLT SHEAR = $\frac{4586\#}{2} = 2300\#$

SINGLE SHEAR ALLOWABLE FOR $\frac{3}{4}" \phi = 4,400\#$ (TABLE I-D, AISC, Pg 4-5)



SUBJECT	OFFICE CBI DOE		REVISION		REFERENCE NO. 93022
	MADE BY MRS	CHKD BY DDG	MADE BY	CHKD BY	SHT 37 OF 48
	DATE 2/23/94	DATE 4/6/94	DATE	DATE	10.37

COMBINED TENSION & SHEAR

(AISC J3-5)

A307 BOLTS

$$\text{BOLT SHEAR} = 2300^{\#} \quad \text{BOLT TENSION} = 7719^{\#}$$

ASSUME SINGLE SHEAR

TRY 1" ϕ BOLTS @ THREADS IN SHEAR PLANE.

$$A_k = .551 \text{ IN}^2 \quad A_t = 0.606 \text{ IN}^2 \quad A_D = .785 \text{ IN}^2$$

$$F_v = \frac{2300^{\#}}{.563 \text{ IN}^2} = 4,124 \text{ PSI}$$

$$F_t = 26,000 - 1.8 F_v = 18,486 \text{ PSI}$$

$$f_t = \frac{7719}{.606} = 12,738 \text{ PSI} < F_t \quad \infty \text{ OK}$$

(AISC J3-6)

$$f_t = 12,738 \text{ PSI} \quad T_b = 0.7 F_u A_T = .7(60)(.606) = 25,452^{\#}$$

$$A_b = .785 \text{ IN}^2$$

$$F_v = (10,000 \text{ PSI}) \left(1 - \frac{P_t A_b}{T_b}\right) = 6,069 \text{ PSI}$$

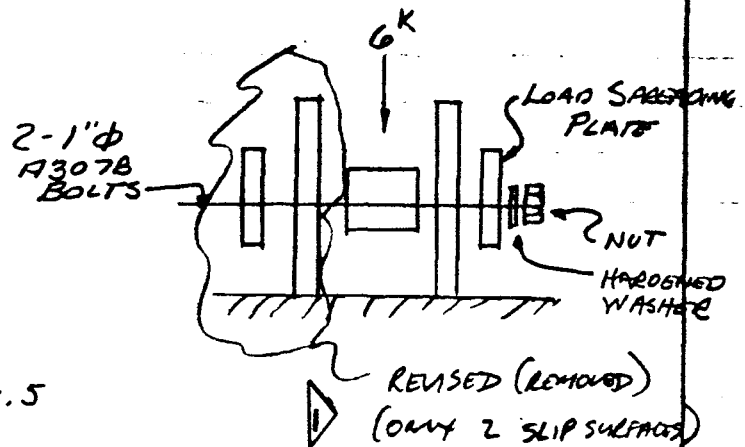
$$P_v = 4,085 \text{ PSI} < F_v \quad \infty \text{ OK}$$

USE 1" ϕ A307 BOLTS

SUBJECT	OFFICE NOE		REVISION		REFERENCE NO.
	MADE BY mrs	CHKD BY DDG	MADE BY	CHKD BY	93022
	DATE 2/23/94	DATE 4/6/94	DATE	DATE	SHT 38 OF 48
				10.38	

DESIGN FRICTION CONNECTION

LOADS ~
 6 K VERT
 1.2 K LATERAL



$$\text{LOAD ON JOINT} = [6^2 + 1.2^2]^{.5}$$

$$\approx 6.2 \text{ K}$$

$R_s = N_s F_p N M$ WHERE $R_s = \text{SLIP RESISTANCE LOAD (lb)}$
 $N_s = \text{FRICTION COEFF}$
 $F_p = \text{LOAD PER BOLT}$
 $N = \text{NUMBER OF BOLTS}$
 $M = \text{NUMBER OF SLIP SURFACES}$

Now
 $6200 = (5)(F_p)(2)(2)$

$F_p = 3100 \#$

USE A307B BOLTS @ 20 KSI DESIGN STRESS ~ 1" x 8UNC
 PRELOAD AT 20 KSI = 12.12 KIP
 TORQUE TO ACHIEVE 20 KSI @ 200 FT-LB

SUMMARY

USE A307B BOLTS TORQUED TO 200 FT-LB. USE GOOD LUBRICANT SUCH AS LOCTITE NICKEL ANTI-SEIZE 771 ON THREADS AND BEARING SURFACE OF NUT. SLIP PLANES TO BE CLEAN AND DRY

NOTE: THIS CALCULATION EXCEEDS $F_p \times 7.8$ TIMES. TORQUE, COEFF FRICTION & /OR NUMBER OF SLIP PLANES CAN BE REDUCED WHILE STILL MEETING THE LOAD REQUIREMENT

SUBJECT L160 FRICTION LOADING ON FIXED SUPPORT	OFFICE CBI EDA		REVISION 1		REFERENCE NO. 920212
	MADE BY FAF	CHKD BY DDG	MADE BY MRS	CHKD BY DDG	SHT 39 OF 48
	DATE 2/23/94	DATE 4/6/94	DATE 3/7/94	DATE 4/6/94	10.39

DESIGN VERTICAL GUIDE PLATES

$$M = 2''(6060) = 12,120 \text{ IN-LBS}$$

$$\frac{12,120''}{2} = 6060''$$

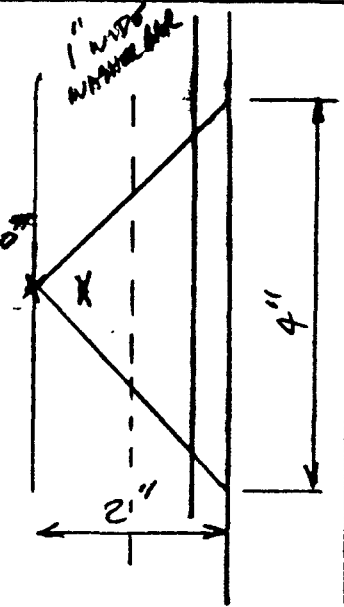
$$\sigma_{\text{ALLOW}} = .75(F_y) = .75(36,000) = 27,000 \text{ PSI}$$

$$S_{\text{REQ'D}} = \frac{M}{\sigma} = \frac{12,120}{27,000} = 0.449 \text{ IN}^3$$

$$S = \frac{t^2 b}{6} \Rightarrow 0.449 = \frac{t^2(4)}{6}$$

$$t = 0.821''$$

USE 1" PLATE



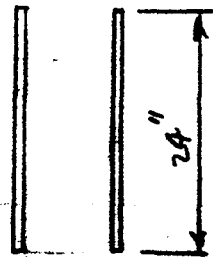
DESIGN GUSSET PLATES

@ BASE $M = 21,200 \text{ IN-LBS} + 2650''(16625 - 4.75 \frac{1}{2})$
 $M = 59,000 \text{ IN-LBS}$

$$S_{\text{REQ'D}} = \frac{M}{\sigma F_y} = \frac{59,000}{.6(36,000)} = 2.73 \text{ IN}^3$$

$$2.73 = \frac{t^2(24)}{6} (2)$$

$$t = .584'' \quad \text{USE } \frac{5}{8}'' \text{ PLATE}$$



SUBJECT	OFFICE CBI NOE		REVISION		REFERENCE NO. 930212
	MADE BY MRS	CHKD BY DDG	MADE BY	CHKD BY	SHT 40 OF 48
	DATE 2/23/94	DATE 4/6/94	DATE	DATE	10.40

ANCHORAGE DESIGN

LOAD COMB. #4

$$V = 4586 \#$$

$$H = \frac{5300 \#}{2} = 2650 \#$$

$$T = 21,200 \text{ IN-LBS}$$

$$\sum M @ A = 0$$

$$25" (4586 \#) + 21,200 \text{ IN-LBS} + 14.25" (2650 \#) - 2P(22") = 0$$

$$P = 1600 \# / \text{BOLT}$$

$$\text{MAXIMUM SHEAR (S)} = \frac{2650 \#}{6 \text{ BOLTS}} = 442 \# / \text{BOLT}$$

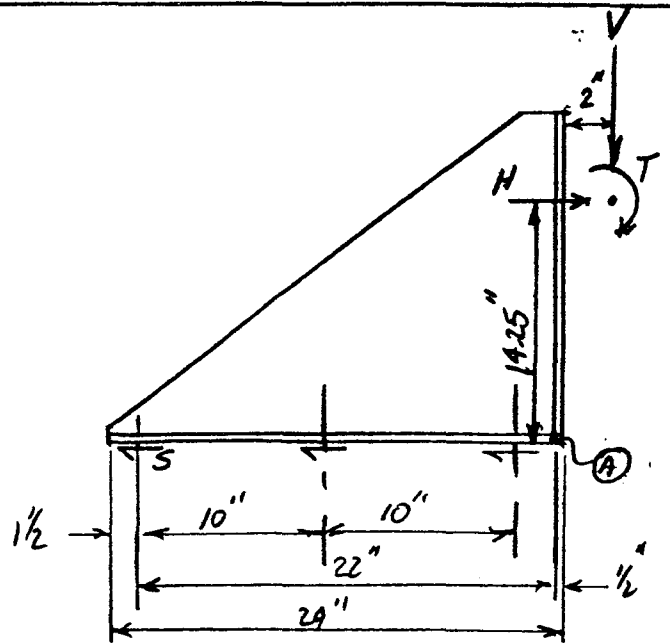
TRY $\frac{5}{8} \phi$ HILTI EXPANSION ANCHORS KB-II

ASSUME 4" THICK 3000# CONCRETE

$$P_{\text{ALLOW}} = 1750 \# > P \quad \text{OK}$$

(ASSUMES UBC SPECIAL INSPECTION)

$$S_{\text{ALLOW}} = 2875 \# > S \quad \text{OK}$$



SUBJECT

OFFICE CBI DOE		REVISION		REFERENCE NO. 930212
MADE BY MRS	CHKD BY DDG	MADE BY	CHKD BY	SHT. 41 OF 48
DATE 2/23/94	DATE 4/6/94	DATE	DATE	10.41

DESIGN TUBE BEARING PLATE

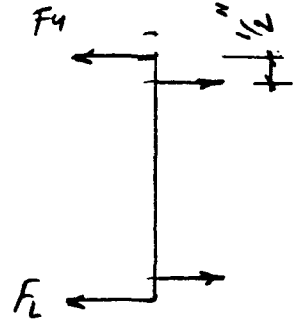
$$\text{BOLT PRELOAD} = 12.2 \text{ K}$$

$$\text{MOMENT} = 12.2 \text{ K} (0.5") = 6,100 \text{ IN-LBS}$$

TRY 1" X 3" BAR

$$S = \frac{bt^2}{6} = \frac{3(1)^2}{6} = .5$$

$$f_b = \frac{M}{S} = \frac{6,100 \text{ IN-LBS}}{.5"} = 12,200 \text{ PSI} < .75F_y = 27,000 \text{ PSI}$$



SUBJECT	OFFICE CBI DDG		REVISION		REFERENCE NO. 930212
	MADE BY MPS	CHKD BY DDG	MADE BY	CHKD BY	SHT 42 OF 48
	DATE 3/7/94	DATE 4/6/94	DATE	DATE	10.42

CHECK TUBE FOR CONCENTRATED LOADS

$$P_{max} = (\text{BOLT AREA}) \times (\text{BOLT YIELD STRESS})$$

$$P_{max} = (6.785 \text{ in}^2) (36,000 \text{ PSI}) = 28,275 \text{#}$$

(AISC K1.3)

$$\frac{R}{t(N+5k)} \leq 0.66F_y = 23,760 \text{ PSI} \quad 4 \times 4 \times \frac{3}{8} \text{ TUBE OK}$$

$$R = 28,275 \text{#} \quad t = 0.375 \text{''} \quad N = 3 \text{''} \quad k = 2(0.375 \text{''}) = 0.75 \text{''}$$

$$\frac{R}{t(N+5k)} = \frac{28,275 \text{#}}{0.375 \text{''} (3 \text{''} + 5(0.75 \text{''}))} = 11,170 \text{ PSI} < 0.66F_y \text{ OK}$$

(AISC K1.4)

$$R = 67.5 t_w^2 \left[1 + 3 \left(\frac{N}{d} \right) \left(\frac{t_w}{t_f} \right)^{1.5} \right] \sqrt{F_{yw} (t_f/t_w)}$$

$$R = (67.5)(0.375)^2 \left[1 + 3 \left(\frac{3}{4} \right) \left(\frac{0.375}{0.375} \right)^{1.5} \right] \sqrt{(36 \text{ KSI}) (0.375/0.375)}$$

$$R = 185 \text{K} > 28,275 \text{#} \text{ OK} \quad 4 \times 4 \times \frac{3}{8} \text{ TUBE OK}$$

SUBJECT	OFFICE CBI NOE		REVISION		REFERENCE NO. 930212
	MADE BY MRS	CHKD BY DDG	MADE BY	CHKD BY	SHT 43 OF 48
	DATE 3/7/94	DATE 4/6/94	DATE	DATE	10.43

BASE PLATE DESIGN

P_{MAX} ON 1" SLOTTED PLATE

← DUE TO ECCENTRIC LOADS (SEE A.B. CALC SHEET)

$$P_{MAX} = 2(1600^{\#}) + \frac{6064^{\#}}{2} = 6232^{\#}$$

LET ALLOWABLE BEARING ON CONCRETE = 750 PSI

$$\text{AREA REQ'D} = \frac{6232^{\#}}{750} = 8.31 \text{ IN}^2$$

BASE WIDTH = 5" MIN.

$$\text{PLATE LENGTH} = \frac{8.31 \text{ IN}^2}{5"} = 1.66'$$

CHECK PUNCHING ON 4" SLAB OF CONCRETE

(ACI 318-89 11.12)

ASSUME 1" WIDE BASE R ALL AROUND

$$\beta = \frac{7}{3} = 2.33 \quad f'_c = 3000 \text{ PSI}$$

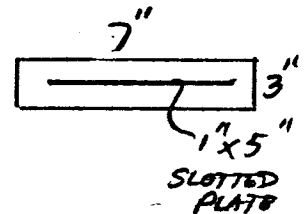
$$V_c = \left(2 + \frac{4}{\beta}\right) \sqrt{f'_c} b_o d$$

$$b_o = 2(7+3) = 20" \quad d = 4"$$

$$V_c = 3.714 \sqrt{3000} (20)(4) = 16,274^{\#}$$

$$V_n = \phi V_c = .85(16,274^{\#}) = 13,833^{\#}$$

$$U = (6232^{\#})(1.7) = 10,594^{\#} < V_n \quad \text{OK}$$



SUBJECT	OFFICE		REVISION		REFERENCE NO.
	CBI NOE				930212
	MADE BY	CHKD BY	MADE BY	CHKD BY	SHT 44 OF 48
	MRS	DDG			
DATE	DATE	DATE	DATE		
2/28/94	4/6/94			10.44	

BASE PLATE THICKNESS

VERTICAL DOWN LOAD

$$w = \frac{6232 \#}{(7")(3")} = 297 \text{ PSI}$$

$$M = \frac{w L^2 (5)}{2} = \frac{297 (1")^2 (5)}{2} = 743 \text{ IN-LBS}$$

$$S = \frac{M}{S} = \frac{6M}{t^2 b} \leq 0.75 F_y$$

$$S = \frac{6(743 \text{ IN-LBS})}{t^2 (5)} \leq 27,000 \text{ PSI}$$

$$t = 0.182"$$

PULL OUT LOAD

MAX. ALLOWABLE PULLOUT LOAD OF EXPANSION ANCHOR = 3290 #

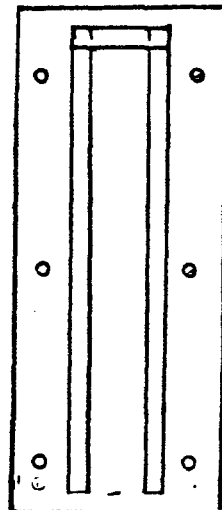
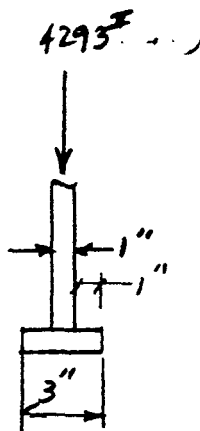
ACTUAL PULL OUT LOAD = 1341 #

$$M = 1.5" (3290 \#) = 4,935 \text{ IN-LBS}$$

ASSUME 3" OF BASE PLATE RESISTS BENDING $S = \frac{t^2 3}{6} = \frac{t^2}{2}$

$$t = \sqrt{\frac{2M}{0.75 F_y}} = \sqrt{\frac{2(4935)}{27000}} = 0.605"$$

USE $\frac{5}{8}"$ PLATE (MIN)



SUBJECT	OFFICE CBI NOE		REVISION		REFERENCE NO. 930212
	MADE BY MRS	CHKD BY DOG	MADE BY	CHKD BY	SHT 45 OF 48
	DATE 2/12/94	DATE 4/6/94	DATE	DATE	10.45

WELD DESIGN

LUGS TO RING

$$P_{max} \approx 6000 \#$$

ALLOWABLE LOAD PER INCH OF WELD FOR $\frac{1}{16}$ " WELD

$$f_{WA} = 600 \#/IN$$

ASSUME 4 WELDS @ 3" EA.

$$F_W = 6000 \# / 12" = 500 \#/IN$$

USE MIN. SIZE WELDS $\frac{3}{16}$ "

VERTICAL TUBE TO HORIZONTAL TUBE

$$P_{max} = 6000 \# \quad \text{USE } \frac{3}{16}" \text{ ALL AROUND}$$

$$F = 16" (600 \#) (3) = 28,800 \#$$

LONGITUDINAL LUG ON BEAM TUBE WALL TO LONGIT. GUSSET

$$P = 5300 \# \quad L = 18"$$

$$P/L = \frac{5300 \#}{18} = 294 \#/IN \quad \text{USE } \frac{1}{8}" \text{ WELD}$$

SUBJECT	OFFICE CBI NRE		REVISION		REFERENCE NO. 930212
	MADE BY MAS	CHKD BY DDG	MADE BY	CHKD BY	SHT 46 OF 48
	DATE 2/24/94	DATE 4/6/94	DATE	DATE	10.4b

HORIZONTAL RESTRAINT TO HORIZONTAL TUBE

$M_x = 15''(5300^{\#}) = 26,500 \text{ IN-LBS}$ $M_y = 4''(600^{\#}) = 2400 \text{ IN-LBS}$

$V = 5300^{\#}$

$V = 600^{\#}$

$L_w = (12'')/2 = 24''$

$J_w = (58.7 \text{ IN}^2)(2) = 117.3 \text{ IN}^2$

$f_{wB} = \frac{V}{L_w} - \frac{(M_x)(2'')}{J_w}$, $f_{wT} = \frac{V}{L_w} + \frac{M_x(2'')}{J_w}$

$f_{wT} = \frac{5300}{24} + \frac{(26,500)(2'')}{117.3} = 673^{\#}/\text{IN}$

$f_{wB} = \frac{5300}{24} - \frac{26,500(2'')}{117.3} = -231^{\#}/\text{IN}$

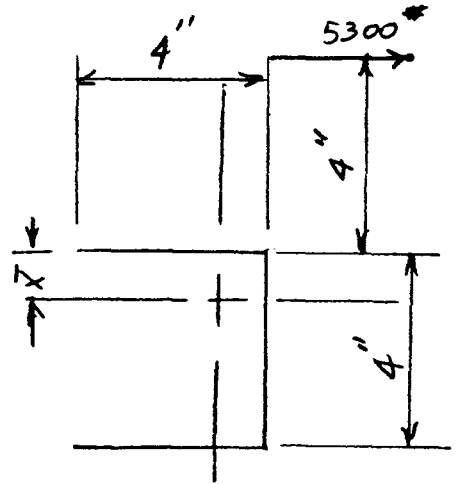
$\text{SIZE} = \frac{673^{\#}/\text{IN}}{(9600^{\#}/\text{IN}^2)(\frac{19.2}{30})} = 0.110''$

USE $\frac{3}{16}''$ FILLETS

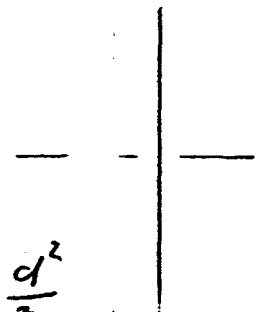
$f_{wy} = \frac{V}{L_w} + \frac{M_y}{S}$

$f_{wy} = \frac{600}{6'} + \frac{2400}{6 \text{ IN}^2} = 500^{\#}/\text{IN}$

$\frac{3}{16}''$ FILLETS OK



WELD OK



$S = \frac{d^2}{2}$
 $S = \frac{6^2}{16} = 6 \text{ IN}^2$
 $L_w = 6''$

SUBJECT	OFFICE CBI NoE		REVISION		REFERENCE NO. 930212
	MADE BY mrs	CHKD BY DDG	MADE BY	CHKD BY	SHT 47 OF 48
	DATE 2/29/94	DATE 4/6/94	DATE	DATE	10.47

DESIGN SUPPORT LUG ASSEMBLY WELDS

MAX BOLT LOAD ON ONE 1" R = 12,200#

ASSUME 4" OF R & WELD RESIST LOAD

$$f_w = \frac{12200\#}{4} = 3050\#/\text{IN}$$

$$\text{SIZE IN } \frac{1}{16}'' = \frac{3050}{600} = 5.1 \left(\frac{1}{16}''\right)$$

USE $\frac{3}{8}''$ WELD ONE SIDE

GUSSET RS TO BASE R

$$V = 5300\# / 2 = 2650\#$$

$$P = 6000\#$$

$$M = 59000 \text{ N-LBS}$$

$$\text{LAT. LOAD} = 600\#$$

$$S_w = \frac{24^2}{3} = 192$$

$$L_w = 2(24'') = 48''$$

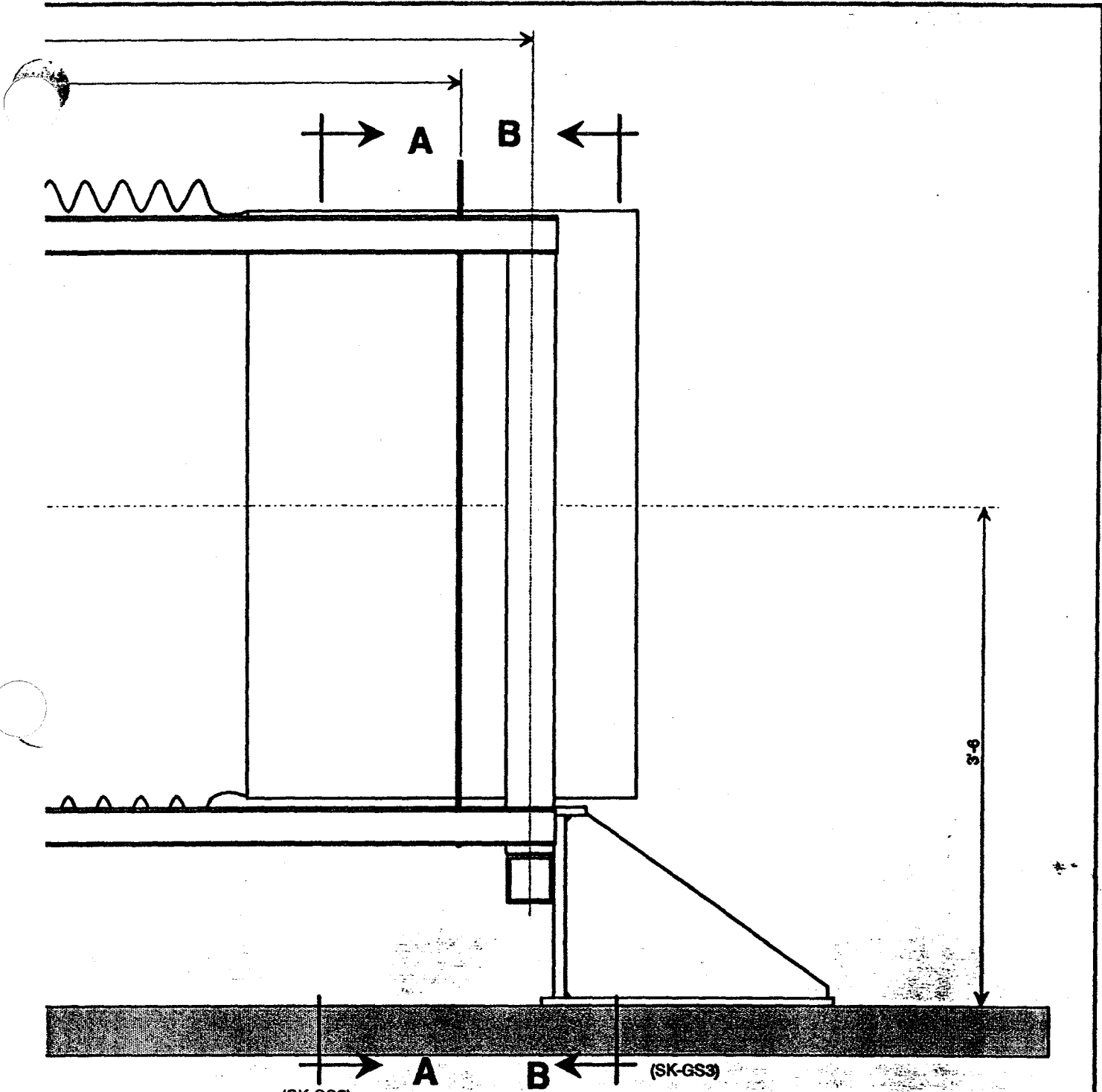
$$f_w = \sqrt{\left(\frac{P}{L_w} + \frac{M}{S_w}\right)^2 + \left(\frac{V}{L_w}\right)^2 + \left(\frac{\text{LAT LOAD}}{L_w}\right)^2}$$

$$f_w = 436\#/\text{IN}$$

$$\text{SIZE IN } \frac{1}{16}'' = \frac{436}{600} = .727$$

USE $\frac{5}{16}''$ ONE SIDE

SUBJECT	OFFICE CBI NOE		REVISION		REFERENCE NO. 930212
	MADE BY MRS	CHKD BY DIG	MADE BY	CHKD BY	SHT 48 OF 48
	DATE 2/25/94	DATE 4/6/94	DATE	DATE	10.48



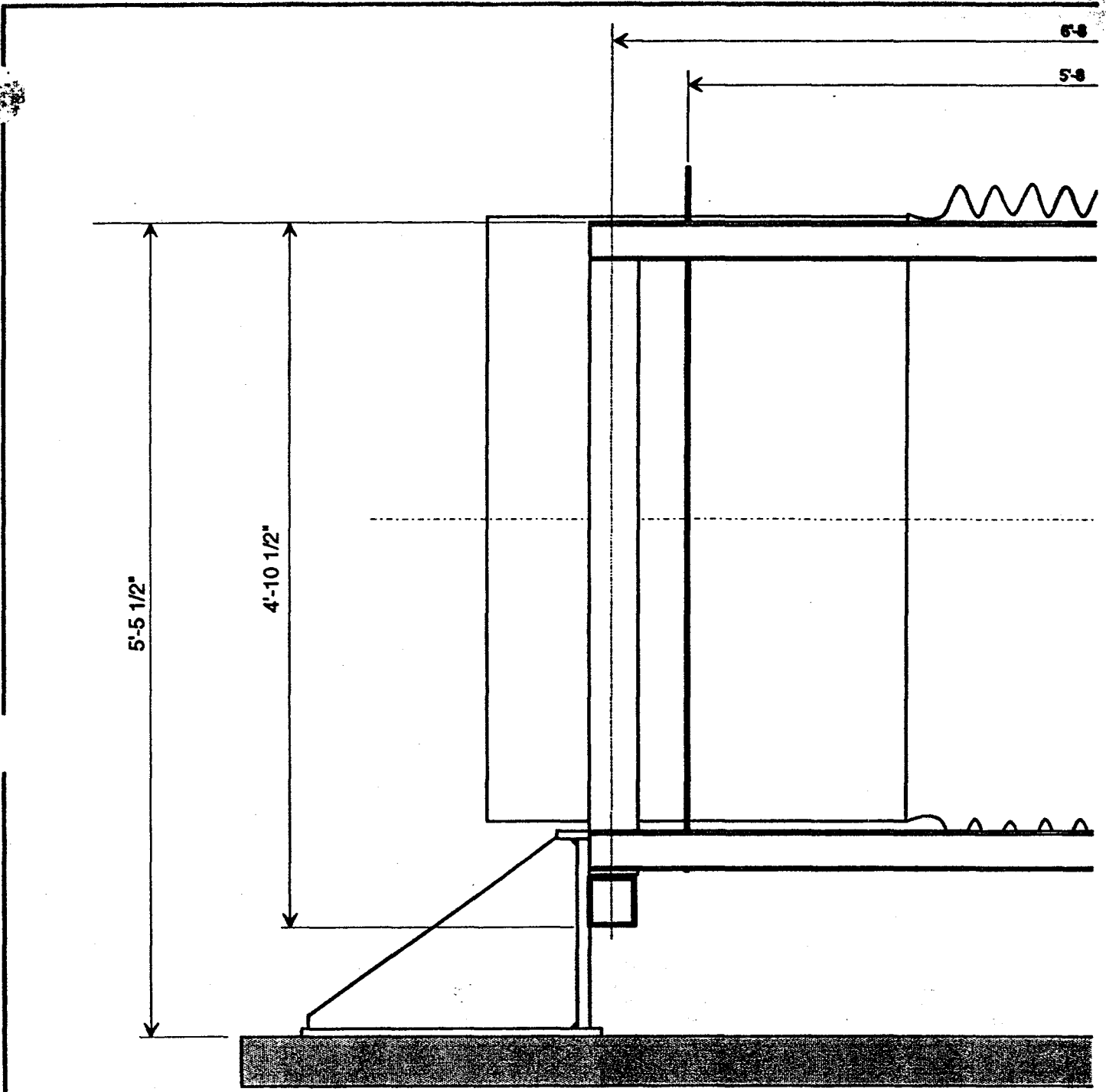
VIEW

(SK-GS2)

(SK-GS3)

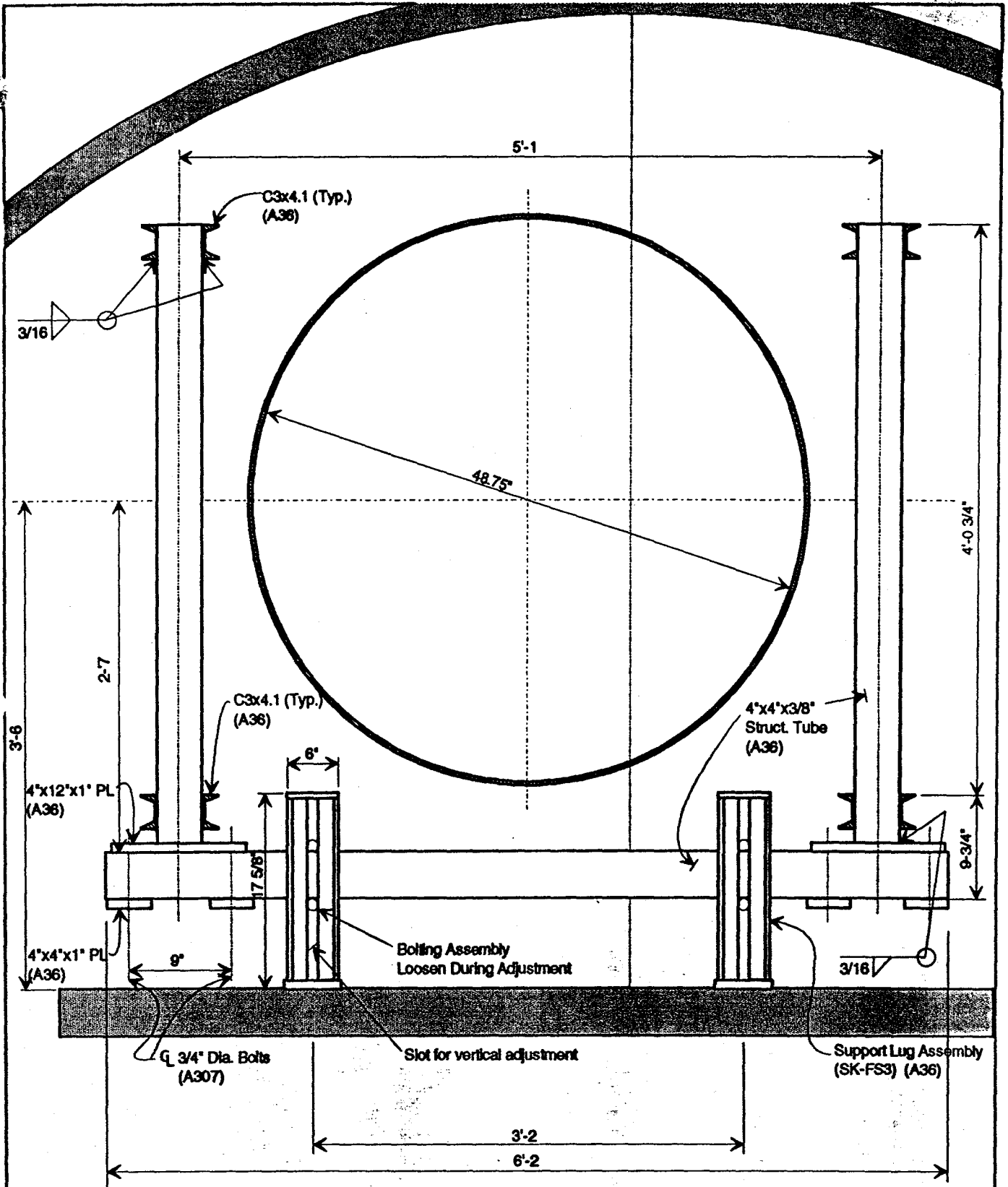
		CBI		Supplier's/Purchaser's No.
		BEAM TUBE GUIDED SUPPORT ELEVATION VIEW LIGO		
		Customer's No.		Contract No.
		By <u>MRS Chkd <u>DDG</u></u> Date <u>3/3/94</u>		<u>930212</u>
		Engineering Supervisor		Drawing No. SK-GS1
				Rev.
By	Chkd	Date	By	Date
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▷ Indicates change from previous issue



ELEVATION

SK-GS1



SECT B-B

SUBJECT

OUTER FRAME ASSEMBLY
 BEAM TUBE GUIDED SUPPORT
 LIGO

CBI OFFICE		REVISION:		REFERENCE NO.
MADE	CHK'D	MADE	CHK'D	930212
MRS	DDG	MRS	DDG	SHT 4 OF 35
DATE	DATE	DATE	DATE	SK - GS3
3/3/94	4/6/94	3/8/94	4/6/94	

Load Case	Guided Support Loads, Total	Magnitude		Direction	Comments		
1	DL (Metal Wt + Insul'n)	5,100	lbs	down	(per RISA-2D analysis for continuous beam system)		
2	Axial Seismic Load	0	lbs	axial	(assumes fixed support takes all axial loads)		
3	Lateral Seismic Load	1,130	lbs	lateral	(~ 18% * 90 pft * 65 ft), assumes guided support takes lateral loads		
4	Axial BakeOut Load	0	lbs	axial	(assumes taken by fixed support)		
5	Axial Contraction Load	0	lbs	axial	(assumes taken by fixed support)		
6	Axial Extension Load	0	lbs	axial	(assumes taken by fixed support)		
7	Vertical Stability Load	600	lbs	down	(assumed)		
8	Lateral Stability Load	600	lbs	lateral	(assumes guided support takes lateral loads)		
Comb'n	Load Combination			Vert Load	Axial Load	Lateral Load	Stress Increase
1	DL + Vertical Stability			5,700	0	0	1.000
2	DL + Axial EQ + Axial Ext'n + Vert Stability + Lat Stability			5,700	0	600	1.333
3	DL + Lat EQ + Axial Ext'n + Vert Stability + Lat Stability			5,700	0	1,730	1.333
4	DL + Axial BakeOut + Vert Stability + Lat Stability			5,700	0	600	1.000

AXIAL MOVEMENT - 3.25 EXP. ST. CONTRACTION
 1.63 ERCT HANGERS

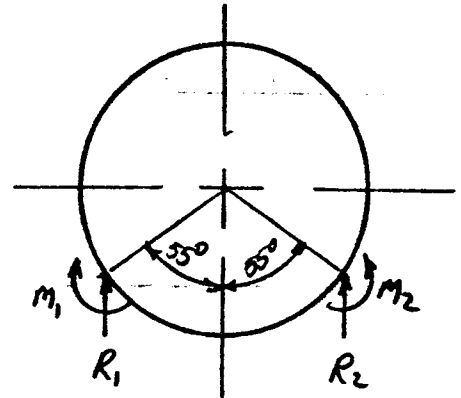
SHELL RING DESIGN

LOAD COMBINATION #1

VERTICAL LOAD = 5700[#] ACTING 9.5"
OUTSIDE RING CENTROID @ $\theta = 55^\circ$

$$R_1 = R_2 = \frac{5700^{\#}}{2} = 2850^{\#}$$

$$M_1 = M_2 = 2850^{\#}(9.5") = 27,075 \text{ IN-LBS}$$



$$\text{RADIAL LOAD (P)} = R \cos 55^\circ = 2850^{\#}(.5736) = 1635^{\#}$$

$$\text{TANGENT LOAD (T)} = R \sin 55^\circ = 2850^{\#}(.8192) = 2335^{\#}$$

$$M/R = \frac{27,075 \text{ IN-LBS}}{26.07"} = 1038.6^{\#}$$

USE SAME RING AS FIXED SUPPORT 4" x 3/8"

$$A = 1.897 \text{ IN}^2 \quad C_1 = 1.695" \quad C_2 = 2.432" \quad R = 26.07"$$

$$I = 3.34 \text{ IN}^4 \quad S_1 = 1.97 \text{ IN}^3 \quad S_2 = 1.37 \text{ IN}^3$$

OUTPUT PROGRAM F0405A
@ 55°

$$V = (899.4 \text{ PSI})(1.897 \text{ IN}^2) = 1,706^{\#}$$

$$T = (918.45 \text{ PSI})(1.897 \text{ IN}^2) = -1,743^{\#}$$

$$M = 22,155 \text{ IN-LBS}$$

SUBJECT Ligo BEAM TUBE GUIDEP (SUPPORT DESIGN)	OFFICE CBI NOE	REVISION	REFERENCE NO. 930210
	MADE BY MRS	CHKD BY JGG	SHT 6 OF 35
	DATE 3/1/94	DATE 4/6/94	11.6

110.00	-1.4529E+02	-5.1116E+02	-1.0185E+03	6.2252E-04	-5.5369E-03	4.6663E-03	110
115.00	-1.0357E+02	-4.4487E+02	-1.5535E+03	5.9087E-04	-3.7282E-03	5.0703E-03	115
120.00	-6.7658E+01	-3.7828E+02	-1.9209E+03	5.4829E-04	-1.9762E-03	5.3186E-03	120
125.00	-3.7529E+01	-3.1244E+02	-2.1458E+03	4.9857E-04	-3.1486E-04	5.4178E-03	125
130.00	-1.3075E+01	-2.4840E+02	-2.2530E+03	4.4485E-04	1.2308E-03	5.3770E-03	130
135.00	5.9058E+00	-1.8715E+02	-2.2665E+03	3.8971E-04	2.6429E-03	5.2069E-03	135
140.00	1.9697E+01	-1.2961E+02	-2.2095E+03	3.3515E-04	3.9098E-03	4.9199E-03	140
145.00	2.8661E+01	-7.6664E+01	-2.1035E+03	2.8261E-04	5.0250E-03	4.5289E-03	145
150.00	3.3234E+01	-2.9094E+01	-1.9684E+03	2.3304E-04	5.9858E-03	4.0474E-03	150
155.00	3.3916E+01	1.2397E+01	-1.8222E+03	1.8693E-04	6.7921E-03	3.4887E-03	155
160.00	3.1264E+01	4.7200E+01	-1.6805E+03	1.4435E-04	7.4460E-03	2.8664E-03	160
165.00	2.5887E+01	7.4806E+01	-1.5563E+03	1.0502E-04	7.9502E-03	2.1935E-03	165
170.00	1.8429E+01	9.4815E+01	-1.4600E+03	6.8402E-05	8.3077E-03	1.4831E-03	170
175.00	9.5682E+00	1.0694E+02	-1.3992E+03	3.3702E-05	8.5210E-03	7.4773E-04	175
180.00	0.0000E+00	1.1100E+02	-1.3784E+03	0.0000E+00	8.5919E-03	0.0000E+00	180
180.00	0.0000E+00	1.1100E+02	-1.3784E+03	0.0000E+00	8.5919E-03	0.0000E+00	180

PRINTOUT AT LOAD AND/OR MOMENT POINTS

55.00	-8.9941E+02	-9.1845E+02	2.2155E+04	-1.3926E-03	-8.1512E-03	-6.5397E-03	55
54.99	-3.7469E+01	3.1231E+02	-4.9212E+03	-1.3924E-03	-8.1438E-03	-6.5411E-03	55
305.00	3.7524E+01	3.1244E+02	-4.9215E+03	1.3926E-03	-8.1512E-03	6.5397E-03	305
304.99	8.9925E+02	-9.1847E+02	2.2147E+04	1.3916E-03	-8.1587E-03	6.5382E-03	305

PRINTOUT OF MAXIMUM AND MINIMUM VALUES OF SHEAR, THRUST, AND MOMENT VALUES TAKEN FROM BOTH OF THE TABLES ABOVE

MAXIMUM V/A=	8.9925E+02	AT	304.99	DEGREES
MINIMUM V/A=	-8.9941E+02	AT	55.00	DEGREES
MAXIMUM T/A=	3.1244E+02	AT	305.00	DEGREES
MINIMUM T/A=	-9.2305E+02	AT	60.00	DEGREES
UM M(FORCE-LG)=	2.2155E+04	AT	55.00	DEGREES
UM M(FORCE-LG)=	-5.6890E+03	AT	.00	DEGREES

PRINTOUT OF MAXIMUM AND MINIMUM (AXIAL + BENDING) STRESSES FOR MOMENT ARMS OF C1 AND C2 - VALUES TAKEN FROM BOTH OF THE TABLES ABOVE

MAXIMUM STRESS (C1)=	1.0325E+04	AT	55.00	DEGREES
MINIMUM STRESS (C1)=	-2.9981E+03	AT	.00	DEGREES
MAXIMUM STRESS (C2)=	4.0314E+03	AT	.00	DEGREES
MINIMUM STRESS (C2)=	-1.7050E+04	AT	55.00	DEGREES

TOP [SYSIN]

DDG 4/6/94

USING $4 \times \frac{3}{8}$ STIFFENER RING

MAXIMUM STRESS IN SHELL = 10,325 PSI @ 55°

MINIMUM STRESS IN SHELL = -2,998 PSI @ 0°

MAXIMUM STRESS IN STIFFENER BAR = 4,031 PSI @ 0°

MINIMUM STRESS IN STIFFENER BAR = -17,050 PSI @ 55°

TRY INCREASING THE RING TO 6" WIDE $\times \frac{3}{8}$

$$A = 2.647 \text{ in}^2 \quad I = 9.92 \text{ in}^4 \quad C1 = 2.668''$$

$$C2 = 3.459'' \quad S1 = 3.72 \text{ in}^3 \quad S2 = 2.87 \text{ in}^3$$

$$16t = 16(.375) = 6''$$

$$f = \frac{T}{A} \pm \frac{M}{S}$$

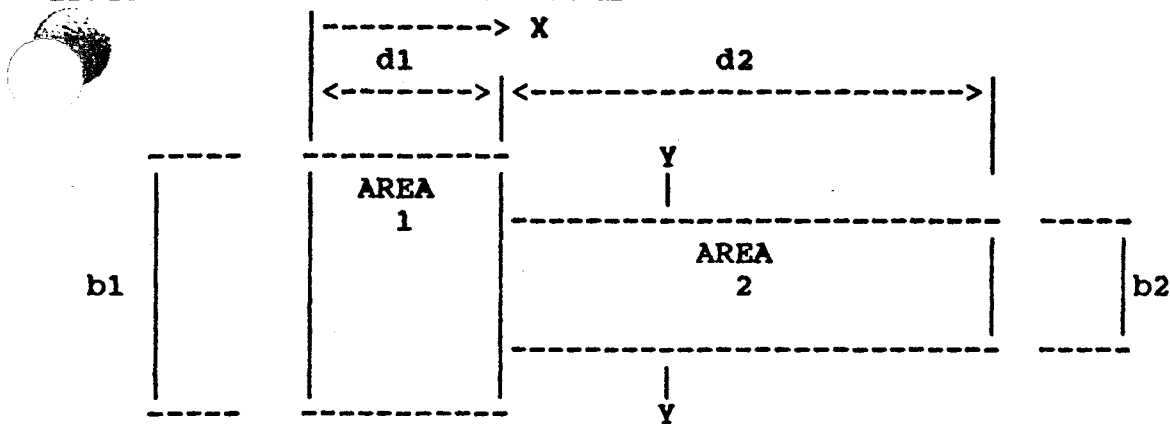
$$f_{55^\circ} = \frac{-1743''}{2.647''} - \frac{22,155 \text{ in-lbs}}{2.87 \text{ in}^3} = 658 + 7,720$$

$$f = 8,378 \text{ PSI}$$

$$FA = .6R_y = .6(25,000 \text{ PSI}) = 15,000 \text{ PSI}$$

SUBJECT	OFFICE		REVISION		REFERENCE NO.
		CBI	NOE		
	MADE BY	CHKD BY	MADE BY	CHKD BY	SHT 9 OF 35
	DATE	DATE	DATE	DATE	11.9
	3/2/94	4/6/94			

MOMENT OF INERTIA CALCULATION



	1	2	3	4	5	6	7
d	0.127	6					
b	3.125	0.375					
X	0.0635	3.1270	6.1270	6.1270	6.1270	6.1270	6.1270
THETA	0	0	0	0	0	0	0

	AREA	X	AX	AX ²	Io
1	0.397	0.064	0.03	0.00	0.00
2	2.250	3.127	7.04	22.00	6.75
3	0.000	6.127	0.00	0.00	0.00
4	0.000	6.127	0.00	0.00	0.00
5	0.000	6.127	0.00	0.00	0.00
6	0.000	6.127	0.00	0.00	0.00
7	0.000	6.127	0.00	0.00	0.00
	2.647		7.06		28.75

TOTAL DEPTH = 6.1270 in.

CENTROID (\bar{X}) = $\text{SUM}(AX)/\text{SUM}(AREA)$ = 2.668 in.

$I(\text{total}) = [\text{SUM}(AX^2) + \text{SUM}(Io)] - (AREA)(\bar{X})^2 = 9.92 \text{ in.}^4$

$C1 = \bar{X} = 2.668 \text{ in.}$ $C2 = \text{DEPTH} - \bar{X} = 3.459 \text{ in.}$

$Sy1 = I/C1 = 3.72 \text{ in.}^3$ $Sy2 = I/C2 = 2.87 \text{ in.}^3$

Radius of gyration (r) = $(I/A)^{1/2} = 1.936 \text{ in.}$

Torsional constant (J) = in.^4

OBJECT	!MADE BY: MRS ! R !MADE BY:	!REFERENCE NO.!
SECTION PROPERTIES	!-----! E !-----!	93021~ !
1/6" X 3/8" STIFFENER	!CHKD BY: DDG ! V !CHKD BY:	!-----!
!FIXED SUPPORT SHELL RING	!-----!	!-----!
!LIGO BEAM TUBE	!DATE: 3/01/94!	!DATE: !SHT 10 OF 35!

CHECK 4" x 3/8" RING @ 90°

$$T/A = -751 \text{ PSI}$$

$$M = 3,285 \text{ IN-LBS}$$

$$f = -751 \text{ PSI} - \frac{3,285 \text{ IN-LBS}}{1.37 \text{ IN}} = -3,149 \text{ PSI}$$

$$F_A = 15,000 \text{ PSI}$$

CHECK 4 x 3/8 RING @ 40°

$$T/A = 130 \text{ PSI}$$

$$M = -4,858 \text{ IN-LBS}$$


$$f_1 = T/A + M/S_1$$

$$f_2 = T/A - M/S_2$$

$$f_1 = 130 + \frac{-4,858}{1.97} = -2,336 \text{ PSI}$$

$$f_2 = 130 - \frac{-4,858}{1.37} = 3,676 \text{ PSI}$$

$$F_A = 15,000 \text{ PSI}$$

SUBJECT	 OFFICE	REVISION		REFERENCE NO.
	MADE BY	CHKD BY	MADE BY	CHKD BY
	DATE	DATE	DATE	DATE
	NOE			930212
	MRS	DDG		SHT # OF 35
	3/2/94	4/6/94		11.11

LOAD COMBINATION #3

VERTICAL LOAD = 5700 #

LATERAL LOAD = 1,730 #

$H = 1730 \#$

VERTICAL LOADS DUE TO SEISMIC

$\sum M_{QH} = 0$

$(1730 \#)(28502") - (21355")R_1 + (21355")R_2 - M_1 + M_2 = 0$

$M_1 = 9.5 R_1$ $M_2 = 9.5 R_2$

$R_2 = -R_1$

$R_1 = R_2 = 799 \#$

VERTICAL LOAD DUE TO DL + VERT. STABILITY

$R_1 = R_2 = \frac{5700 \#}{2} = 2850 \#$

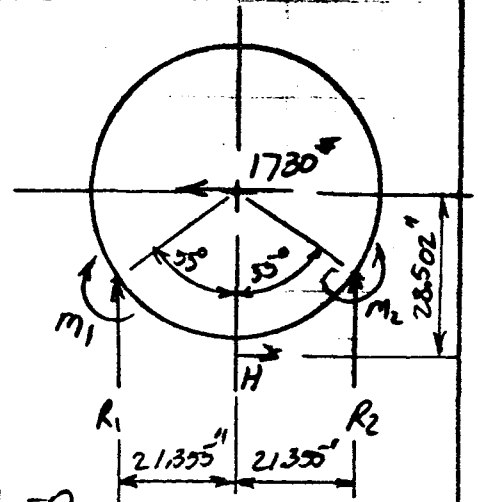
$R_1 = 2850 \# + 799 \# = 3649$

$R_2 = 2850 \# - 799 \# = 2051$

$M_1 = (3649)(9.5') = 34,666 \text{ IN-LBS}$

$M_2 = (2051)(9.5') = 19,485 \text{ IN-LBS}$

MOMENT AT INVERT (M_b) = $(4.127' - 1.695')(1730 \#) = 4,208 \text{ IN-LBS}$



SUBJECT	OFFICE		REVISION		REFERENCE NO.
	CBI NOE				930212
	MADE BY	CHKD BY	MADE BY	CHKD BY	SHT. 12 OF 35
	MRS	DDG			
DATE	DATE	DATE	DATE		11-12
	3/2/94	4/6/94			

INPUT INTO CBI COMPUTER PROGRAM EDA05A

2 RADIAL LOADS AT 55° & 305°

$$P_{55} = (3649^*) \cos 55^\circ = 2,093^*$$

$$P_{305} = (2051^*) \cos 55^\circ = 1176^*$$

3 TANGENT LOADS @ 0°, 55° & 305°

$$T_0 = 1730^*$$

$$T_{55} = (-3649) \sin 55^\circ = -2989^*$$

$$T_{305} = (2051) \sin 55^\circ = 1680^*$$

3 MOMENT LOADS @ 0°, 55° & 305°

$$\frac{M_0}{R} = \frac{-4208 \text{ IN-LBS}}{26.07''} = -161.4^*$$

$$\frac{M_{55}}{R} = \frac{34,666 \text{ IN-LBS}}{26.07''} = 1330^*$$

$$\frac{M_{305}}{R} = \frac{-19,485 \text{ IN-LBS}}{26.07''} = -747^*$$

4" x 3/8" RWB

$$R = 26.07'' \quad E = 28,000,000 \text{ PSI} \quad I = 3.39 \text{ IN}^4$$

$$A = 1.897 \text{ IN}^2 \quad C_1 = 1.695'' \quad C_2 = 2.432''$$

SUBJECT	OFFICE		REVISION		REFERENCE NO.
	CBI NOE				930212
	MADE BY	CHKD BY	MADE BY	CHKD BY	SHT 13 OF 35
	MRS	DDG			
DATE	DATE	DATE	DATE		
3/2/99	4/6/99			11.13	

PROGRAM E0405A (STD 9110-1-2) [IBM RS/6000]
 STRESS AND STRAIN COEFFICIENTS FOR CIRCULAR RINGS
 REVISION 11 - JANUARY 7, 1992

STANDARDS 9110-1-1 AND 9110-1-2

GUIDE SUPPORT LOAD COMBINATION #3 - 2 PT. AT 55 DEG.

N(R) N(T) N(M) DATA SHEET NO. PLOT CODE
 2 3 3 1 0

RADIUS MOD-ELAS MOM-INER AREA C(1) C(2)
 26.07 28000000.00 3.34 1.90 1.70 2.43

EQUILIBRIUM LOADS FV FH M/R
 INPUT 5700.000 1730.000 .000
 COMPUTED 5699.642 1730.351 .600

INPUT LOADS AND THEIR LOCATIONS

RADIAL LOADS AND ANGLES

LOADS 2093.00 1176.00
 ANGLES 55.00 305.00

TANGENTIAL LOADS AND ANGLES

LOADS 1730.00 -2989.00 1680.00
 ANGLES .00 55.00 305.00

MOMENT LOADS/R AND ANGLES

LOADS -161.40 1330.00 -747.00
 ANGLES .00 55.00 305.00

UNIT STRESS VALUES FOR CIRCULAR RINGS

PRINTOUT AT EQUAL ANGLE INCREMENTS

11.14

X	V/A	T/A	M(FORCE-LG)	THETA	DR	DC	
.00	2.7303E+02	3.4510E+02	-7.7916E+03	-3.5152E-04	1.4830E-02	-4.3481E-03	
5.00	2.4291E+02	3.4595E+02	-6.6782E+03	-5.2726E-04	1.3391E-02	-5.5832E-03	5
10.00	2.1250E+02	3.5161E+02	-5.6953E+03	-6.7749E-04	1.1481E-02	-6.6716E-03	10
15.00	1.8140E+02	3.6214E+02	-4.8450E+03	-8.0543E-04	9.1667E-03	-7.5752E-03	15
20.00	1.4916E+02	3.7751E+02	-4.1312E+03	-9.1433E-04	6.5145E-03	-8.2616E-03	20
25.00	1.1537E+02	3.9759E+02	-3.5598E+03	-1.0076E-03	3.5835E-03	-8.7041E-03	25
30.00	7.9635E+01	4.2218E+02	-3.1382E+03	-1.0887E-03	4.2769E-04	-8.8805E-03	30
35.00	4.1566E+01	4.5096E+02	-2.8758E+03	-1.1616E-03	-2.9057E-03	-8.7736E-03	35
40.00	8.1767E-01	4.8353E+02	-2.7833E+03	-1.2300E-03	-6.3768E-03	-8.3694E-03	40
45.00	-4.2922E+01	5.1941E+02	-2.8730E+03	-1.2984E-03	-9.9542E-03	-7.6575E-03	45
50.00	-8.9916E+01	5.5804E+02	-3.1584E+03	-1.3714E-03	-1.3616E-02	-6.6296E-03	50
55.00	-1.2437E+03	-9.7685E+02	3.1019E+04	-1.4538E-03	-1.7349E-02	-5.2791E-03	55
60.00	-1.1563E+03	-1.0248E+03	2.5839E+04	-7.6301E-04	-2.0237E-02	-3.6266E-03	60
65.00	-1.0652E+03	-1.0595E+03	2.1044E+04	-1.9355E-04	-2.1540E-02	-1.7931E-03	65
70.00	-9.7173E+02	-1.0810E+03	1.6648E+04	2.6409E-04	-2.1514E-02	9.4271E-05	70
75.00	-8.7693E+02	-1.0897E+03	1.2658E+04	6.1972E-04	-2.0401E-02	1.9302E-03	75
80.00	-7.8191E+02	-1.0861E+03	9.0788E+03	8.8328E-04	-1.8431E-02	3.6300E-03	80
85.00	-6.8771E+02	-1.0708E+03	5.9081E+03	1.0647E-03	-1.5817E-02	5.1284E-03	85
90.00	-5.9534E+02	-1.0445E+03	3.1402E+03	1.1740E-03	-1.2754E-02	6.3776E-03	90
95.00	-5.0571E+02	-1.0080E+03	7.6545E+02	1.2207E-03	-9.4178E-03	7.3463E-03	95
100.00	-4.1617E+02	-9.6275E+02	-4.2700E+03	1.2143E-03	-5.9659E-03	8.0179E-03	100

10.00	-2.6133E+02	-7.8025E+02	-5.1269E+03	9.6427E-04	3.8173E-03	8.2627E-03	115
20.00	-1.2529E+02	-7.0826E+02	-5.8056E+03	8.3073E-04	6.5652E-03	7.8072E-03	120
25.00	-6.6759E+01	-6.3262E+02	-6.2176E+03	6.8397E-04	8.9432E-03	7.1277E-03	125
30.00	-1.4947E+01	-5.5451E+02	-6.3915E+03	5.3015E-04	1.0910E-02	6.2583E-03	130
35.00	2.9986E+01	-4.7518E+02	-6.3565E+03	3.7470E-04	1.2442E-02	5.2361E-03	135
40.00	6.7986E+01	-3.9581E+02	-6.1426E+03	2.2234E-04	1.3528E-02	4.0997E-03	140
45.00	9.9101E+01	-3.1758E+02	-5.7796E+03	7.7050E-05	1.4172E-02	2.8879E-03	145
50.00	1.2348E+02	-2.4162E+02	-5.2969E+03	-5.7891E-05	1.4390E-02	1.6386E-03	150
55.00	1.4137E+02	-1.6902E+02	-4.7231E+03	-1.7993E-04	1.4205E-02	3.8811E-04	155
60.00	1.5311E+02	-1.0079E+02	-4.0855E+03	-2.8717E-04	1.3651E-02	-8.2987E-04	160
65.00	1.5912E+02	-3.7880E+01	-3.4098E+03	-3.7839E-04	1.2767E-02	-1.9848E-03	165
70.00	1.5990E+02	1.8868E+01	-2.7197E+03	-4.5296E-04	1.1598E-02	-3.0499E-03	170
75.00	1.5602E+02	6.8691E+01	-2.0364E+03	-5.1077E-04	1.0190E-02	-4.0021E-03	175
80.00	1.4813E+02	1.1094E+02	-1.3787E+03	-5.5224E-04	8.5917E-03	-4.8228E-03	180
80.00	1.4813E+02	1.1094E+02	-1.3787E+03	-5.5224E-04	8.5917E-03	-4.8228E-03	180

PRINTOUT AT LOAD AND/OR MOMENT POINTS

.00	2.7303E+02	3.4510E+02	-7.7916E+03	-3.5152E-04	1.4830E-02	-4.3481E-03	
-.01	2.7293E+02	-5.6686E+02	-3.5862E+03	-3.5134E-04	1.4833E-02	-4.3455E-03	
55.00	-1.2437E+03	-9.7685E+02	3.1019E+04	-1.4538E-03	-1.7349E-02	-5.2791E-03	55
54.99	-1.4028E+02	5.9871E+02	-3.6529E+03	-1.4536E-03	-1.7341E-02	-5.2821E-03	55
305.00	-6.5192E+01	2.6036E+01	-6.1908E+03	1.3312E-03	1.0495E-03	7.7991E-03	305
304.99	5.5458E+02	-8.5950E+02	1.3279E+04	1.3305E-03	1.0420E-03	7.7993E-03	305

PRINTOUT OF MAXIMUM AND MINIMUM VALUES OF SHEAR, THRUST, AND MOMENT
VALUES TAKEN FROM BOTH OF THE TABLES ABOVE

MAXIMUM V/A=	5.5458E+02	AT	304.99	DEGREES
MINIMUM V/A=	-1.2437E+03	AT	55.00	DEGREES
MAXIMUM T/A=	5.9871E+02	AT	54.99	DEGREES
MINIMUM T/A=	-1.0897E+03	AT	75.00	DEGREES
MAXIMUM M(FORCE-LG)=	3.1019E+04	AT	55.00	DEGREES
MINIMUM M(FORCE-LG)=	-7.7916E+03	AT	.00	DEGREES

PRINTOUT OF MAXIMUM AND MINIMUM (AXIAL + BENDING) STRESSES FOR MOMENT
ARMS OF C1 AND C2 - VALUES TAKEN FROM BOTH OF THE TABLES ABOVE

MAXIMUM STRESS (C1)=	1.4765E+04	AT	55.00	DEGREES
MINIMUM STRESS (C1)=	-3.7981E+03	AT	130.00	DEGREES
MAXIMUM STRESS (C2)=	6.0185E+03	AT	.00	DEGREES
MINIMUM STRESS (C2)=	-2.3563E+04	AT	55.00	DEGREES

TOP [SYSIN]

DDG

4/6/94

E0405A OUTPUT

4" x 3/8" RING

$\sigma_{MAX} = 14,765 \text{ PSI}$ IN SHELL @ 55°
 $\sigma_{MIN} = -3798 \text{ PSI}$ IN SHELL @ 130°

$\sigma_{MAX} = 6019 \text{ PSI}$ IN STIFF. @ 0°
 $\sigma_{MIN} = -23,563 \text{ PSI}$ IN STIFF. @ 55°

TRY 6" x 3/8" SECTION @ 55°

$A = 2.647 \text{ in}^2$ $I = 9.92 \text{ in}^4$ $C_1 = 2.668"$ $C_2 = 3.459"$
 $S_1 = 3.72 \text{ in}^3$ $S_2 = 2.87 \text{ in}^3$

$f = T/A I \text{ m/s}$

$T/A = (-977 \text{ PSI}) \left(\frac{1.897 \text{ in}^2}{2.647 \text{ in}^2} \right) = -700 \text{ PSI}$ @ 55°

$M = 31,019 \text{ in-lbs}$ @ 55°

$f_{SHELL} = -700 + \frac{31,019}{3.72 \text{ in}^3} = 7,638 \text{ PSI}$

$f_{STIFF} = -700 + \frac{-31,019}{2.87} = 11,508 \text{ PSI}$

$F_A = .6R_y = .6(25,000 \text{ PSI})(1.33) = 19,950 \text{ PSI}$

SUBJECT	OFFICE		REVISION		REFERENCE NO.
	CBI	NOE			930212
	MADE BY	CHKD BY	MADE BY	CHKD BY	SHT 16 OF 35
	<i>MRS</i>	<i>DJS</i>			11.16
DATE	DATE	DATE	DATE		
3/2/94	4/6/94				

LOAD COMBINATION #4

ADDITIONAL LOAD DUE TO TORSION MOMENT

$$\text{VERTICAL LOAD} = 5,700^{\#} + 102^{\#} = 5,802^{\#}$$

$$\text{LATERAL LOAD} = 600^{\#} = H$$

PER SKETCH & METHODS FOR LOAD COMBINATION #3

FOR: LATERAL FORCES

$$\sum M_{OH} = 0$$

$$(600^{\#})(28.502'') - (21.355'')R_1 + (21.355'')R_2 - M_1 + M_2 = 0$$

$$M_1 = 9.5' R_1 \quad M_2 = 9.5' R_2$$

$$R_2 = -R_1$$

$$R_1 = R_2 = 277^{\#}$$

DUE TO VERT LOADS

$$R_1 = R_2 = \frac{5802}{2} = 2901^{\#}$$

$$R_1 = 2901 + 277 = 3,178^{\#}$$

$$R_2 = 2901 - 277 = 2,624^{\#}$$

$$M_1 = 3178^{\#}(9.5') = 30,191^{\#}$$

$$M_2 = 2,624^{\#}(9.5') = 24,928^{\#}$$

$$M_{O INVERT} = M_0 = (4.127' - 1.695')(600^{\#}) = 1459 \text{ N-LBS}$$

SUBJECT	OFFICE		REVISION		REFERENCE NO.
		CBI NOE			930212
	MADE BY MRS	CHKD BY DJS	MADE BY	CHKD BY	SHT 17 OF 35
	DATE 3/2/94	DATE 4/6/94	DATE	DATE	11.17

ED905A INPUT

$$P_{55} = (3178) \cos 55^\circ = 1823 \#$$

$$P_{205} = (2624) \cos 55^\circ = 1505 \#$$

$$T_0 = 600 \#$$

$$T_{55} = (-3178) \sin 55^\circ = -2603 \#$$

$$T_{205} = (2624) \sin 55^\circ = 2149 \#$$

$$m_{0/R} = \frac{-1459}{2607} = -56 \#$$

$$m_{55/R} = \frac{30191}{2607} = 1,158 \#$$

$$m_{205/R} = \frac{-24,928}{2607} = -956 \#$$

4" x 3/8" RING

$$R = 26.07" \quad E = 27,000,000 \text{ PSI @ } 300^\circ \text{F} \quad I = 3.34 \text{ in}^4$$

$$A = 1.897 \text{ in}^2 \quad C_1 = 1.695" \quad C_2 = 2.432"$$

$$F_y = 19,200 \text{ PSI @ } 300^\circ \text{F}$$

SUBJECT	OFFICE CBI NOE		REVISION		REFERENCE NO. 930212
	MADE BY MRS	CHKD BY DDG	MADE BY	CHKD BY	SHT <u>18</u> OF <u>35</u>
	DATE 3/2/94	DATE 4/6/94	DATE	DATE	11-18

PROGRAM E0405A (STD 9110-1-2) [IBM RS/6000]
 STRESS AND STRAIN COEFFICIENTS FOR CIRCULAR RINGS
 REVISION 11 - JANUARY 7, 1992

STANDARDS 9110-1-1 AND 9110-1-2

GUIDE SUPPORT LOAD COMBINATION #4 - 2 PT. AT 55 DEG.

N(R) N(T) N(M) DATA SHEET NO. PLOT CODE
 2 3 3 1 0

RADIUS MOD-ELAS MOM-INER AREA C(1) C(2)
 26.07 27000000.00 3.34 1.90 1.70 2.43

EQUILIBRIUM LOADS FV FH M/R
 INPUT 5802.000 600.000 .000
 COMPUTED 5801.473 600.087 .000

INPUT LOADS AND THEIR LOCATIONS

RADIAL LOADS AND ANGLES

LOADS 1823.00 1505.00
 ANGLES 55.00 305.00

TANGENTIAL LOADS AND ANGLES

LOADS 600.00 -2603.00 2149.00
 ANGLES .00 55.00 305.00

MOMENT LOADS/R AND ANGLES

LOADS -56.00 1158.00 -956.00
 ANGLES .00 55.00 305.00

UNIT STRESS VALUES FOR CIRCULAR RINGS

PRINTOUT AT EQUAL ANGLE INCREMENTS

11.19

X	V/A	T/A	M(FORCE-LG)	THETA	DR	DC	X
.00	9.4730E+01	4.5107E+01	-6.5208E+03	-1.2648E-04	1.5657E-02	-1.5641E-03	.
5.00	9.0713E+01	4.8128E+01	-6.1205E+03	-2.8590E-04	1.4991E-02	-2.9049E-03	5.
10.00	8.6124E+01	5.8227E+01	-5.7386E+03	-4.3545E-04	1.3859E-02	-4.1669E-03	10.
15.00	8.0348E+01	7.5306E+01	-5.3789E+03	-5.7563E-04	1.2292E-02	-5.3111E-03	15.
20.00	7.2783E+01	9.9162E+01	-5.0477E+03	-7.0708E-04	1.0324E-02	-6.3007E-03	20.
25.00	6.2853E+01	1.2948E+02	-4.7541E+03	-8.3063E-04	7.9870E-03	-7.1022E-03	25.
30.00	5.0008E+01	1.6585E+02	-4.5094E+03	-9.4736E-04	5.3164E-03	-7.6850E-03	30.
35.00	3.3745E+01	2.0775E+02	-4.3273E+03	-1.0587E-03	2.3465E-03	-8.0214E-03	35.
40.00	1.3605E+01	2.5458E+02	-4.2237E+03	-1.1664E-03	-8.8995E-04	-8.0848E-03	40.
45.00	-1.0811E+01	3.0563E+02	-4.2160E+03	-1.2726E-03	-4.3623E-03	-7.8592E-03	45.
50.00	-3.9838E+01	3.6013E+02	-4.3236E+03	-1.3800E-03	-8.0439E-03	-7.3193E-03	50.
55.00	-1.0347E+03	-9.5492E+02	2.5622E+04	-1.4919E-03	-1.1913E-02	-6.4498E-03	55.
60.00	-9.5045E+02	-9.7465E+02	2.1338E+04	-9.0030E-04	-1.5126E-02	-5.2595E-03	60.
65.00	-8.6497E+02	-9.8260E+02	1.7420E+04	-4.1219E-04	-1.6999E-02	-3.8488E-03	65.
70.00	-7.7929E+02	-9.7924E+02	1.3872E+04	-1.8256E-05	-1.7742E-02	-2.3254E-03	70.
75.00	-6.9437E+02	-9.6518E+02	1.0693E+04	2.9083E-04	-1.7552E-02	-7.7934E-04	75.
80.00	-6.1112E+02	-9.4112E+02	7.8762E+03	5.2430E-04	-1.6613E-02	7.1621E-04	80.
85.00	-5.3038E+02	-9.0785E+02	5.4141E+03	6.9121E-04	-1.5095E-02	2.1034E-03	85.
90.00	-4.5291E+02	-8.6627E+02	3.2936E+03	8.0034E-04	-1.3149E-02	3.3384E-03	90.
95.00	-3.7940E+02	-8.1733E+02	1.4991E+03	8.6013E-04	-1.0913E-02	4.3900E-03	95.
100.00	-3.0999E+02	-7.6222E+02	0.0000E+00	0.0000E+00	-8.5049E-03	5.2382E-03	100.

10.00	-1.8813E+02	-6.3080E+02	-2.1238E+03	8.2000E-04	-1.2222E-03	6.5005E-03	115.
15.00	-1.3550E+02	-5.6905E+02	-2.8200E+03	7.5802E-04	-1.2222E-03	6.5005E-03	115.
20.00	-8.8867E+01	-4.9941E+02	-3.3020E+03	6.8037E-04	9.8540E-04	6.5096E-03	120.
25.00	-4.8355E+01	-4.2901E+02	-3.5959E+03	5.9300E-04	2.9970E-03	6.3343E-03	125.
30.00	-1.3980E+01	-3.5897E+02	-3.7282E+03	5.0030E-04	4.7803E-03	5.9932E-03	130.
35.00	1.4338E+01	-2.9036E+02	-3.7252E+03	4.0603E-04	6.3140E-03	5.5072E-03	135.
40.00	3.6769E+01	-2.2424E+02	-3.6129E+03	3.1326E-04	7.5865E-03	4.8988E-03	140.
45.00	5.3576E+01	-1.6159E+02	-3.4160E+03	2.2445E-04	8.5945E-03	4.1908E-03	145.
50.00	6.5100E+01	-1.0333E+02	-3.1580E+03	1.4142E-04	9.3414E-03	3.4064E-03	150.
55.00	7.1763E+01	-5.0323E+01	-2.8611E+03	6.5434E-05	9.8362E-03	2.5678E-03	155.
60.00	7.4057E+01	-3.3159E+00	-2.5449E+03	-2.7767E-06	1.0092E-02	1.6966E-03	160.
65.00	7.2536E+01	3.7021E+01	-2.2273E+03	-6.2959E-05	1.0125E-02	8.1285E-04	165.
70.00	6.7807E+01	7.0123E+01	-1.9234E+03	-1.1527E-04	9.9538E-03	-6.4693E-05	170.
75.00	6.0521E+01	9.5532E+01	-1.6457E+03	-1.6023E-04	9.5959E-03	-9.1900E-04	175.
80.00	5.1367E+01	1.1291E+02	-1.4037E+03	-1.9861E-04	9.0704E-03	-1.7346E-03	180.
80.00	5.1367E+01	1.1291E+02	-1.4037E+03	-1.9861E-04	9.0704E-03	-1.7346E-03	180.

PRINTOUT AT LOAD AND/OR MOMENT POINTS

.00	9.4730E+01	4.5107E+01	-6.5208E+03	-1.2648E-04	1.5657E-02	-1.5641E-03	
-.01	9.4683E+01	-2.7118E+02	-5.0617E+03	-1.2622E-04	1.5658E-02	-1.5614E-03	
55.00	-1.0347E+03	-9.5492E+02	2.5622E+04	-1.4919E-03	-1.1913E-02	-6.4498E-03	55.
54.99	-7.3669E+01	4.1713E+02	-4.5663E+03	-1.4917E-03	-1.1905E-02	-6.4519E-03	55.
65.00	2.5003E+00	2.1855E+02	-5.4495E+03	1.4480E-03	-5.2950E-03	7.3567E-03	305.
64.99	7.9570E+02	-9.1428E+02	1.9467E+04	1.4470E-03	-5.3028E-03	7.3557E-03	305.

PRINTOUT OF MAXIMUM AND MINIMUM VALUES OF SHEAR, THRUST, AND MOMENT VALUES TAKEN FROM BOTH OF THE TABLES ABOVE

MAXIMUM V/A=	7.9570E+02	AT	304.99	DEGREES
MINIMUM V/A=	-1.0347E+03	AT	55.00	DEGREES
MAXIMUM T/A=	4.1713E+02	AT	54.99	DEGREES
MINIMUM T/A=	-9.8260E+02	AT	65.00	DEGREES
MAXIMUM M(FORCE-LG)=	2.5622E+04	AT	55.00	DEGREES
MINIMUM M(FORCE-LG)=	-6.5208E+03	AT	.00	DEGREES

PRINTOUT OF MAXIMUM AND MINIMUM (AXIAL + BENDING) STRESSES FOR MOMENT TERMS OF C1 AND C2 - VALUES TAKEN FROM BOTH OF THE TABLES ABOVE

MAXIMUM STRESS (C1)=	1.2048E+04	AT	55.00	DEGREES
MINIMUM STRESS (C1)=	-3.2641E+03	AT	.00	DEGREES
MAXIMUM STRESS (C2)=	4.7932E+03	AT	.00	DEGREES
MINIMUM STRESS (C2)=	-1.9612E+04	AT	55.00	DEGREES

TOP [SYSIN]

DDG 4/6/94

EO405A OUTPUT

4 x 3/8 RMS

$\tau_{MAX} (SHALL) = 12,048 \text{ PSI} @ 55^\circ$

$\tau_{MIN} (SHALL) = -3264 \text{ PSI} @ 0^\circ$

$\tau_{MAX} (STIFF) = 4793 \text{ PSI} @ 0^\circ$

$\tau_{MIN} (STIFF) = -9,612 \text{ PSI} @ 55^\circ$

TRY 6" x 3/8" SECTION @ 55°

$A = 2.647 \text{ IN}^2 \quad I = 9.92 \text{ IN}^4 \quad C_1 = 2.668" \quad C_2 = 3.459"$

$S_1 = 3.72 \text{ IN}^3 \quad S_2 = 2.87 \text{ IN}^3$

$A = T/A \pm M/S$

@ 55° $T/A = (-955 \text{ PSI}) \left(\frac{1.897 \text{ IN}^2}{2.647 \text{ IN}^2} \right) = -685 \text{ PSI}$

$M = 25,622 \text{ IN-LBS}$

$F_{SHALL} = -685 \text{ PSI} + \frac{25,622}{3.72 \text{ IN}^3} = 6,203 \text{ PSI}$

$F_{STIFF} = -685 \text{ PSI} - \frac{25,622}{2.87} = -9,613 \text{ PSI}$

$F_A = .6F_y = .6(19,200) = 11,520 \text{ PSI}$

SUBJECT	OFFICE		REVISION		REFERENCE NO.
	CBI	NoE			930212
	MADE BY	CHKD BY	MADE BY	CHKD BY	SHT 21 OF 35
	MRS	DDG			
DATE	DATE	DATE	DATE		11.21
	3/2/94	4/6/94			

RING HANGER LUG DESIGN

$$V_{max} = 3650^{\#} \quad \text{LOAD COMBINATION \#3}$$

CHECK BENDING OF LUG

2" FROM HANGER CENTERLINE

$$M = 2" (3650^{\#}) = 7300 \text{ IN-LBS}$$

$$\text{PLATE } t = \frac{3}{8}'' \quad \text{WIDTH} = 5\frac{1}{2}''$$

$$S = \frac{(5.5)^2 (0.375)}{6} = 1.89 \text{ IN}^3$$

$$f_{bx} = \frac{7300 \text{ IN-LBS}}{1.89 \text{ IN}^3} = 3862 \text{ PSI}$$

7" FROM HANGER ϕ

$$M = 7" (3650^{\#}) = 25,550 \text{ IN-LBS}$$

$$\text{ASSUME LUG WIDTH} = 5.5'' + 2(3'') = 11.5'' \quad S = 8.26 \text{ IN}^3$$

$$f_{bx} = \frac{25,550 \text{ IN-LBS}}{8.26 \text{ IN}^3} = 3,093 \text{ PSI} < F_b = 0.6(19,200) = 11,520 \text{ PSI}$$

SHEAR ON LUG

$$V = 3650^{\#} \quad A = 5.5'' (0.375'') = 2.06 \text{ IN}^2$$

$$f_v = \frac{V}{A} = 1772 \text{ PSI} < A F_v = A(19,200 \text{ PSI}) = 7,680 \text{ PSI}$$

SUBJECT	OFFICE		REVISION		REFERENCE NO.
		GBI	NOE		
	MADE BY	CHKD BY	MADE BY	CHKD BY	SHT 22 OF 35
	DATE	DATE	DATE	DATE	11.22
	3/2/94	4/6/94			

TORSION ON LUG

$$T = \sqrt{\left[\frac{(t_1 + t_2)}{2}\right]^3 + 1''} = 3650 \left[\frac{(.375 + .1875)}{2} + 1''\right] = 4677 \text{ M-LBS}$$

$$\text{TORSIONAL RESISTANCE (R)} = \frac{bd^3}{3} = \frac{(5.5'')(3.75'')^3}{3} = 0.10$$

$$\theta = \frac{TL}{E_s R} \quad (\text{RAD})$$

$$\tau = \frac{T}{bd^2/3} \quad (\text{PSI})$$

$$E_s = 12,000,000 \text{ PSI}$$

$$@ L = 2''$$

$$\theta = \frac{(4677 \text{ M-LBS})(2'')}{(12,000,000)(0.10)} = 0.0078 \text{ RAD} = .447^\circ$$

$$\tau = \frac{4677}{(5.5)(3.75^2/3)} = 18,141 \text{ PSI} > .4F_y$$

ATTACH A CHANNEL TO STIFFEN AGAINST TORSION

TRY 3 x 4.1

$$\tau = \frac{T}{2[A]t_s}$$

$$[A] = (3 - .273)(1.41 + \frac{.17}{2} + \frac{1.375}{2}) = 4.12 \text{ in}^2$$

$$\tau = \frac{4677}{2[4.12](.17'')} = 3,335 \text{ PSI} < .4F_y = 7,680 \text{ PSI}$$

SUBJECT	OFFICE	REVISION		REFERENCE NO.
	CBI DOE			930212
MADE BY	CHKD BY	MADE BY	CHKD BY	SHT 23 OF 35
DATE	DATE	DATE	DATE	11.23
	02/94	4/6/94		

HANGER STRAP DESIGN

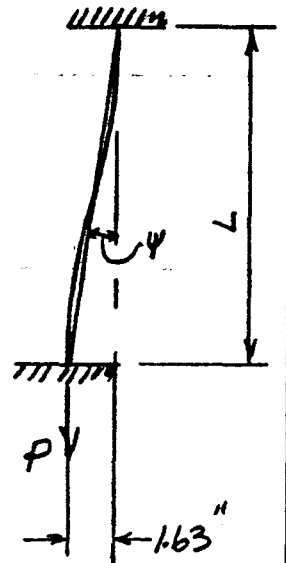
USING THE SLOPE - DEFLECTION METHOD

$$M_{TOP} = \frac{2EI}{L} [2\theta_{TOP} + \theta_{BOT} - 3\psi]$$

$$M_{BOT} = \frac{2EI}{L} [2\theta_{BOT} + \theta_{TOP} - 3\psi]$$

LET ROTATION @ TOP (θ_{TOP}) = 0
 ROTATION @ BOTTOM (θ_{BOT}) = 0

$$M_{TOP} = M_{BOT} = \frac{-6EI}{L} \psi$$



USE A572 GR. 50 MATERIAL FOR STRAPS $F_y = 50,000 \text{ PSI}$

TRANSLATION = 1.63"

$$P_{max} = 3178 \text{ lb}$$

(BASE-CASE) ← GOVERNS

$$P_{max} = 3677 \text{ lb}$$

(SEISMIC)

TRY $2" \times \frac{3}{16}"$ STRAP

$$A = 2(.1875") = .375 \text{ in}^2 \quad I = \frac{2(.1875")^3}{12} = 0.0011 \text{ in}^4 \quad S = \frac{2(.1875")^2}{6} = 0.0117 \text{ in}^3$$

$$E = 29,000,000 \text{ PSI}$$

$$\text{LET } L = 32" \quad \psi = \frac{1.63}{32} = 0.05094 \text{ RAD.} = 2.919^\circ$$

SUBJECT	OFFICE		REVISION		REFERENCE NO.
	GBI NOE				930212
	MADE BY	CHKD BY	MADE BY	CHKD BY	SHT 24 OF 25
	MRS	DJG			
DATE	DATE	DATE	DATE		
3/2/94	4/6/94			11.24	

$$M_{top} = \frac{6EI}{L} \psi$$

$$= \frac{6(29,000,000)(.001)}{32'} (.05094)$$

$$= 305 \text{ IN-LBS}$$

$$f_a = \frac{P}{A} = \frac{3178^{**}}{0.375 \text{ in}^2} = 8,475 \text{ PSI}$$

$$f_b = \frac{M}{S} = \frac{305 \text{ IN-LBS}}{0.0117 \text{ in}^3} = -26,068 \text{ PSI}$$

$$F_a = 0.6F_y = .6(50,000) = 30,000 \text{ PSI}$$

$$F_b = 0.75F_y = .75(50,000) = 37,500 \text{ PSI}$$

$$\frac{f_a}{F_a} + \frac{f_b}{F_b} \leq 1.0$$

$$\frac{8475}{30,000} + \frac{26,068}{37,500} = 0.28 + 0.70 = 0.98 < 1.0 \quad \text{OK}$$

SUBJECT	OFFICE CBI NOE		REVISION		REFERENCE NO. 930212
	MADE BY MRS	CHKD BY DDG	MADE BY	CHKD BY	SHT 25 OF 35
	DATE 3/2/94	DATE 4/6/94	DATE	DATE	11.25

DESIGN OVERHEAD BEAM

$$M = 6'' (3178) = 19,068 \text{ IN-LBS}$$

TRY 2 C3x4.1

$$A = 2.42 \text{ IN}^2$$

$$S = 2(1.10) = 2.20 \text{ IN}^3$$

$$d/A_F = 7.78$$

$$f_b = \frac{M}{S} = \frac{19,068}{2.20} = 8,667 \text{ PSI}$$

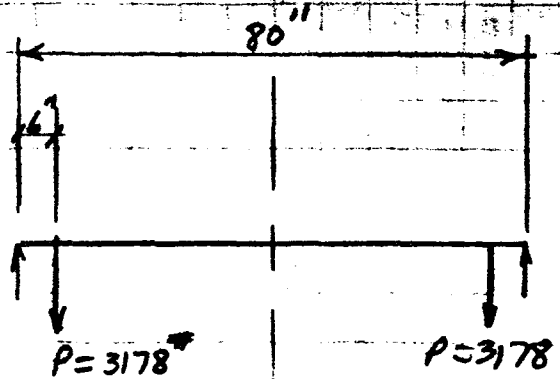
(AISC F3)

$$F_b = \frac{12000 C_b}{l d/A_F}$$

$$C_b = 1.0 \quad l = 80'$$

$$F_b = \frac{12000}{80(7.78)} = 19.28 \text{ KSI}$$

$f_b < F_b$ OK



SUBJECT

OFFICE NOE		REVISION		REFERENCE NO. 930212
MADE BY MRS	CHKD BY DDG	MADE BY	CHKD BY	SHT 26 OF 35
DATE 3/2/94	DATE 4/6/94	DATE	DATE	11-26

COLUMN DESIGN

$$P = 3178^* \text{ MAX (BACK OUT CONTROLS)}$$

TRY \square TUBE $4 \times 4 \times \frac{3}{8}$

$$A = 5.08 \text{ IN}^2 \quad I = 10.7 \text{ IN}^4 \quad S = 5.35 \text{ IN}^3 \quad r = 1.45''$$

LET $K = 2.1$ (CANTEVER)

$$L = 50''$$

$$KL/r = \frac{(2.1)(50)}{1.45} = 72.4$$

$$F_a = 16,120 \text{ PSI}$$

$$f_a = \frac{3178^*}{5.08 \text{ IN}^2} = 626 \text{ PSI} < F_a \text{ } \circ \circ \text{ OK}$$

BENDING IN COLUMN DUE TO LONGITUDINAL LOAD

$$P_L = P \sin 2.919^\circ = 3178^* (0.0509) = 162^*$$

$$M = 50'' (162^*) = 8100 \text{ IN-LBS}$$

$$f_b = \frac{8100 \text{ IN-LBS}}{5.35 \text{ IN}^3} = 1514 \text{ PSI}$$

$$\Delta = \frac{P_L^3}{3EI} = \frac{(162^*)(50)^3}{3(29,000,000)(10.7)} = 0.022''$$

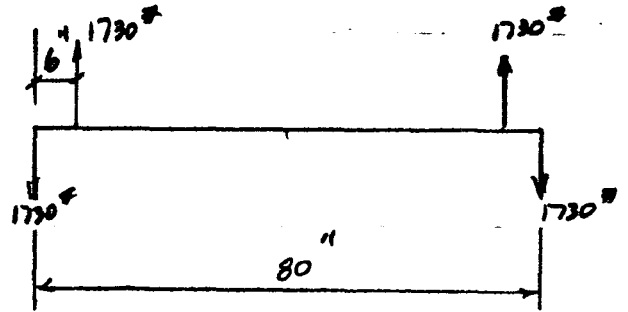
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DESIGN LOWER LONGITUDINAL BEAM

MAXIMUM LATERAL LOAD = 1730* SEISMIC, 600* STABILITY

$$M_{max} = 6'' (1730^*) = 10,380 \text{ in-LBS}$$

TRY 2-C3x4" (BACK TO BACK)



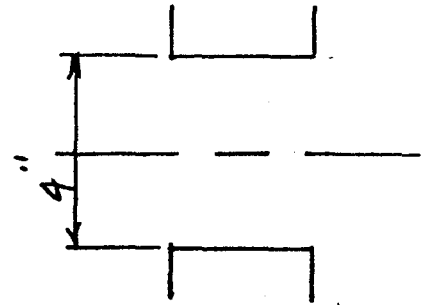
$$A = 2(1.21) = 2.42 \text{ in}^2$$

$$I = 2(.197) + 2(1.21)(2 + .436'')^2 = 14.75 \text{ in}^4$$

$$S = \frac{I}{c} = \frac{14.75 \text{ in}^4}{2 + 1.41''} = 4.32 \text{ in}^3$$

$$f_b = \frac{m}{s} = \frac{10,380 \text{ in-LBS}}{4.32 \text{ in}^3} = 2,403 \text{ PSI}$$

$$F_b = 1.33(.6 F_y) = 28,800 \text{ PSI}$$



TRY 4x4x3/8 STRUCTURAL TUBE

$$A = 5.08 \text{ in}^2 \quad I = 10.7 \text{ in}^4 \quad S = 5.35 \text{ in}^3$$

TUBE IS OK BY INSPECTION

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DESIGN TRANSVERSE BEAM

$$M = 3178 (11.5) = 36,547 \text{ in-LBS}$$

TRY $4 \times 4 \times \frac{3}{8}$ TUBE

$$S = 5.35 \text{ in}^3$$

$$f_b = \frac{36,547}{5.35} = 6,831 \text{ PSI}$$

CHECK FLEXIBILITY OF COLUMN

APPLY $600^{\#}$ LOAD @ TOP OF COLUMN.

$$M_{RL} = 60(60) = 36,000 \text{ in-LBS}$$

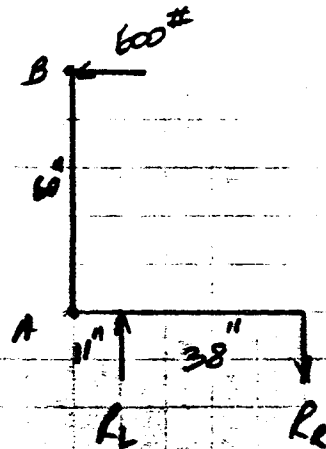
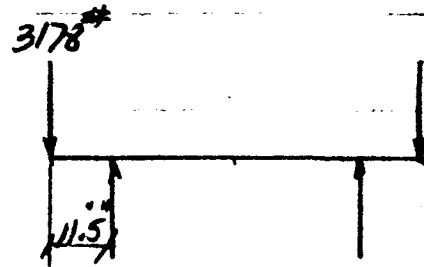
$$\theta_{CA} = \frac{ML}{3EI} = \frac{36,000(38)}{3(29,000,000)(1.27)}$$

$$\theta = .0017 \text{ RAD}$$

$$\Delta_b = 60(.0017) = .102$$

$$\Delta_b = \frac{WL^3}{3EI} = \frac{60(60)^3}{3(29,000,000)(1.27)} = 0.139$$

$$\Delta_{TOTAL} = .139 + .088 = 0.227$$



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SUPPORT LUG DESIGN

USE SAME LUG AS THE FIXED SUPPORT

HANGER BOLT DESIGN

$P_{MAX} = 3650^{\#}$ (SEISMIC)

USE 2 BOLTS IN SINGLE SHEAR

ALLOWABLE SHEAR PER BOLT = 4.4^k

TOTAL = $8.8^k > 3.65^k$ ϕ OK

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LATERAL RESTRAINT CABLES

LATERAL LOADS = 1730[#] (SEISMIC)
 = 600[#] (NORMAL)

LONGITUDINAL PIPE MOVEMENT = 1.63" MAX

TRY 1/4" ϕ 7x19 IPS IWRC GALVANIZED CABLE⁺
 OR 6x19

MIN. BREAKING STRENGTH = 7000[#] (CATALOG NO. 11, SAVA INDUSTRIES, INC)

FACTOR OF SAFETY (FS) = $\frac{7000}{600} = 11.7$ (NORMAL)

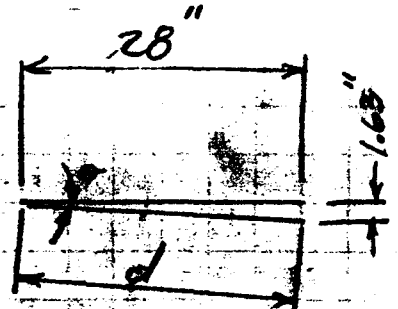
FACTOR OF SAFETY (FS) = $\frac{7000}{1730} = 4.0$ (SEISMIC)

ADDITIONAL LOAD DUE TO BAKE-OUT

L = 28"

$d = \sqrt{(28)^2 + (1.63)^2} = 28.0474"$

$\theta = 3.3317^\circ = .05815$ RAD



E = 13,000,000 PSI (WIRE ROPE ENGINEERING HANDBOOK)

$\Delta = 0.0474"$

WIRE METAL AREA = 0.0341 in² 1/4" ϕ

$P = \frac{A E \Delta}{L} = \frac{(0.0341 \text{ in}^2)(13,000,000 \text{ PSI})(0.0474")}{28"} = 1750[#]$

FS = $\frac{7000}{1350} = 5.2$ (BAKE OUT)

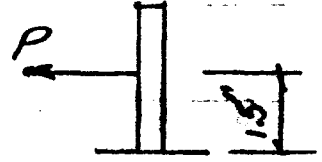
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RESTRAINT LUG DESIGN

$$P = 1730^{\#} \text{ (SEISMIC)}$$

$$P = (600^{\#} + 750^{\#}) = 1350^{\#} \text{ (BACK-OUT)}$$



$$M = (1730^{\#})(2") = 3460^{\#}$$

TRY 3" x 3" x 7/8" R

$$S = \frac{(6.25)^2 (3)}{6} = 0.195 \text{ IN}^3$$

$$f_b = \frac{3460}{0.195} = 17,744 \text{ PSI}$$

PUT 7/8" GUSSET TO SUPPORT MOMENT

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DESIGN BOLTED RING LUG CONNECTION

TRY 3 BOLTS

$$f_{vm} = (3650^*)(9.5'') / 9'' = 3,853^*$$

$$f_v = \frac{3650}{3} = 1217^*$$

$$f_{vt} = \sqrt{f_{vm}^2 + f_v^2}$$

$$f_{vt} = 4,041^* / \text{BOLT}$$

$$\text{ALLOWABLE LOAD} = 4,400^* / \text{BOLT}$$

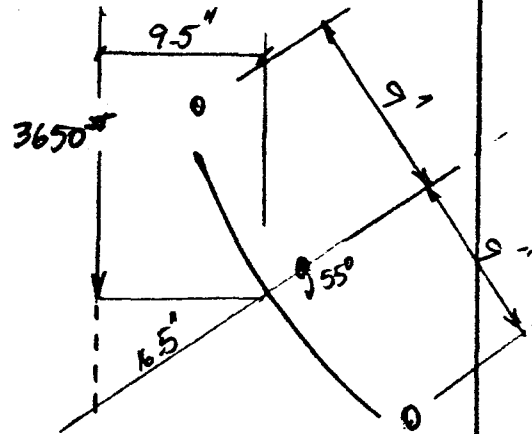
TRY 4 - 3/4" ϕ BOLTS

PER AISC TABLE II, A 4-62

$$P = C \times r_v \quad n=4 \quad l=9''$$

$$r_v = 4400^* \quad C=1.21$$

$$P = 1.21(4400^*) \leq 5,324^* > 4,041^* \quad \text{OK}$$



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DESIGN OUTER FRAME VERTICAL BOLTED CONNECTION

ASSUME 600# LATERAL LOAD @ TOP OF COLUMN

$$M = (600\#)(58.5') = 35,100 \#-ft$$

TRY 2 BOLTS @ 9" APART

$$P_{\text{BOLT}} = \frac{35,100 \text{ IN-LBS}}{9"} = 3900 \#/\text{BOLT}$$

PER AISC 3/4" ϕ BOLT $P_{\text{ALLOW}} = 8.8 \text{ K} > P \text{ } \checkmark \text{ OK}$

BASE PLATE

$$e = 2.5"$$

$$M = 2.5" (3649\#) = 9,123 \text{ IN-LBS} \quad \text{USE } 10,000 \text{ IN-LBS}$$

$$S_{\text{REQ'D}} = \frac{M}{.75F_y} = \frac{10,000}{.75(36,000)} = .371 \text{ IN}^3$$

$$S = \frac{bt^2}{6}$$

$$.371 \text{ IN}^3 = \frac{(4")t^2}{6}$$

$$t = 0.746"$$

USE 1" R

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CHECK TUBE WALL FOR BOLT LOAD

CHECK TUBE WALL FOR BOLT YIELD LOAD

$$P = A_b F_y = (0.4418 \text{ in}^2) (36,000 \text{ PSI}) = 15,905 \text{ \#}$$

REF. "FORMULAS FOR STRESS & STRAIN", ROARK 4TH ED., P 226, CASE 38

ASSUME 4" SQ. PLATE @ 1" LOAD AREA

$$a = b = 4" \quad a_1 = b_1 = 1"$$

$$b_1/b = .25 \quad a_1/b = .25$$

$$\beta = 1.18$$

3/8" WALL t


$$S_b = \beta \frac{W}{t^2} = 1.18 \frac{16,000 \text{ \#}}{(.375)^2} = 139,260 \text{ PSI}$$

SOLVE FOR t @ $S_{ALLOW} = .75 F_y = 27,000 \text{ PSI}$

$$27,000 = 1.18 \frac{(16,000)}{t^2}$$

$$t_{REQ'D} = 0.837"$$

USE 4" x 4" x 1" SQ. WASHER PLATES

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Thermal Analysis of Ligo Supports

Prepared for
Ligo Beam Tube Design and
Qualification Test - CalTech

by

CBI Technical Services Company

Plainfield, Illinois

CBI Contract Number 930212

March 1994

SUBJECT Ligo Heat Transfer Analysis	MADE BY DEP	CHKD BY KAD	REV	BY	CHARGE NO. 930212
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**I. INTRODUCTION:**

This report documents the finite element heat transfer analysis of the Ligo tube system, near the fixed supports. This thermal analysis determines the temperature drop of the Ligo tube near the fixed supports during bake-out conditions.

The Ligo structure consists of a tube approximately four kilometers in length. A current within the tube wall maintains a tube temperature of 284.0°F away from the supports, during bake-out. While heating of the tube occurs, the supports act as a heat sink. This analysis determines the minimum tube wall temperature, due to the sink effects of the supports; this tube wall temperature determines the acceptability of the insulation system at the supports. The minimum acceptable tube wall temperature is 266.0°F.

Included in this report is a description of model geometry, finite element modelling of the same, material properties, thermal loads, thermal boundary conditions, and temperature distribution results.

II. SUMMARY OF RESULTS:

Figures 1 through 4 show the support details. Separate finite element models determine the tube wall temperature at the transfer lug and column supports. The minimum tube wall temperatures occur at these locations. The minimum tube wall temperatures at these locations are greater than the specified minimum value of 266.0°F.

The minimum tube wall temperature occurs at the column support. While not shown in the attached figures, two inches of duct wrap insulation covering the plates, micarta, and column is required to meet the minimum tube wall temperature requirement. The transfer lug support detail shown in the attached figures is adequate.

III. METHOD OF ANALYSIS:**A. Rationale of Separate Point Support Models**

Figure 1 shows a typical three point fixed support for the Ligo tube. Both the Ligo tube and supports are symmetric about the $x=0$ plane shown in Figure 1. Two distinct heat sinks exist due to the presence of the support, one at the transfer lug location, and one at the column support location. Since the supports are about twenty-five inches apart along the circumference of the tube, these heat sinks are assumed to act independently of each other. This analysis uses separate finite element models, called the transfer lug model and column support model, respectively, to analyze each location. Figures 5 and 6 show the geometry of the transfer lug and column support models, respectively.

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B. Rationale of Axisymmetric Analysis

Both finite element models are axisymmetric, with the z-direction the axis of symmetry. The main path of heat flow from the tube is through the transfer lug or column support, the horizontal structure tube, and the support lug assembly to the concrete foundation. The axisymmetric modelling of the non-axisymmetric support components utilizes smearing of the thermal conductivities based on cross sectional area of the component versus modelled cross sectional area along the flow path. Similarly, axisymmetric modelling of convective film coefficients utilizes smearing based on exposed surface area ratios.

C. Axisymmetric Analysis of a Concentrated Heat Sink

The transfer lug and column support models determine which of the two locations produces the minimum tube wall temperature, and also the total heat flux through this governing heat sink. The governing total heat flux is then used in an additional axisymmetric model to more accurately determine the minimum tube wall temperature. Figure 7 shows the concentrated heat sink model geometry.

D. Acceptance Criteria

The maximum temperature in the tube is 284.0°F. The minimum tube temperature is 266.0°F. This is a pure temperature requirement, not a requirement based on a maximum temperature drop in the tube.

E. Computer Programs

The ADINA-T computer program determines the temperature distribution in the tube, insulation, stiffener, and support components. ADINA-T (Automatic Dynamic Incremental Non-linear Analysis of Temperatures) is a general purpose, commercially available computer program for the solution of linear or non-linear, steady-state or transient, finite element analysis of heat transfer and field problems. ADINA R&D, Inc., of Watertown, Massachusetts developed and maintains the ADINA-T program.

PATRAN Plus software generates the finite element models and presents the analysis results. PATRAN Plus is a pre- and post- processing software package developed and maintained by PDA Engineering of Costa Mesa, California. PATRAN is an open-ended (solver independent), general purpose, commercially available, engineering software package that utilizes interactive graphics to generate finite element models and evaluate and present analysis results.

F. Assumptions

The analysis uses the assumptions shown below.

- Steady state heat transfer analysis. The analysis assumes no transient effects.
- No thermal gradient exists at the model end away from the support.

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- Symmetry exists along the z=0 plane for both models.
- No thermal gradient exists across the plane of symmetry.
- Smearing of thermal conductivities used to reflect true material cross sectional area of non-axisymmetric support components.
- Smearing of convective film coefficients used to reflect true exposed surface area of non-axisymmetric support components.
- Transfer lug and column support heat sinks act independently of each other.
- Free convection acts on all exposed surfaces. Analysis includes support insulation if initial results require refinement.
- Convective film coefficients are a function of temperature.
- Thermal conductivity of micarta is constant. Other thermal conductivities are a function of temperature.
- Tube wall thickness intentionally modelled as 0.125 inches. The actual thickness of the tube wall is 0.127 inches.
- Ambient temperature used for prescribed temperature of concrete.
- Analysis does not include the teflon fabric.

IV. FINITE ELEMENT MODEL DESCRIPTION:

A. Geometry

Figures 1 through 4 show the geometry of a typical fixed support. Figures 5 and 6 show the geometries used for the transfer lug and column support models, respectively. The dimensions shown in Figures 1 through 4, along the heat flow path and beyond the micarta insulation (foundation side of the support), are used to establish the smeared geometry used in the axisymmetric modelling.

The extents of the tube and duct wrap insulation modelled away from the support is sixty inches. Boundary condition effects at this location have no effect on the temperature distribution at the support.

Figure 7 shows the geometry of the concentrated heat sink model. This model includes sixty inches of tube and insulation.

B. Finite Element Mesh

Figures 8 through 10 show the finite element mesh of the transfer lug, column support, and concentrated heat sink models, respectively. All models use eight-noded axisymmetric elements. The figures show the materials used for the respective analyses.

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C. Material Properties

Table 1 shows the base thermal conductivities for the materials used in the analysis. With the exception of micarta, all thermal conductivities are a function of temperature. Table 2 shows the smearing factors used in the analyses. The thermal conductivities of the analyses are the base values times the smearing factors. Appendix A contains the calculations of the conductivity smearing factors for the transfer lug and column support analyses.

D. Thermal Loads

The lone heat source in all models is an applied distributed heat flux which simulates the effect of the electric current in the tube. Away from the support, the temperature of the tube is 284.0°F. Appendix B contains the calculations used to determine the approximate required value of this heat source, given an insulation thickness and free convection to ambient conditions. This calculation considers one constant thermal conductivity value of the insulation per iteration; therefore, this calculation is approximate. The required heat source is about 20.05 BTU/hr-in, or 15,640 BTU/hr for each sixty-five foot section of tube.

E. Thermal Boundary Conditions

Both models have prescribed temperatures assigned at the support lug end, where it attaches to the concrete foundation. The temperature value is the same used for the ambient conditions. In the analyses, the ambient temperature is 70°F.

Free convection to ambient air is assigned to all exposed surfaces. Figures 1 through 4 do not include insulation details for the supports. The analyses consider support insulation details only if the conservative non-insulated approximation yields unacceptable results. The base convective film coefficients are a function of temperature and are shown in Table 3. Table 4 shows the smearing factors used in the analyses. The convective film coefficients used in the analyses are the base values times the smearing factors. Appendix A contains the calculations of the convective film coefficient smearing factors for the transfer lug and column support analyses. Appendix B contains the calculation used to determine the convective film coefficients shown in Table 3.

The concentrated heat sink model includes distributed heat flow out of the model at the heat sink location. The results from the transfer lug and column support models determine the magnitude of this heat sink. This model contains no prescribed nodal temperatures.

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V. INITIAL RESULTS AND ANALYSIS REFINEMENT:

A. Transfer Lug Model - Initial Minimum Tube Wall Temperature

Figure 11 shows an isotherm plot of the temperature distribution in the tube, stiffener, and support components. The plot shows that the minimum temperature in the tube exceeds the minimum requirement of 266.0°F. The calculated minimum temperature of the tube wall is 280.3°F.

B. Transfer Lug Model - Analysis Refinement

An axisymmetric analysis assumes that no heat flow occurs in the circumferential direction. Some heat flow occurs in the Ligo structure circumferentially around the stiffener, between the column support and transfer lug. The distance between these supports is about twenty-five inches around the circumference of the pipe. Between these supports, a location exists where no heat flow occurs circumferentially; the maximum temperature between the supports occurs at this location. It is reasonable to assume that this maximum occurs about half way between the supports.

From the temperature distribution shown in Figure 11, the tube wall temperature about twelve and one-half inches away from the support is about 282.5°F. To conservatively estimate the effect of potential interaction of the heat sink supports, the heat flow input is revised, such that the maximum pipe temperature is reduced from 284.0°F to 282.5°F. This assumption is conservative, since the maximum tube wall temperature of 284.0°F occurs in the Ligo structure along the length of the tube, as well as the top of the tube.

C. Transfer Lug Model - Refined Analysis Results

Figure 12 shows an isotherm plot of the revised temperature distribution in the tube, stiffener, and support components. The plot shows that the minimum temperature in the tube exceeds the minimum requirement of 266.0°F. The calculated minimum temperature of the tube wall is 279.2°F.

D. Column Support Model - Initial Minimum Tube Wall Temperature

Figure 13 shows an isotherm plot of the temperature distribution in the tube, stiffener, and support components. The plot shows that the minimum temperature in the tube is below the minimum requirement of 266.0°F. The calculated minimum temperature of the tube wall is 257.9°F.

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E. Column Support Model - Analysis Refinement

Since the minimum temperature of the tube wall does not meet the minimum temperature requirement, the model refinement includes two inches of duct wrap insulation covering horizontal and vertical plates, micarta, and column support. This refinement uses equivalent convective film coefficients considering both conduction through the insulation and free convection to ambient air. Appendix C contains this calculation.

F. Column Support Model - Refined Analysis Results

Figure 14 contains an isotherm plot of the refined analysis. Figure 14 shows that the minimum temperature in the tube exceeds the minimum requirement of 266.0°F. The calculated minimum temperature of the tube wall is 271.2°F.

An additional one degree temperature drop is assumed due to interaction effects between the two supports. The resulting minimum temperature of 270.2°F is still above the required minimum value. This is verified by the concentrated heat sink model.

G. Concentrated Heat Sink Model - Initial Minimum Tube Wall Temperature

The minimum tube wall temperature occurs at the column support. Therefore, the heat loss is higher at the column support, than at the transfer lug. Appendix C contains the calculations for the heat loss at the column support, and the equivalent heat loss applied in the concentrated heat sink model.

Figure 15 contains an isotherm plot of the temperature distribution in the tube. This figure shows that the minimum temperature in the tube exceeds the minimum requirement of 266.0°F. The calculated minimum temperature of the tube wall is 270.3°F.

H. Concentrated Heat Sink Model - Analysis Refinement

The revision of the heat flow in the tube is similar to the technique used in the transfer lug analysis. The maximum tube wall temperature is 280.3°F in the revised analysis. No change is made in the heat loss rate.

I. Concentrated Heat Sink Model - Refined Analysis Results

Figure 16 contains an isotherm plot of the temperature distribution in the tube. This figure shows that the minimum temperature in the tube exceeds the minimum requirement of 266.0°F. The calculated minimum temperature of the tube wall is 266.7°F.

Since the analysis of the concentrated heat sink model yields acceptable results, the insulation detail of the column support region of the support is adequate. The heat loss is greater at the column support than at the transfer lug. The insulation detail at the transfer lug is also adequate.

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VI. CONCLUSIONS:

The insulation details at the transfer lug and column support analyzed in the refined analyses are adequate. The minimum temperature in the tube wall occurs at the column support. This minimum calculated temperature is 266.7°F, above the required temperature of 266.0°F.

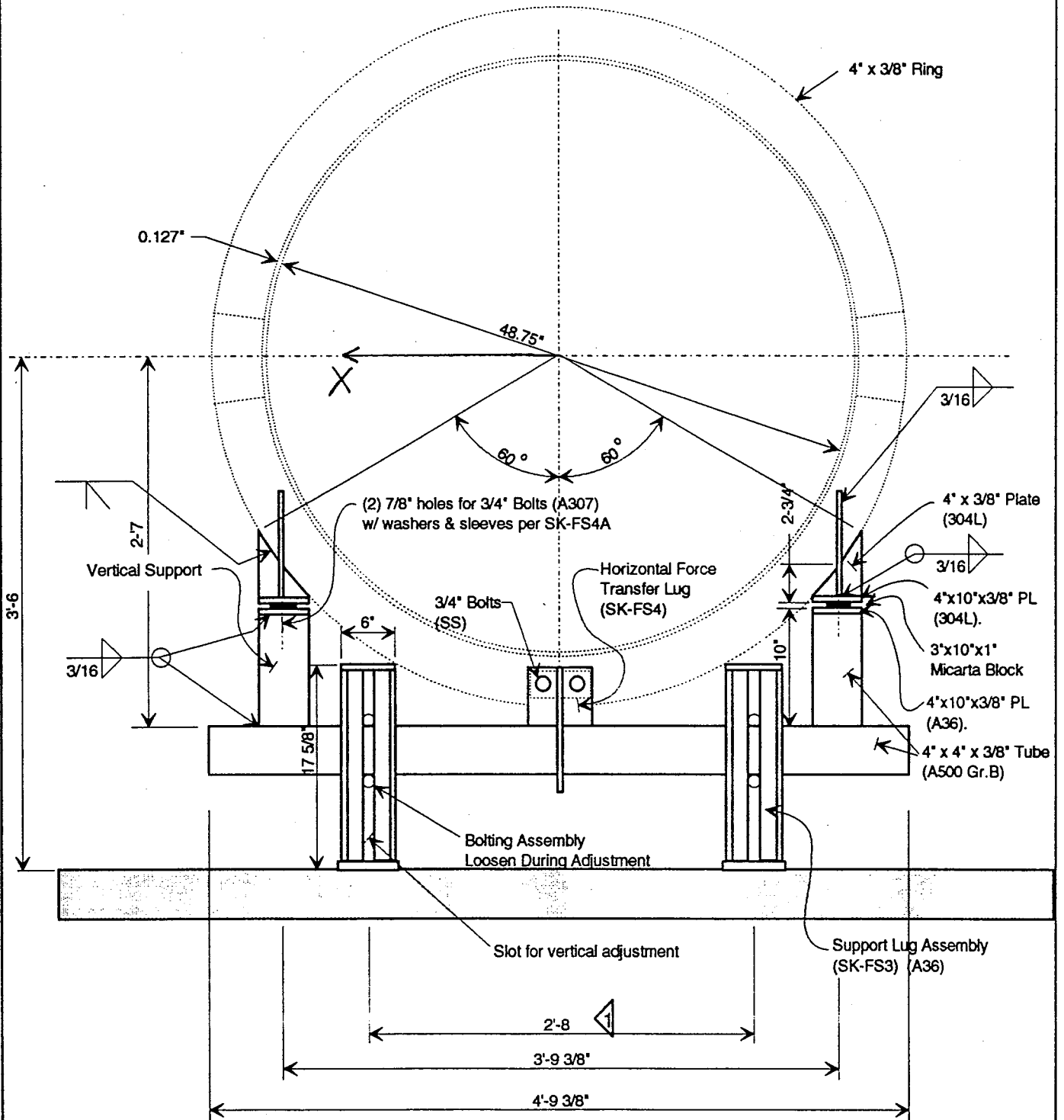
While not shown in Figures 1 through 4, two inches of duct wrap insulation covering the plates, micarta, and column is required to meet the minimum tube wall temperature requirement. The transfer lug support detail shown in the attached figures is adequate.

VII. REFERENCES:

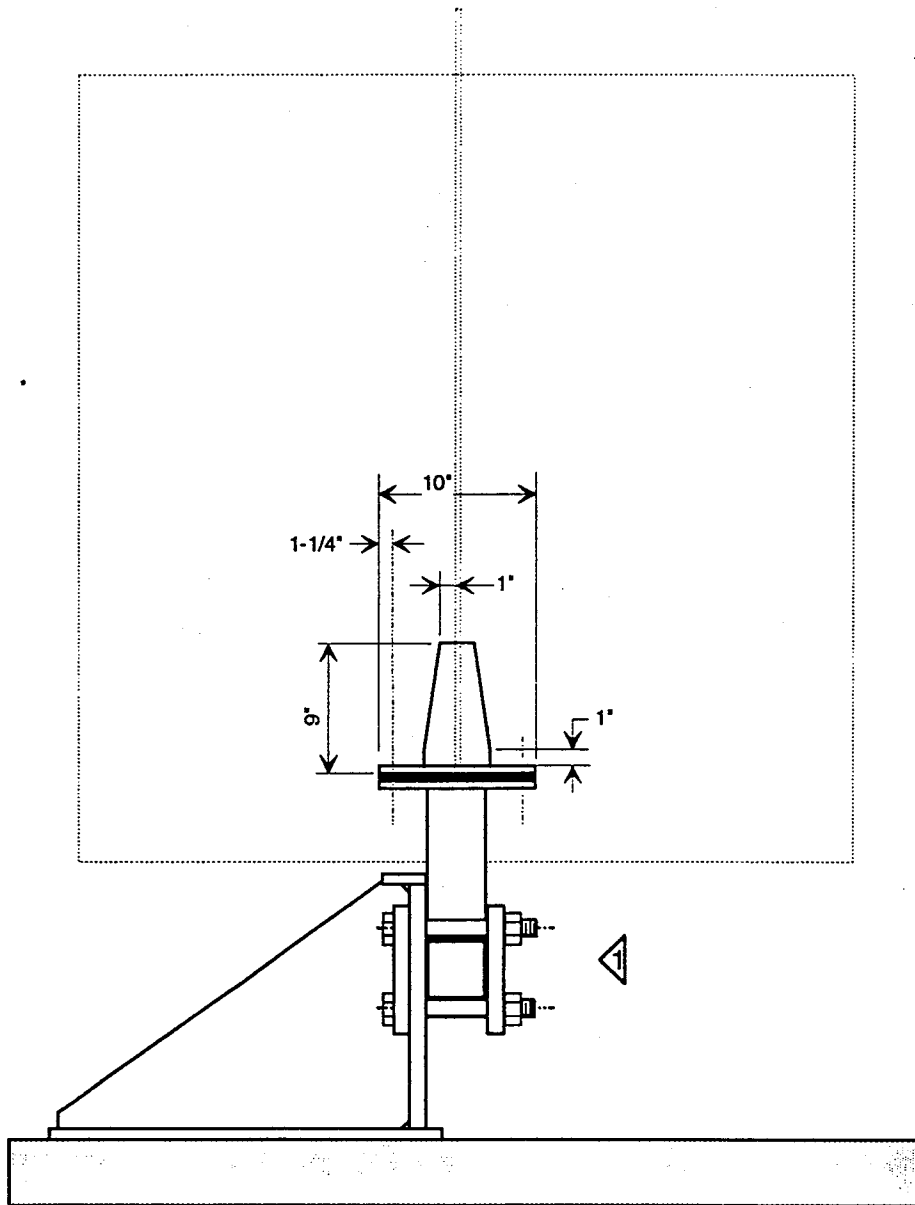
- 1) Fundamentals of Heat Transfer, F.P. Incropera, D.P. DeWitt, John Wiley and Sons, 1981 Edition.
- 2) ASME Code, Section II, Part D, Table TCD, 1992 Edition.
- 3) Krauf Duct Wrap Technical Data Manual

SUBJECT Ligo Heat Transfer Analysis CalTech	MADE BY DEP	CHKD BY KAD	BY	CHARGE NO. 930212
	DATE 3/30/94	DATE 3/31/94	CHKD	
			DATE	SHT 7 OF

Figure 1:
Ligo Support Geometry - Elevation View



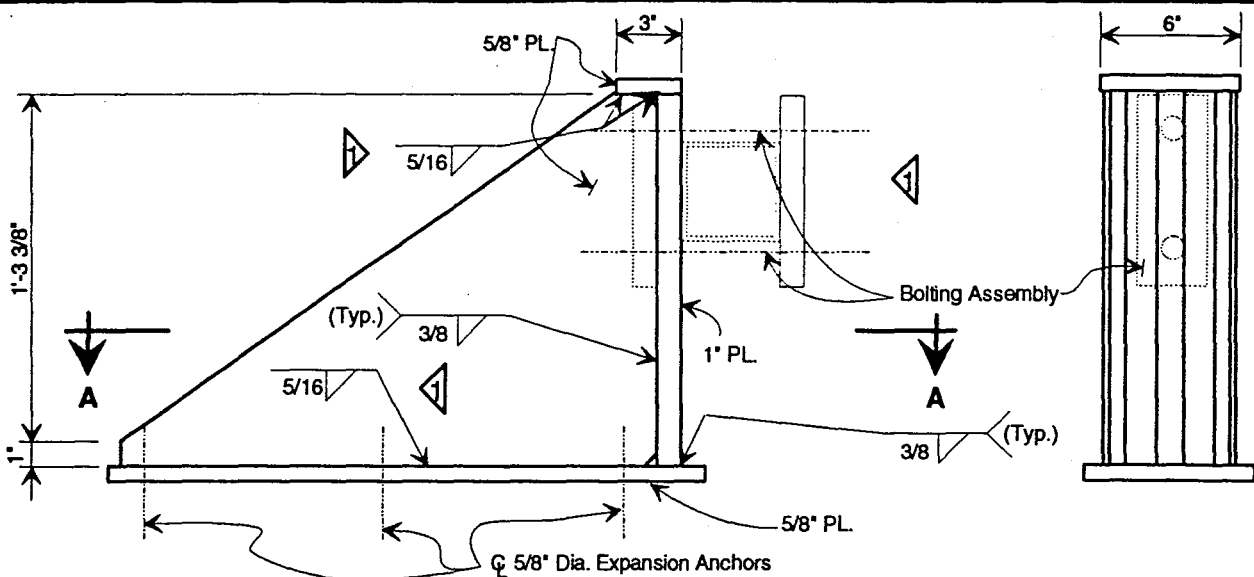
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	MADE MRS	CHK'D	MADE MRS	CHK'D WJC	SHT 8 OF
	DATE 2/22/94	DATE	DATE 3/7/94	DATE 3-30-94	SK - FS1A



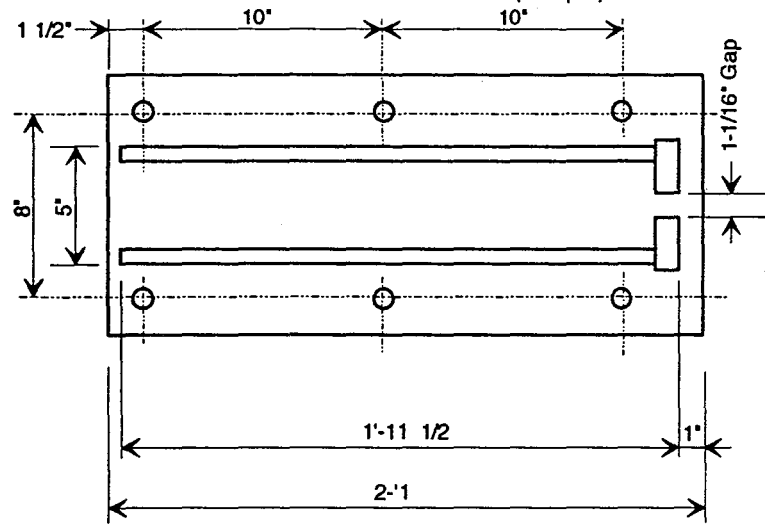
SIDE VIEW

**Figure 2:
Ligo Support Geometry - Side View**

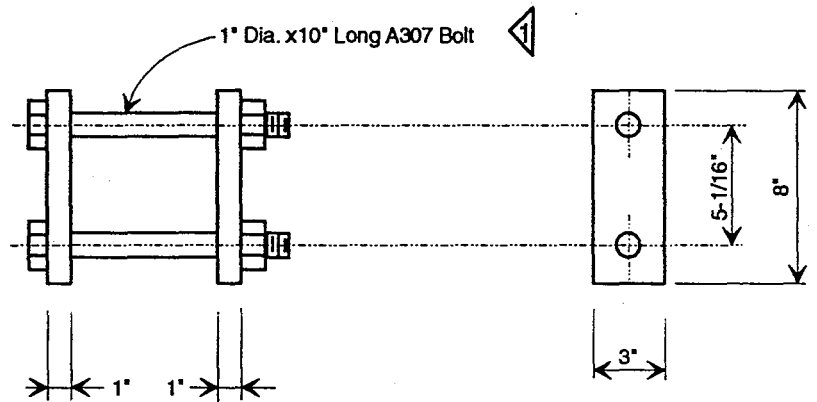
SUBJECT LIGO BEAM TUBE FIXED SUPPORT	OFFICE 		REVISION: 1		REFERENCE NO.
	MADE MRS	CHK'D	MADE MRS	CHK'D WJC	SHT <u>9</u> OF <u> </u>
	DATE 2/19/94	DATE	DATE 3/7/94	DATE 3-30-94	SK - FS2A



5/8" Dia. Expansion Anchors
 HILTI KB II 58-434
 4-3/4" Long, 2-3/4" Embed. Depth
 (or Equal)



SECTION A-A



BOLTING ASSEMBLY

Figure 3:
Ligo Support Geometry - Support Lug Assembly

SUBJECT SUPPORT LUG ASSEMBLY BEAM TUBE FIXED SUPPORT LIGO	OFFICE CBI		REVISION: 1		REFERENCE NO.
	MADE MRS	CHK'D	MADE MRS	CHK'D WJC	SHT <u>10</u> OF <u> </u>
	DATE 2/23/94	DATE	DATE 3/7/94	DATE 3-30-94	SK - FS3A

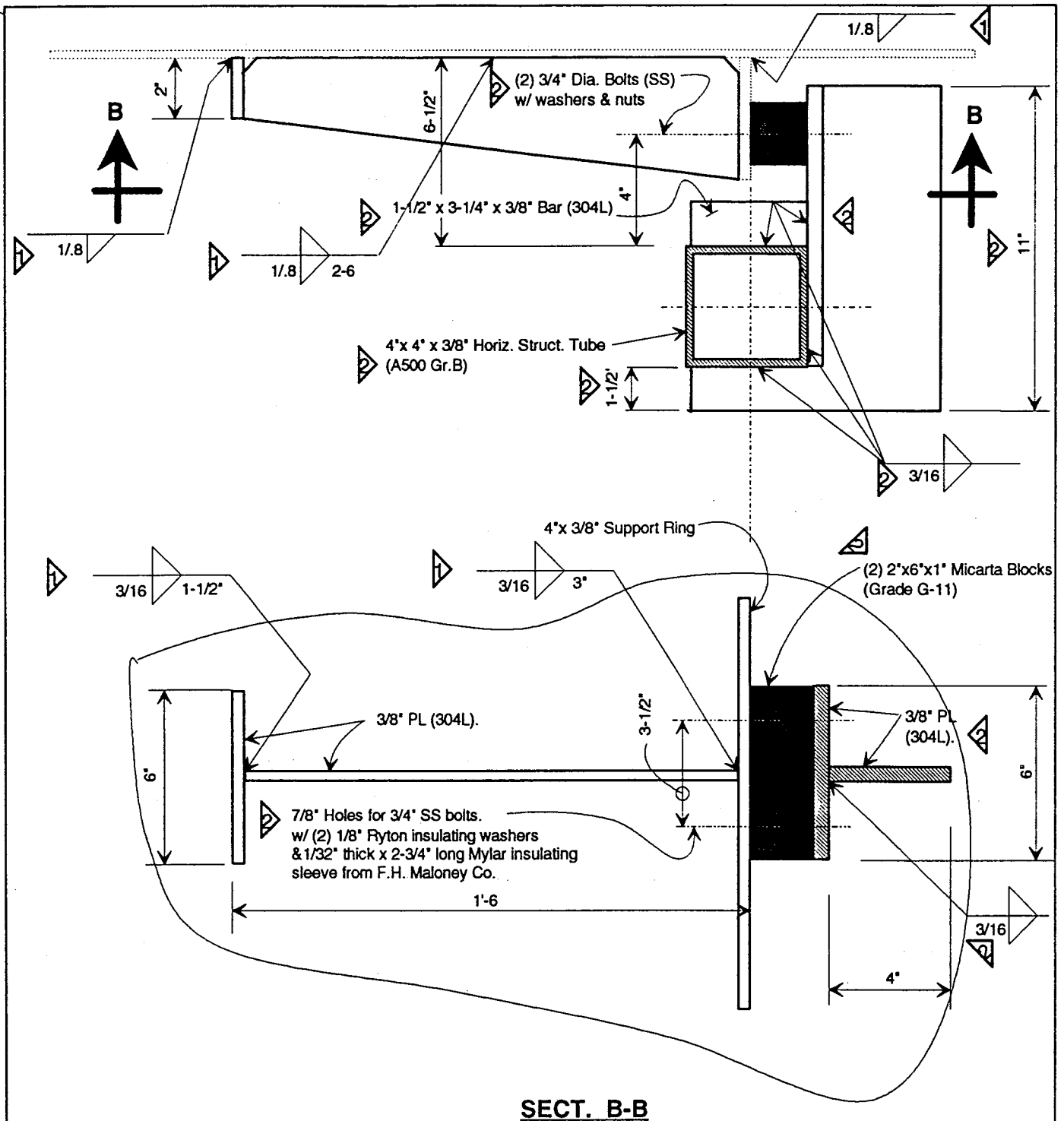
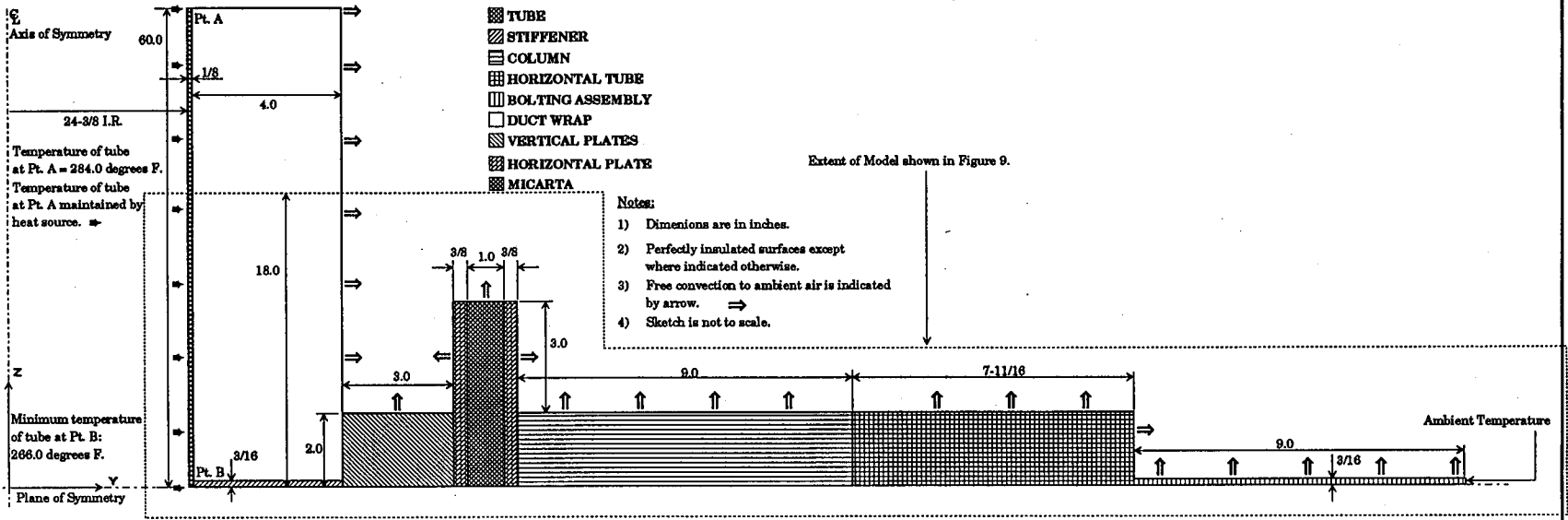


Figure 4:
Ligo Support Geometry - Transfer Lug Details

SUBJECT	OFFICE		REVISION: 2		REFERENCE NO.
	CBI				
	MADE MRS	CHK'D	MADE MRS	CHK'D WJC	SHT <u>11</u> OF <u> </u>
	DATE 2/24/94	DATE	DATE 3/7/94	DATE 3-30-94	SK - FS4A

SUBJECT
**LIGO HEAT TRANSFER
 ANALYSIS**

CBI NOE - A		OFFICE		REVISION		REFERENCE NO.	
MADE BY <i>DEP</i>	CHKD BY <i>KAD</i>	MADE BY	CHKD BY	MADE BY	CHKD BY	9302/2	
DATE 3/30/94	DATE 8/31/94	DATE	DATE	DATE	DATE	SHT 13 OF	



**Figure 6:
 Column Support Model - Geometry**

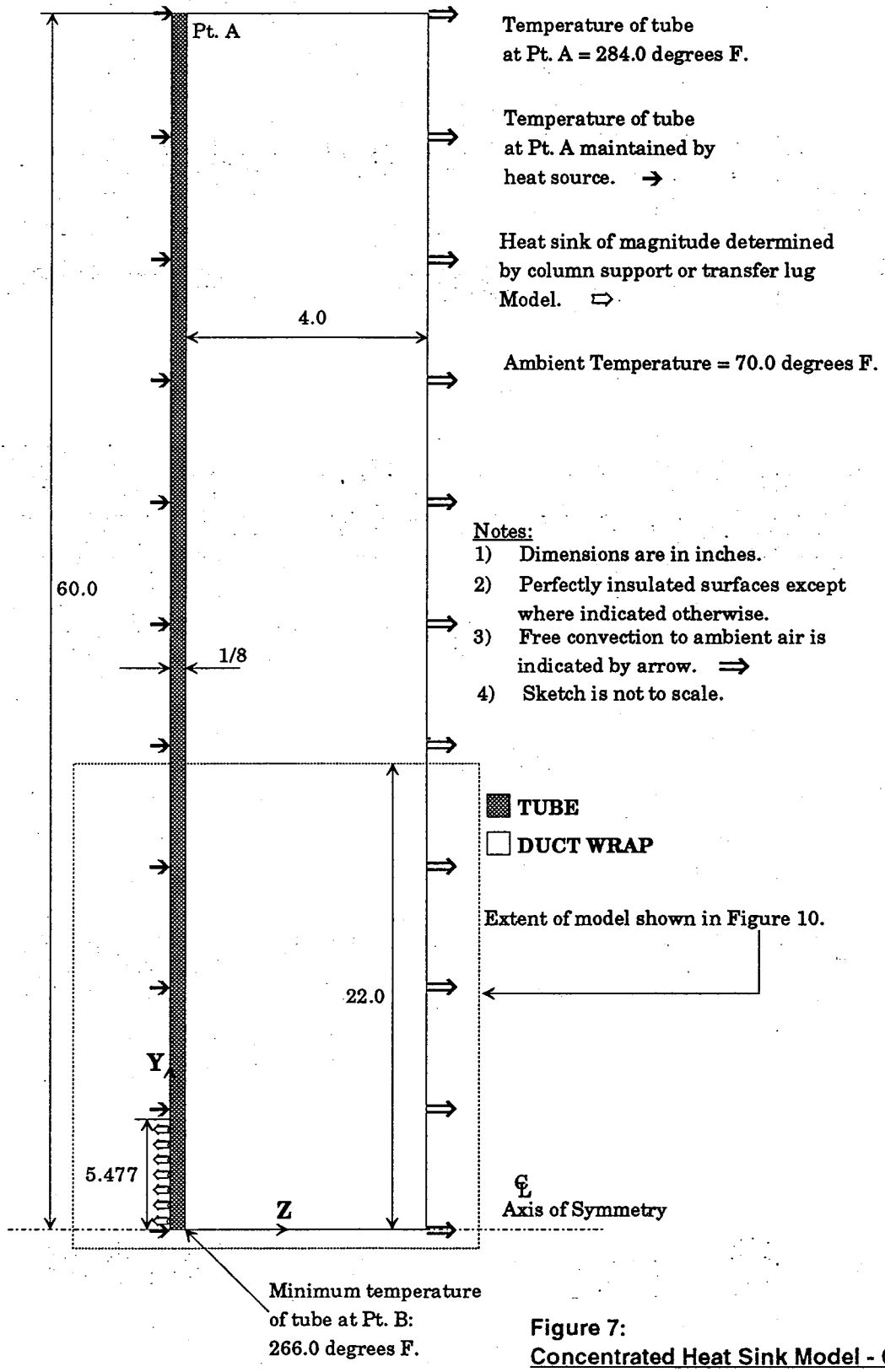


Figure 7:
Concentrated Heat Sink Model - Geometry

SUBJECT LIGO HEAT TRANSFER ANALYSIS	OFFICE CEI NOE-A		REVISION		REFERENCE NO. 930212
	MADE BY DEP	CHKD BY KAD	MADE BY	CHKD BY	SHT 14 OF
	DATE 3/30/94	DATE 3/31/94	DATE	DATE	

SUBJECT
Ligo Beam Tube Design and Qualification
Test - CalTech

MADE BY DEP	CHKD BY KAD	REV	MADE BY	CHKD BY	CHARGE NO.
DATE 3/29/94	DATE 3/29/94		DATE	DATE	930212
					OF
					15

NOTE: NOT ALL TUBE AND DUCT WRAP ELEMENTS SHOWN

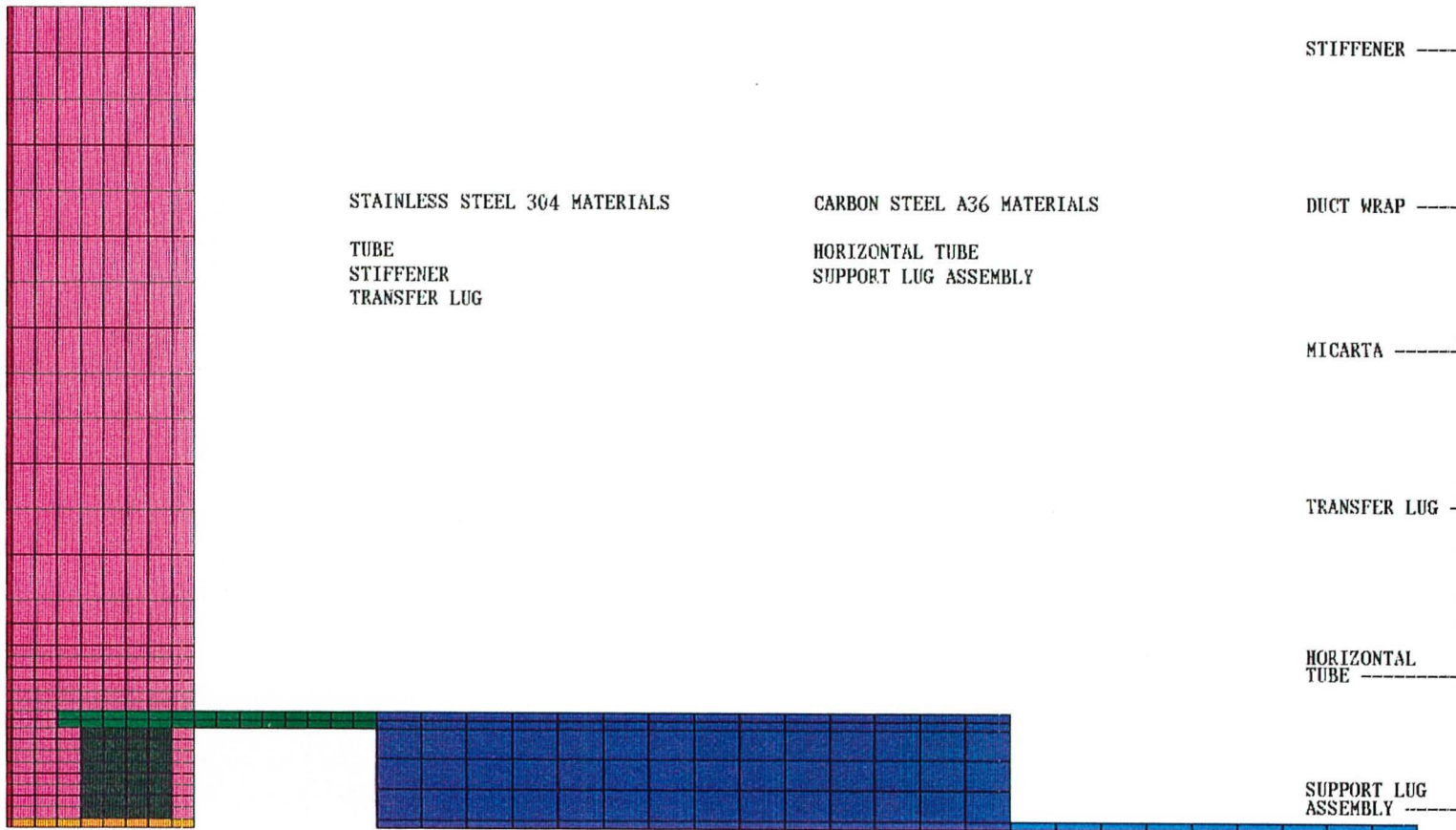
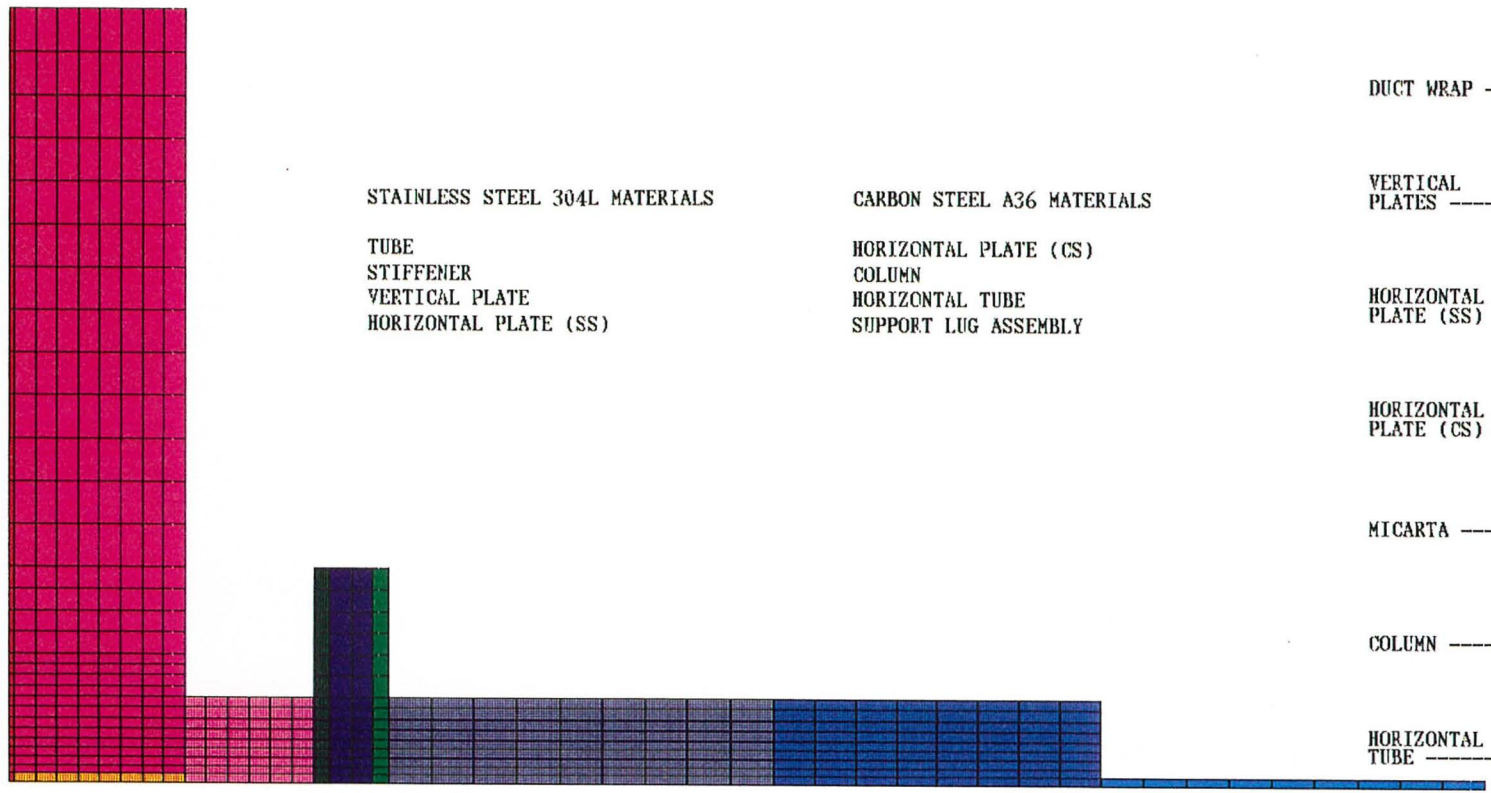


FIGURE 8
TRANSFER LUG MODEL - ELEMENT MESH

SUBJECT Ligo Beam Tube Design and Qualification Test - CalTech	MADE BY DEP	CHKD BY RAD	MADE BY	CHKD BY	CHANGE NO. 930212
	DATE 3/29/94	DATE 3/29/94	DATE	DATE	OF
	REV				SHEET 16

NOTE: NOT ALL TUBE AND DUCT WRAP ELEMENTS SHOWN



STAINLESS STEEL 304L MATERIALS	CARBON STEEL A36 MATERIALS
TUBE	HORIZONTAL PLATE (CS)
STIFFENER	COLUMN
VERTICAL PLATE	HORIZONTAL TUBE
HORIZONTAL PLATE (SS)	SUPPORT LUG ASSEMBLY

- TUBE ----- I
- STIFFENER ----- A
- DUCT WRAP ----- B
- VERTICAL PLATES ----- C
- HORIZONTAL PLATE (SS) ----- D
- HORIZONTAL PLATE (CS) ----- E
- MICARTA ----- F
- COLUMN ----- G
- HORIZONTAL TUBE ----- H
- SUPPORT LUG ASSEMBLY ----- I

FIGURE 9
COLUMN SUPPORT MODEL - ELEMENT MESH

PATMAN Delleasa 2.5-1 PHA Engineering

RED - TUBE
BLUE - DUCT WRAP

NOTE: NOT ALL TUBE AND DUCT WRAP ELEMENTS SHOWN

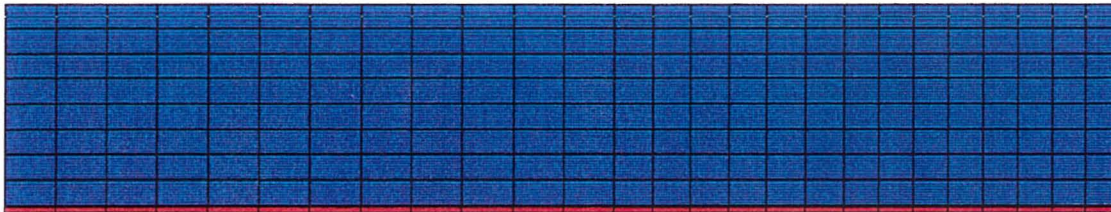
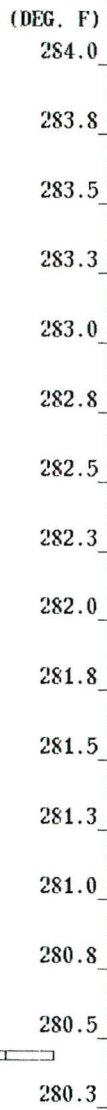


FIGURE 10
CONCENTRATED HEAT SINK MODEL - ELEMENT MESH

SUBJECT Ligo Beam Tube Design and Qualification	MADE BY DEP	CHKD BY KAD	REV	MADE BY	CHKD BY	CHARGE NO. 930212	
	DATE 3/29/94	DATE 3/29/94		DATE	DATE	SHEET 17	OF
Test - CalTech							



NOTE: NOT ALL TUBE AND DUCT WRAP ELEMENTS SHOWN

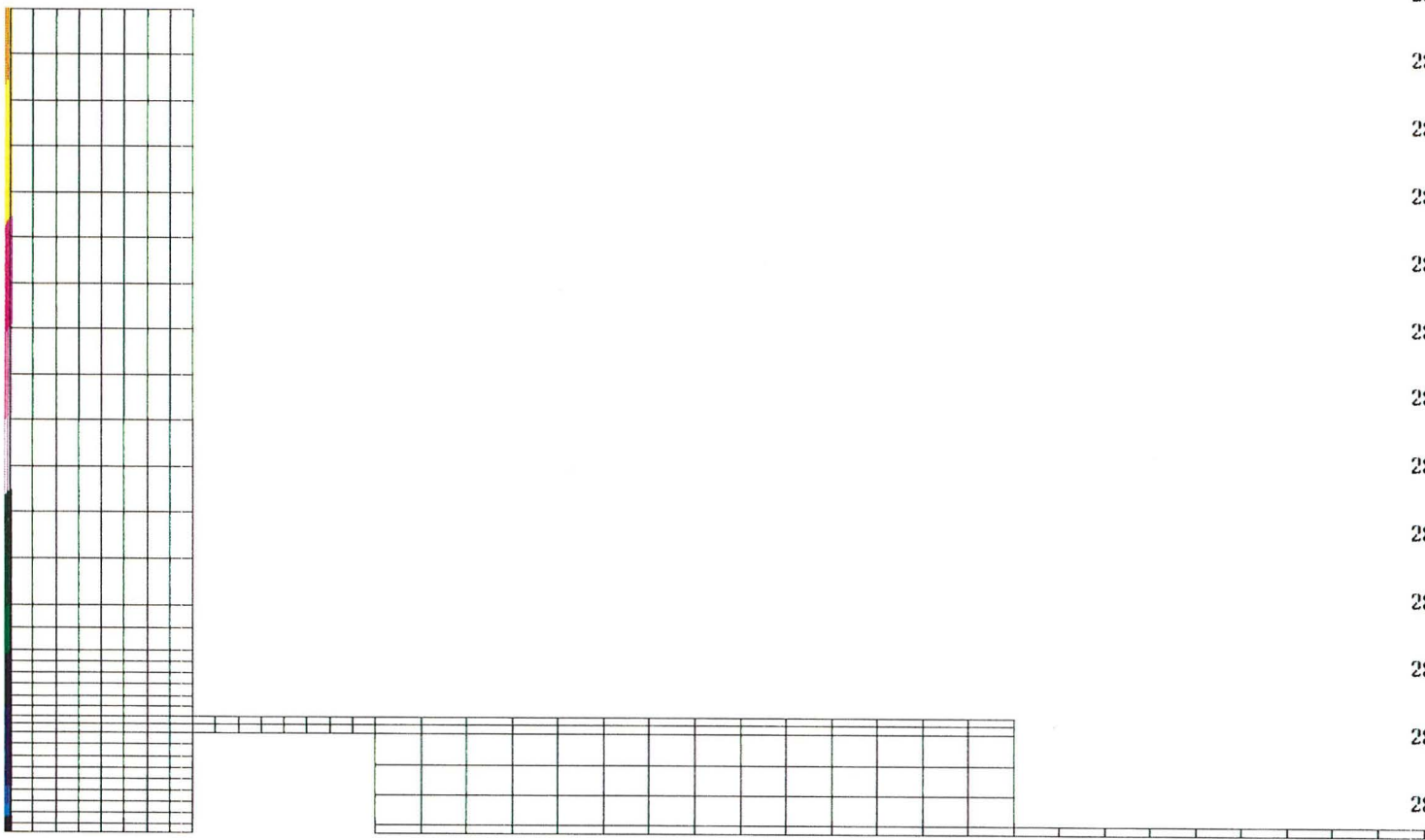


FIGURE 11
TRANSFER LUG MODEL - INITIAL TEMPERATURE DISTRIBUTION IN TUBE

SUBJECT		Ligo Beam Tube Design and Qualification	
MADE BY	DEP	CHKD BY	KAD
DATE	3/29/94	DATE	3/29/94
REV		REV	
MADE BY		CHKD BY	
DATE		DATE	
CHARGE NO.		930212	
SHEET		18	
OF			

Test - CalTech

PROJECT
Ligo Beam Tube Design and Qualification
Test - CalTech

MADE BY
DEP
DATE
3/29/94

CHKD BY
KAD
DATE
3/29/94

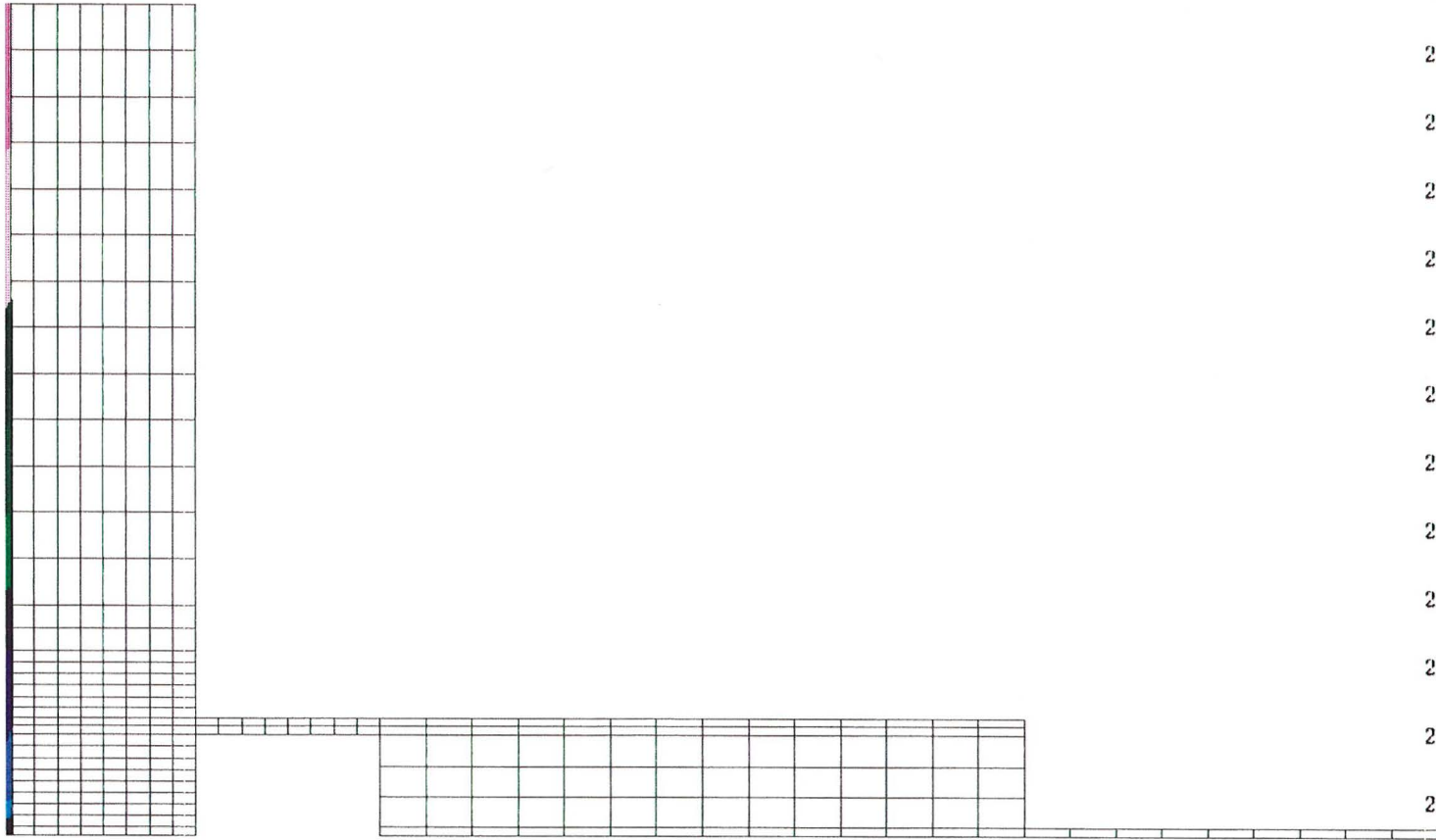
REV

MADE BY
DATE

CHKD BY
DATE

CHARGE NO.
930212
SHEET
19
OF

NOTE: NOT ALL TUBE AND DUCT WRAP ELEMENTS SHOWN



(DEG. F)
284.0
283.7
283.4
283.0
282.7
282.4
282.1
281.8
281.4
281.1
280.8
280.5
280.2
279.8
279.5
279.2

FIGURE 12
TRANSFER LUG MODEL - REFINED TEMPERATURE DISTRIBUTION IN TUBE

(DEG. F)

284.0

282.3

280.5

278.8

277.0

275.3

273.6

271.8

270.1

268.3

266.6

264.9

263.1

261.4

259.6

257.9

NOTE: NOT ALL TUBE AND DUCT WRAP ELEMENTS SHOWN

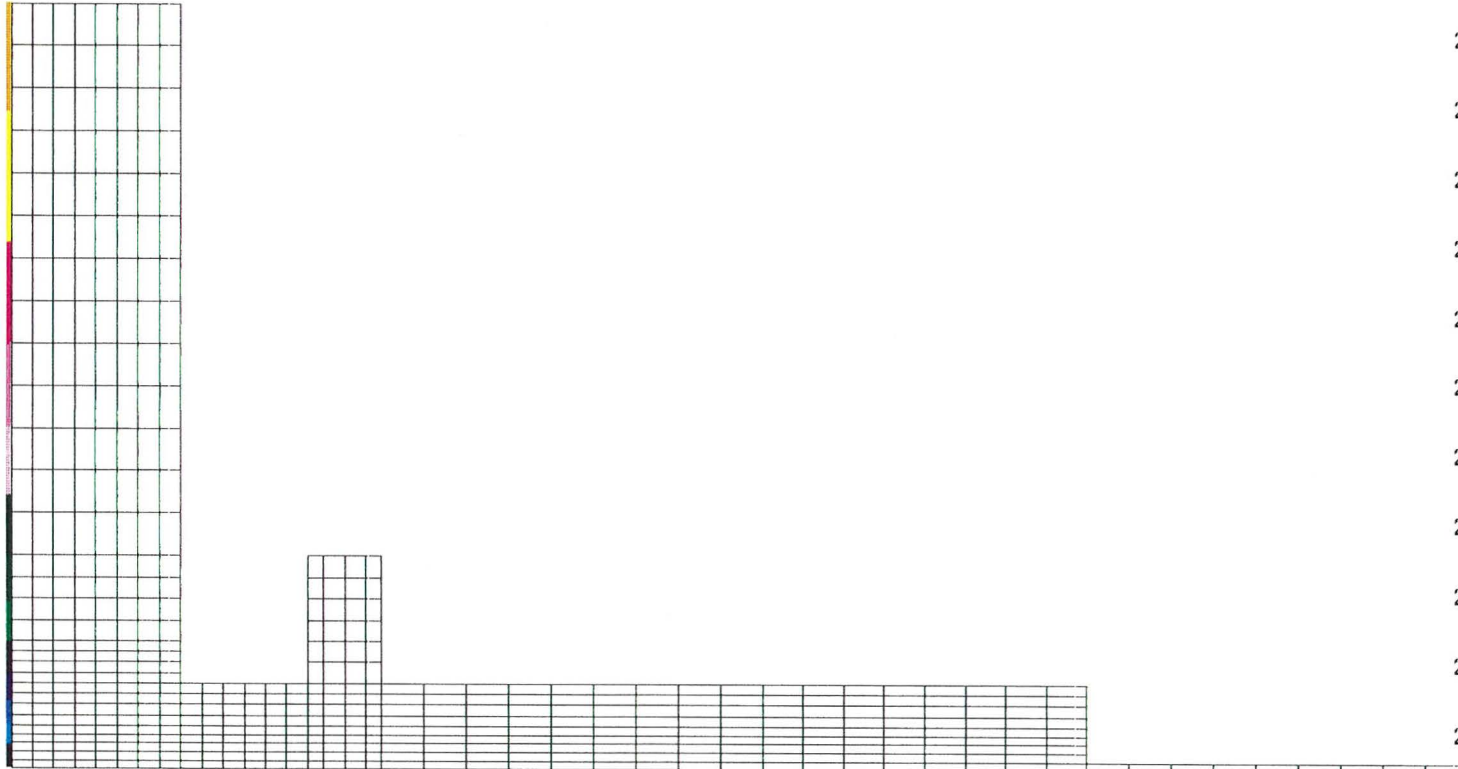


FIGURE 13
COLUMN SUPPORT MODEL - INITIAL TEMPERATURE DISTRIBUTION IN TUBE

SUBJECT Ligo Beam Tube Design and Qualification Test - CalTech	MADE BY DEP	CHKD BY KAD	REV	MADE BY	CHKD BY	CHARGE NO. 930212
	DATE 3/29/94	DATE 3/29/94		DATE	DATE	SHEET 20
						OF

SUBJECT		Ligo Beam Tube Design and Qualification		CHARGE NO.		930212	
Test - CalTech		MADE BY	DEP	CHKD BY	KAD	SHEET	21
	DATE	3/29/04	DATE	3/29/04		OF	
	REV						
	MADE BY			CHKD BY			
	DATE			DATE			

NOTE: NOT ALL TUBE AND DUCT WRAP ELEMENTS SHOWN

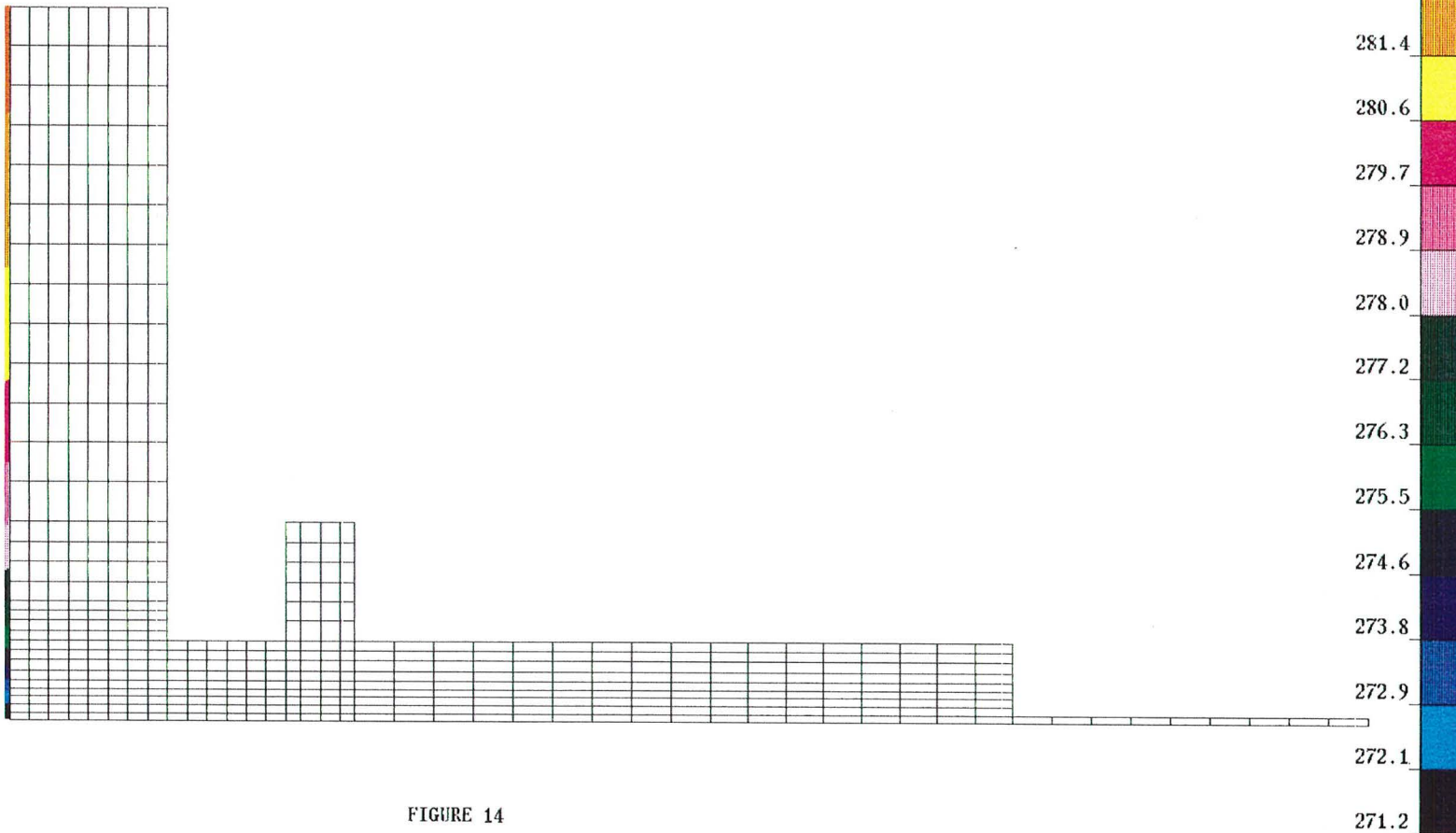
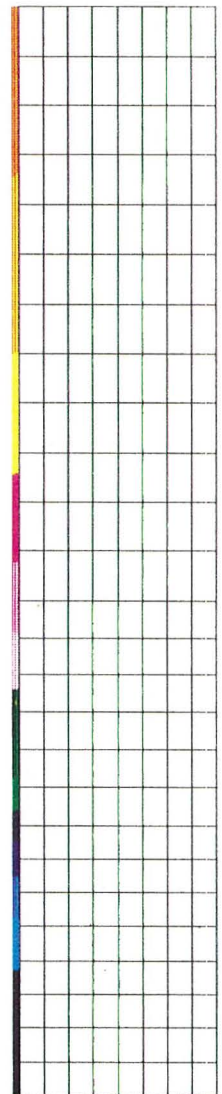


FIGURE 14
COLUMN SUPPORT MODEL - REFINED TEMPERATURE DISTRIBUTION IN TUBE

SUBJECT Ligo Beam Tube Design and Qualification		MADE BY DEP	CHKD BY RAD	REV	MADE BY	CHKD BY	CHARGE NO. 930212
Test - CalTech		DATE 3/89/94	DATE 8/29/94		DATE	DATE	OF
							SHEET 22

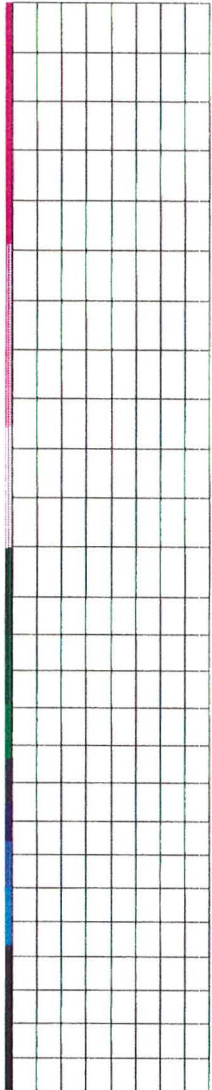


NOTE: NOT ALL TUBE AND DUCT WRAP ELEMENTS SHOWN

FIGURE 15
CONCENTRATED HEAT SINK MODEL - INITIAL TEMPERATURE DISTRIBUTION IN TUBE

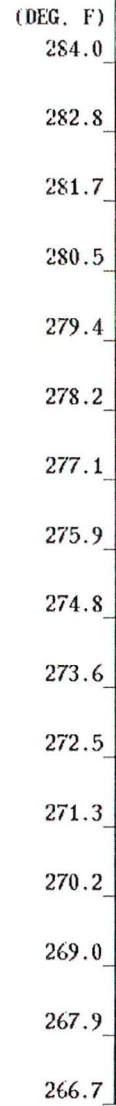
(DEG. F)
284.0
283.1
282.2
281.3
280.3
279.4
278.5
277.6
276.7
275.8
274.9
274.0
273.0
272.1
271.2
270.3

SUBJECT		Ligo Beam Tube Design and Qualification		CHARGE NO.		930212	
Test - CalTech		MADE BY		CHKD BY		SHEET	
		DEP		KAD		23	
		DATE		DATE		OF	
		3/29/94		3/29/94			
		REV		MADE BY		CHARGE NO.	
						930212	
		DATE		CHKD BY		SHEET	
						23	
						OF	



NOTE: NOT ALL TUBE AND DUCT WRAP ELEMENTS SHOWN

FIGURE 16
CONCENTRATED HEAT SINK MODEL - REFINED TEMPERATURE DISTRIBUTION IN TUBE





Location NOE - A

Table 1

Thermal Conductivities - Base Values

Temperature (Deg. F)	Stainless Steel 304L (BTU/hr in F)	Carbon Steel A36 (BTU/hr in F)	Duct Wrap (BTU/hr in F)
70.0	0.717	1.967	1.792E-03
75.0	-----	-----	1.792E-03
100.0	0.725	1.992	1.917E-03
125.0	-----	-----	2.069E-03
150.0	0.750	2.017	2.236E-03
175.0	-----	-----	2.458E-03
200.0	0.775	2.033	2.667E-03
225.0	-----	-----	2.958E-03
250.0	0.800	2.033	3.264E-03
275.0	-----	-----	3.625E-03
300.0	0.817	2.033	4.028E-03

Thermal Conductivity of Micarta
 (Assumed Independent of Temperature)

$k = 1.25E-02$ BTU/hr in F

SUBJECT Ligo Heat Transfer Analysis	MADE BY DEP	CHKD BY KAD	BY	CHARGE NO. 930212
	DATE 3/30/94	DATE 3/31/94	CHKD	
	DATE	DATE	SHT 24 OF	
CalTech				



Location NOE - A

Table 2

Thermal Conductivities - Smearing Factors

Transfer Lug Model

Component	Smearing Factor
Micarta	0.01768
Transfer Lug	0.03724
Horizontal Tube	0.00977
Support Lug	0.71772

Column Support Model

Component	Smearing Factor
Vertical Plates	0.01367
Horizontal Plate (SS)	0.02009
Horizontal Plate (CS)	0.01926
Micarta	0.01475
Column	0.00613
Horizontal Tube	0.00408
Support Lug	0.33447

SUBJECT Ligo Heat Transfer Analysis	MADE BY DEP	CHKD BY KAD	BY	CHARGE NO. 930212
	DATE 3/30/94	DATE 3/31/94	CHKD	
	DATE	DATE	DATE	



Location NOE - A

Table 3

Convective Film Coefficients - Base Values

Free Convection to Ambient Temperature

Temperature (Deg. F)	h (BTU/hr in ² F)
70.0	7.857E-06
75.0	2.385E-03
100.0	4.149E-03
125.0	4.957E-03
150.0	5.506E-03
175.0	5.924E-03
200.0	6.260E-03
225.0	6.541E-03
250.0	6.782E-03

Free Convection Plus Two Inches Duct Wrap Insulation

Temperature (Deg. F)	h (BTU/hr in ² F)
70.0	-----
75.0	6.513E-04
100.0	7.786E-04
125.0	8.559E-04
150.0	9.293E-04
175.0	1.018E-03
200.0	1.099E-03
225.0	1.206E-03
250.0	1.315E-03

SUBJECT Ligo Heat Transfer Analysis CalTech	MADE BY DEP	CHKD BY KAD	BY	CHARGE NO. 930212
	DATE 3/30/94	DATE 3/31/94	CHKD	SHT 26 OF
			DATE	



Location NOE - A

Table 4

Convective Film Coefficients - Smearing Factors

Transfer Lug Model

Component	Smearing Factor
Transfer Lug	0.08675
Horizontal Tube	0.06141
Support Lug	0.21955

Column Support Model

Component	Smearing Factor
Vertical Plates	0.10853
Horizontal Plate (SS)	0.00785
Horizontal Plate (CS)	0.00745
Micarta	0.06391
Column	0.03607
Horizontal Tube	0.02315
Support Lug	0.10284

SUBJECT Ligo Heat Transfer Analysis	MADE BY DEP	CHKD BY KAD	BY	CHARGE NO. 930212
	DATE 3/30/94	DATE 3/31/94	CHKD	
	CalTech		DATE	



Location NOE - A

Appendix A
Axisymmetric Smearing Factor Calculations

SUBJECT Ligo Heat Transfer Analysis	MADE BY DEP	CHKD BY KAD	REV	BY	CHARGE NO. 930212
	DATE 3/29/94	DATE 3/29/94		CHKD	
CalTech				DATE	

TRANSFER LUG MODEL

MICARTA

$$\text{VOLUME IN SUPPORT} = V_1' = 2'' \times 6'' \times 2'' \Rightarrow V_1' = 24 \text{ in}^3$$

$$\text{VOLUME PER 32.50' OF LIGO} = V_1'' = V_1'/2 = 24/2 \Rightarrow V_1'' = 12 \text{ in}^3$$

$$\text{VOLUME IN MODEL} = V_1 = \pi(R_o^2 - R_i^2)h$$

$$R_i = 26'' \quad R_o = 28'' \quad h = 2''$$

$$\Rightarrow V_1 = 678.58 \text{ in}^3$$

$$\text{SMEARING Factor} = R = V_1''/V_1 = \frac{12}{678.58}$$

$$\Rightarrow \underline{\underline{R = 0.01768}}$$

TRANSFER LUG

$$\text{VOLUME IN SUPPORT} = V_1' = (6'' \times \frac{3}{8}'' \times 9\frac{1}{2}'') + (4'' \times \frac{3}{8}'' \times 9\frac{1}{2}'')$$

$$\Rightarrow V_1' = 35.625 \text{ in}^3$$

$$\text{VOLUME PER 32.50' OF LIGO} = V_1'' = V_1'/2 \Rightarrow 35.625/2$$

$$\Rightarrow V_1'' = 17.813 \text{ in}^3$$

$$\text{VOLUME IN MODEL} = V_1 = \pi(R_o^2 - R_i^2)h$$

$$R_i = 25.5'' \quad R_o = 32.5'' \quad h = \frac{3}{8}''$$

$$\Rightarrow V_1 = 478.31 \text{ in}^3$$

$$\text{SMEARING Factor} = R = V_1''/V_1 = \frac{17.813}{478.31}$$

$$\Rightarrow \underline{\underline{R = 0.03724}}$$

SUBJECT LIGO HEAT TRANSFER ANALYSIS - CALTECH	OFFICE CBI NOE-A		REVISION		REFERENCE NO. 930212
	MADE BY DEP	CHKD BY KAD	MADE BY	CHKD BY	SHT <u>A2</u> OF ___
	DATE 3/29/94	DATE 3/29/94	DATE	DATE	

HORIZONTAL TUBE

VOLUME IN SUPPORT = $V_1' = A \cdot L$

$L = 2'-8" \Rightarrow L = 32"$

$A = (4^2 - 3.25^2) \Rightarrow A = 5.4375 \text{ in}^2$

$\Rightarrow V_1' = 174.0 \text{ in}^3$

VOLUME PER 32.50' OF LIGO = $V_1'' = V_1' / 2 = 174 / 2$

$\Rightarrow V_1'' = 87 \text{ in}^3$

VOLUME IN MODEL = $V_1 = \pi(R_o^2 - R_i^2)h$

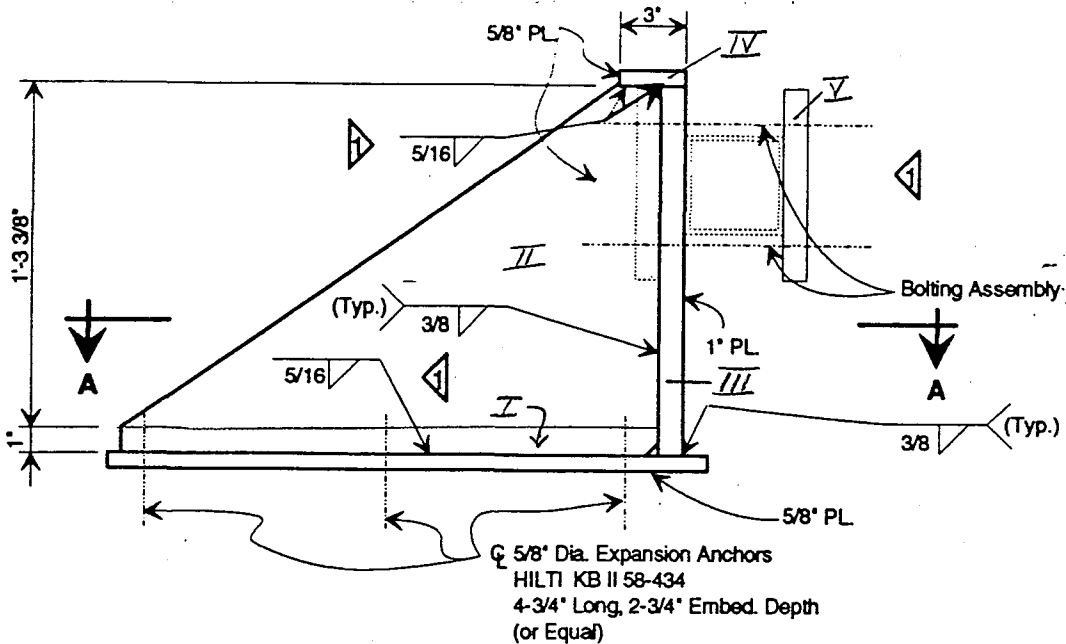
$R_i = 32.5" \quad R_o = 46.5" \quad h = 2 \frac{9}{16}"$

$\Rightarrow V_1 = 8903.67 \text{ in}^3$

SMEARING Factor = $R = V_1'' / V_1 = 87 / 8903.67$

$\Rightarrow \underline{\underline{R = 0.00977}}$

SUPPORT LUG



SUBJECT LIGO HEAT TRANSFER ANALYSIS - CAL TECH	OFFICE NOE-A		REVISION		REFERENCE NO. 930212
	MADE BY DEP	CHKD BY KAD	MADE BY	CHKD BY	SHT A3 OF
	DATE 3/29/94	DATE 3/29/94	DATE	DATE	

Area of I: $1'' \times 22\frac{1}{2}'' \Rightarrow A_1 = 22\frac{1}{2} \text{ in}^2$

Area of II: $\frac{1}{2} [15\frac{3}{8}''] [2'' + 22\frac{1}{2}''] \Rightarrow A_2 = 188.34375 \text{ in}^2$

Area of III: $1'' \times 16\frac{3}{8}'' \Rightarrow A_3 = 16.375 \text{ in}^2$

Area of IV: $3'' \times \frac{5}{8}'' \Rightarrow A_4 = 1.875 \text{ in}^2$

Area of V: $8'' \times 1'' \Rightarrow A_5 = 8 \text{ in}^2$

VOLUME OF SECTIONS I through V:

$t_1 = t_2 = \frac{5}{8}'' \quad t_4 = 6''$

$t_3 = 1''$

$t_5 = 3''$

$V_i = A_i \cdot t_i \Rightarrow V_1 = 14.0625 \text{ in}^3$

$V_2 = 117.7148 \text{ in}^3$

$V_3 = 16.375 \text{ in}^3$

$V_4 = 11.25 \text{ in}^3$

$V_5 = 24 \text{ in}^3$

VOLUME OF BOLTS = 1" DIAMETER

LENGTH (NOT INCLUDING PLATE OVERLAP)

$V_B = 9 \pi \text{ in}^3 \Rightarrow V_B = 28.2743 \text{ in}^3$

VOLUME PER 32.50' OF LIGO = $V_1'' =$

$2 * [V_1 + V_2 + V_3 + \frac{1}{2} V_4 + \frac{1}{2} V_5 + V_B] =$

$\Rightarrow V_1'' = 388.103 \text{ in}^3$

VOLUME IN MODEL = $V_1''' = \pi (R_o^2 - R_i^2) h$

$R_i = 46.5'' \quad R_o = 55.5'' \quad h = \frac{3}{16}''$

$\Rightarrow V_1''' = 540.75 \text{ in}^3$

SMEARING Factor = $R = \frac{V_1''}{V_1'''} = \frac{388.103}{540.75}$

$\Rightarrow R = \underline{\underline{0.71772}}$

SUBJECT LIGO HEAT TRANSFER ANALYSIS - CALTECH	OFFICE CBI NOE-A		REVISION		REFERENCE NO. 930212
	MADE BY DEP	CHKD BY KAD	MADE BY	CHKD BY	SHT 4 OF
	DATE 3/29/94	DATE 3/29/94	DATE	DATE	

TRANSFER LUG - EXPOSED SURFACE AREA

SURFACE AREA IN SUPPORT = E_1' =

$2(4" \times 9\frac{1}{2}") + 6" \times 9\frac{1}{2}" \Rightarrow E_1' = 133 \text{ in}^2$

SURFACE AREA per 32.50' of LIGO = $E_1 = E_1'/2$

$\Rightarrow E_1 = 66.5 \text{ in}^2$

SURFACE AREA IN MODEL = $E_1'' = \pi(R_o^2 - R_i^2)$

$R_i = 28.5" \quad R_o = 32.5"$

$\Rightarrow E_1'' = 766.55 \text{ in}^2$

SMEARING FACTOR = $R = E_1/E_1'' \Rightarrow \underline{\underline{R = 0.08675}}$

HORIZONTAL TUBE - EXPOSED SURFACE AREA

SURFACE AREA IN SUPPORT = $E_1' = 4[4" \times 32"]$

$\Rightarrow E_1' = 512 \text{ in}^2$

SURFACE AREA per 32.50' of LIGO = $E_1 = E_1'/2$

$\Rightarrow E_1 = 256 \text{ in}^2$

SURFACE AREA IN MODEL = $E_1'' = \pi(R_o^2 - R_i^2) + 2\pi(2\frac{3}{8})R_o$

$R_i = 32.5" \quad R_o = 46.5"$

$\Rightarrow E_1'' = 4168.50 \text{ in}^2$

SMEARING FACTOR = $R = E_1/E_1'' \Rightarrow \underline{\underline{R = 0.06141}}$

SUBJECT LIGO HEAT TRANSFER ANALYSIS - CALTECH	OFFICE CBI NOE-A		REVISION		REFERENCE NO. 930212
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	DATE 3/29/94	DATE 3/29/94	DATE	DATE	

SUPPORT LVG - EXPOSED SURFACE AREA

$$\begin{aligned} \text{SURFACE AREA IN SUPPORT} &= E_1' = \\ &4 \times 17\frac{5}{8}'' \times 6'' + 4 [A_1 + A_2] \end{aligned} \quad \begin{aligned} A_1 &= 22.5 \text{ in}^2 \\ A_2 &= 188.34375 \text{ in}^2 \end{aligned}$$

$$\Rightarrow E_1' = 1266.375 \text{ in}^2$$

$$\begin{aligned} \text{SURFACE AREA per 32.50' of LIGO} &= E_1 = E_1'/2 \\ \Rightarrow E_1 &= 633.1875 \text{ in}^2 \end{aligned}$$

$$\begin{aligned} \text{SURFACE AREA IN MODEL} &= E_1'' = \pi(R_o^2 - R_i^2) \\ R_i &= 46.5'' \quad R_o = 55.5'' \\ \Rightarrow E_1'' &= 2883.982 \text{ in}^2 \end{aligned}$$

$$\text{SMEARING Factor} = R = E_1/E_1'' \Rightarrow \underline{\underline{R = 0.21955}}$$

SUBJECT LIGO HEAT TRANSFER ANALYSIS - CALTECH	OFFICE CBI NOE-A		REVISION		REFERENCE NO. 930212
	MADE BY DEP	CHKD BY RAD	MADE BY	CHKD BY	SHT <u>16</u> OF <u> </u>
	DATE 3/29/94	DATE 3/29/94	DATE	DATE	

COLUMN SUPPORT MODEL

VERTICAL PLATES

$$\text{VOLUME IN SUPPORT} = V_1' = 2 \times \frac{3}{16}'' \times (2'' \times 1'' + \frac{1}{2} \times 7\frac{5}{8}'' \times (1'' + 2'')) \\ + 2 \times \frac{3}{8}'' \times 4'' \times 8\frac{5}{8}''$$

$$\Rightarrow V_1' = 30.9141$$

$$\text{VOLUME per 32.50' OF LIGO} = V_1'' = \frac{V_1'}{2}$$

$$\Rightarrow V_1'' = 15.4571$$

$$\text{VOLUME IN MODEL} = V_1 = \pi(R_o^2 - R_i^2)h$$

$$R_i = 28.5'' \quad R_o = 31.5'' \quad h = 2.0''$$

$$\Rightarrow V_1 = 1130.97 \text{ in}^3$$

$$\text{SMEARING Factor} = R = \frac{V_1''}{V_1} = \frac{15.4571}{1130.97}$$

$$\Rightarrow \underline{R = 0.01367}$$

HORIZONTAL PLATES

$$\text{VOLUME IN SUPPORT} = V_1' = 4'' \times 10'' \times \frac{3}{8}'' \Rightarrow V_1' = 15.0 \text{ in}^3$$

$$\text{VOLUME per 32.50' OF LIGO} = V_1'' = \frac{V_1'}{2}$$

$$\Rightarrow V_1'' = 7.50 \text{ in}^3$$

$$\text{VOLUME IN MODEL} = V_1 = \pi(R_o^2 - R_i^2)h$$

$$\text{PLATE 1: } R_i = 31.5'' \quad R_o = 31\frac{7}{8}'' \quad h = 5.0''$$

$$\text{PLATE 2: } R_i = 32\frac{7}{8}'' \quad R_o = 33\frac{1}{4}'' \quad h = 5.0''$$

$$\text{PLATE 1: } V_1 = 373.309 \text{ in}^3$$

$$\text{PLATE 2: } V_1 = 389.508 \text{ in}^3$$

$$\text{SMEARING Factor} = R = \frac{V_1''}{V_1}$$

$$\Rightarrow \text{PLATE 1: } \underline{R = 0.02009}$$

$$\text{PLATE 2: } \underline{R = 0.01926}$$

SUBJECT LIGO HEAT TRANSFER ANALYSIS - CALTECH	OFFICE GEB NOE-A		REVISION		REFERENCE NO. 930212
	MADE BY DEP	CHKD BY KAD	MADE BY	CHKD BY	SHT <u>17</u> OF <u> </u>
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MICARTA

VOLUME IN SUPPORT = $V_1' = 3'' \times 10'' \times 1'' \Rightarrow V_1' = 30 \text{ in}^3$

VOLUME per 32.50' OF LIGO = $V_1'' = V_1'/2 \Rightarrow V_1'' = 15 \text{ in}^3$

VOLUME IN MODEL = $V_1 = \pi(R_o^2 - R_i^2)h$

$R_i = 31\frac{7}{8}'' \quad R_o = 32\frac{7}{8}'' \quad h = 5.0''$

$\Rightarrow V_1 = 1017.09 \text{ in}^3$

SMEARING Factor = $R = V_1''/V_1$

$\Rightarrow \underline{\underline{R = 0.01475}}$

COLUMN

VOLUME IN SUPPORT = $V_1' = A \cdot L$

$L = 10'' - \frac{3}{8}'' \Rightarrow L = 9.625''$

$A = (4^2 - 3.25^2) \Rightarrow A = 5.4375 \text{ in}^2$

$\Rightarrow V_1' = 52.336 \text{ in}^3$

VOLUME per 32.50' OF LIGO = $V_1'' = V_1'/2$

$\Rightarrow V_1'' = 26.168 \text{ in}^3$

VOLUME IN MODEL $\Rightarrow V_1 = \pi(R_o^2 - R_i^2)h$

$R_i = 33\frac{1}{4}'' \quad R_o = 42\frac{1}{4}'' \quad h = 2.0''$

$\Rightarrow V_1 = 4269.424 \text{ in}^3$

SMEARING Factor = $R = V_1''/V_1 = \frac{26.168}{4269.42}$

$\Rightarrow \underline{\underline{R = 0.00613}}$

SUBJECT LIGO HEAT TRANSFER ANALYSIS - CALTECH	OFFICE CBI NOE-A		REVISION		REFERENCE NO. 930212
	MADE BY DEP	CHKD BY KAD	MADE BY	CHKD BY	SHT A8 OF —
	DATE 3/29/94	DATE 3/29/94	DATE	DATE	

HORIZONTAL TUBE

VOLUME IN SUPPORT = $V_1' = A \cdot L$

$L = \frac{1}{2} [(3' - 9\frac{3}{8}'') - (2' - 8'')] \Rightarrow L = 6.6875''$

$A = (4^2 - 3.25^2) \Rightarrow A = 5.4375''$

$\Rightarrow V_1' = 36.363 \text{ in}^3$

VOLUME per 32.50' of LIGO = $V_1'' = V_1' / 2$

$\Rightarrow V_1'' = 18.182 \text{ in}^3$

VOLUME OF MODEL = $V_1 = \pi(R_o^2 - R_i^2)h$

$R_i = 42\frac{1}{4}'' \quad R_o = 49\frac{15}{16}'' \quad h = 2.0''$

$\Rightarrow V_1 = 4452.839 \text{ in}^3$

SMEARING Factor = $R = V_1'' / V_1$

$\Rightarrow \underline{\underline{R = 0.00408}}$

SUPPORT LUG

Volume per 32.50' of LIGO = $\frac{1}{2} V_1''$ from
Support Lug from TRANSFER LUG MODEL.

$V_1'' = 193.052 \text{ in}^3$

VOLUME IN MODEL = $V = \pi(R_o^2 - R_i^2)h$

$R_i = 49\frac{15}{16}'' \quad R_o = 58\frac{15}{16}'' \quad h = \frac{3}{16}''$

$\Rightarrow V = 577.194 \text{ in}^3$

SMEARING Factor = $R = V_1'' / V = \frac{193.052}{577.194}$

$\Rightarrow \underline{\underline{R = 0.33447}}$

SUBJECT LIGO HEAT TRANSFER ANALYSIS - CALTECH	OFFICE CBI NOE-A		REVISION		REFERENCE NO. 930212
	MADE BY DEP	CHKD BY KAD	MADE BY	CHKD BY	SHT <u>A9</u> OF <u> </u>
	DATE 3/29/94	DATE 3/29/94	DATE	DATE	

VERTICAL PLATES - EXPOSED SURFACE AREA

$$\begin{aligned} \text{SURFACE AREA IN SUPPORT} &= E_1' \\ &= 4 [2'' \times 1'' + \frac{1}{2} \times 7\frac{5}{8}'' \times (2'' + 1'')] + 2 [1.4'' \times 8\frac{5}{8}''] \\ &\Rightarrow E_1' = 122.75 \text{ in}^2 \end{aligned}$$

$$\begin{aligned} \text{SURFACE AREA per 32.50' OF LIGO} &= E_1 = E_1'/2 \\ &\Rightarrow E_1 = 61.375 \text{ in}^2 \end{aligned}$$

$$\begin{aligned} \text{SURFACE AREA IN MODEL} &= E_1'' = \pi (R_o^2 - R_i^2) \\ R_i &= 28.5'' \quad R_o = 31.5'' \\ &\Rightarrow E_1'' = 565.487 \text{ in}^2 \end{aligned}$$

$$\begin{aligned} \text{SMEARING Factor} = R &= E_1/E_1'' = \frac{61.375}{565.487} \\ &\Rightarrow \underline{\underline{R = 0.10853}} \end{aligned}$$

HORIZONTAL PLATES - EXPOSED SURFACE AREA

$$\begin{aligned} \text{SURFACE AREA IN SUPPORT} &= E_1' \\ &= 2 [4'' \times \frac{3}{8}''] + 2 [10'' \times \frac{3}{8}''] \\ &\Rightarrow E_1' = 10.50 \text{ in}^2 \end{aligned}$$

$$\begin{aligned} \text{SURFACE AREA per 32.50' OF LIGO} &= E_1 \\ &= E_1'/2 \quad \Rightarrow E_1 = 5.25 \text{ in}^2 \end{aligned}$$

$$\begin{aligned} \text{VOLUME IN MODEL PLATE 1} &= E_1'' = \pi (R_o^2 - R_i^2) + 2\pi (3) R_i \\ \text{PLATE 2} &= E_1'' = \pi (R_o^2 - R_i^2) + 2\pi (3) R_o \end{aligned}$$

$$\begin{aligned} \text{PLATE 1} &: R_i = 31.5'' \quad R_o = 31\frac{7}{8}'' \quad E_1'' = 668.423 \text{ in}^2 \\ \text{PLATE 2} &: R_i = 32\frac{7}{8}'' \quad R_o = 33\frac{1}{4}'' \quad E_1'' = 704.649 \text{ in}^2 \end{aligned}$$

$$\text{SMEARING Factor} = R = E_1/E_1''$$

$$\Rightarrow \text{PLATE 1} : \underline{\underline{R = 0.00785}} \quad \text{PLATE 2} : \underline{\underline{R = 0.00745}}$$

SUBJECT LIGO HEAT TRANSFER ANALYSIS - CALTECH	OFFICE CBI NOE-A		REVISION		REFERENCE NO. 930212
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	DATE 3/29/94	DATE 3/29/94	DATE	DATE	

MICARTA - EXPOSED SURFACE AREA

$$\begin{aligned} \text{SURFACE AREA IN SUPPORT} &= E_1' \\ &= 2[3" \times 1"] + 2[10" \times 1"] \Rightarrow E_1' = 26 \text{ in}^2 \end{aligned}$$

$$\begin{aligned} \text{SURFACE AREA per } 32.50' \text{ OF LIGO} &= E_1 \\ &= E_1'/2 \Rightarrow E_1 = 13 \text{ in}^2 \end{aligned}$$

$$\begin{aligned} \text{SURFACE AREA IN MODEL} &= E_1'' = \pi(R_o^2 - R_i^2) \\ R_i &= 31\frac{7}{8}" \quad R_o = 32\frac{7}{8}" \\ &\Rightarrow E_1'' = 203.418 \text{ in}^2 \end{aligned}$$

$$\begin{aligned} \text{SMEARING Factor} &= R = E_1/E_1'' = 13/203.418 \\ &\Rightarrow \underline{\underline{R = 0.06391}} \end{aligned}$$

COLUMN - EXPOSED SURFACE AREA

$$\begin{aligned} \text{SURFACE AREA IN SUPPORT} &= E_1' \\ &= 4[4" \times 9\frac{5}{8}"] \Rightarrow E_1' = 154 \text{ in}^2 \end{aligned}$$

$$\begin{aligned} \text{SURFACE AREA per } 32.50' \text{ OF LIGO} &= E_1 \\ &= E_1'/2 \Rightarrow E_1 = 77 \text{ in}^2 \end{aligned}$$

$$\begin{aligned} \text{SURFACE AREA IN MODEL} &= E_1'' = \pi(R_o^2 - R_i^2) \\ R_i &= 33\frac{1}{4}" \quad R_o = 42\frac{1}{4}" \\ &\Rightarrow E_1'' = 2134.712 \text{ in}^2 \end{aligned}$$

$$\begin{aligned} \text{SMEARING Factor} &= R = E_1/E_1'' = 77/2134.712 \\ &\Rightarrow \underline{\underline{R = 0.03607}} \end{aligned}$$

SUBJECT LIGO HEAT TRANSFER ANALYSIS - CALTECH	OFFICE CBI NOE-A		REVISION		REFERENCE NO. 930212
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HORIZONTAL TUBE - EXPOSED SURFACE AREA

$$\text{SURFACE AREA IN SUPPORT} = E_1' = 4 [4'' \times 6.6875''] \\ \Rightarrow E_1' = 107 \text{ in}^2$$

$$\text{SURFACE AREA per 32.50' OF LIGO} = E_1 = E_1' / 2 \\ \Rightarrow E_1 = 53.5 \text{ in}^2$$

$$\text{SURFACE AREA IN MODEL} = E_1'' = \pi (R_o^2 - R_i^2) + 1^{13/16} R_o \\ R_i = 42^{1/4}'' \quad R_o = 49^{15/16}'' \\ \Rightarrow E_1'' = 2310.689 \text{ in}^2$$

$$\text{SMEARING Factor} = R = E_1 / E_1'' = \frac{53.5}{2310.689}$$

$$\Rightarrow \underline{\underline{R = 0.02315}}$$

SUPPORT LUG - EXPOSED SURFACE AREA

SURFACE AREA per 32.50' OF LIGO: $1/2 E_1$, from Support Lug from Transfer Lug model.

$$\Rightarrow E_1 = 316.594 \text{ in}^2$$

$$\text{SURFACE AREA IN MODEL} = E_1'' = \pi (R_o^2 - R_i^2) \\ R_i = 49^{15/16}'' \quad R_o = 58^{15/16}'' \\ \Rightarrow E_1'' = 3078.37 \text{ in}^2$$

$$\text{SMEARING Factor} = R = E_1 / E_1'' = \frac{316.594}{3078.37}$$

$$\Rightarrow \underline{\underline{R = 0.10284}}$$

SUBJECT LIGO HEAT TRANSFER ANALYSIS - CALTECH	OFFICE CBI NOE-A		REVISION		REFERENCE NO. 930212
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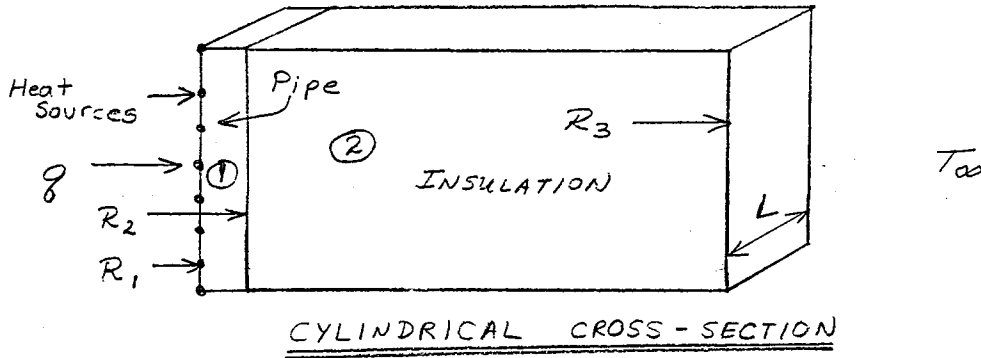
Location NOE - A

Appendix B

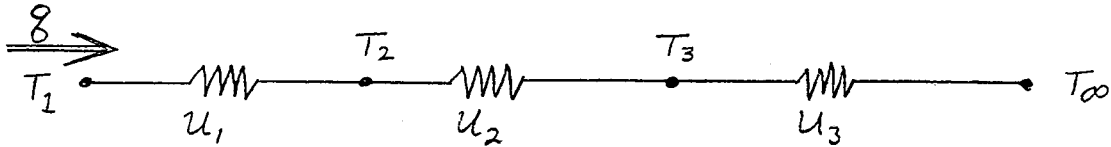
Heat Source and Free Convection Coefficient Calculations

SUBJECT Ligo Heat Transfer Analysis	MADE BY DEP	CHKD BY KAD	REV	BY		CHARGE NO. 930212
	DATE 3/29/94	DATE 3/29/94		CHKD		
	CalTech				DATE	

ONE - D CALCULATION OF ELECTRICAL HEAT INPUT =



ELECTRICAL ANALOGY



U_1 = Conduction through pipe
 U_2 = Conduction through insulation
 U_3 = Free convection from insulation

$$U_1 = \frac{\ln[R_2/R_1]}{2\pi K_1} \quad U_2 = \frac{\ln[R_3/R_2]}{2\pi K_2}$$

$$U_3 = \frac{1}{h 2\pi R_3}$$

$$q = \frac{(T_1 - T_\infty)L}{U_1 + U_2 + U_3}$$

For PATRAN INPUT = $q/L = \frac{T_1 - T_\infty}{U_1 + U_2 + U_3}$

$$q/L = \frac{T_1 - T_3}{U_1 + U_2}$$

SUBJECT LIGO HEAT TRANSFER ANALYSIS - CAL TECH	OFFICE CBI NOE - A		REVISION		REFERENCE NO. 930212
	MADE BY DEP	CHKD BY KAD	MADE BY	CHKD BY	SHT <u>B2</u> OF <u> </u>
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$$\frac{q}{L} = \frac{T_1 - T_\infty}{\frac{\ln[R_2/R_1]}{2\pi K_1} + \frac{\ln[R_3/R_2]}{2\pi K_2} + \frac{1}{h 2\pi R_3}} \Rightarrow \textcircled{A}$$

$$\frac{q}{L} = \frac{T_1 - T_3}{\frac{\ln[R_2/R_1]}{2\pi K_1} + \frac{\ln[R_3/R_2]}{2\pi K_2}}$$

$$T_3 = -\frac{q}{L} \left[\frac{\ln[R_2/R_1]}{2\pi K_1} + \frac{\ln[R_3/R_2]}{2\pi K_2} \right] + T_1 \Rightarrow \textcircled{B}$$

IN EQUATION \textcircled{A} ONLY UNKNOWN IS h .
 K_1 and K_2 assumed independent of temperature

$$\bar{N}_{u,D} = \frac{hD}{K} \Rightarrow D = 2R_3 \Rightarrow h = \frac{\bar{N}_{u,D} K_3}{2R_3} \Rightarrow \textcircled{C}$$

K_3 for air

$$\bar{N}_{u,D} = \left[0.60 + \frac{0.387 Ra_D^{1/6}}{\left[1 + (0.559/Pr)^{9/16} \right]^{8/27}} \right]^2 \quad (10^{-5} < Ra_D < 10^{12}) \Rightarrow \textcircled{D}$$

$$Ra_D = \frac{g\beta(T_3 - T_\infty)(2R_3)^3}{\nu\alpha} \Rightarrow \textcircled{E}$$

IN EQUATION \textcircled{E} ONLY UNKNOWN IS T_3

SOLUTION

- 1) ASSUME T_3
- 2) Solve \textcircled{E} for Ra_D
- 3) Solve \textcircled{D} for $\bar{N}_{u,D}$
- 4) Solve \textcircled{C} for h
- 5) Solve \textcircled{A} for q/L
- 6) Solve \textcircled{B} for T_3
- 7) Repeat 1) through 6) until convergence of T_3 is reached.

Ref: FUNDAMENTS OF HEAT TRANSFER
 F.P. INCROPERA, D.P. DEWITT, 1981 Edition
 Pages 80-82, 447-449

SUBJECT LIGO HEAT TRANSFER ANALYSIS CALTECH	OFFICE CBI NOE - A	REVISION		REFERENCE NO. 930212
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	DATE 3/29/94	DATE 3/29/94	DATE	DATE
				SHT <u>B3</u> OF <u> </u>

K OF INSULATION IS A FUNCTION OF TEMPERATURE

INSULATION = 1.5 PCF

DATA FROM TECHNICAL DATA

T (°F)	K (BTU in/hr ft ² °F)	K (BTU/in hr °F)
* 0	0.220	1.528 · 10 ⁻³
* 25	0.228	1.583 · 10 ⁻³
50	0.240	1.667 · 10 ⁻³
75	0.258	1.792 · 10 ⁻³
100	0.276	1.917 · 10 ⁻³
125	0.298	2.069 · 10 ⁻³
150	0.322	2.236 · 10 ⁻³
175	0.354	2.458 · 10 ⁻³
200	0.384	2.667 · 10 ⁻³
* 225	0.426	2.958 · 10 ⁻³
* 250	0.470	3.264 · 10 ⁻³
* 275	0.522	3.625 · 10 ⁻³
* 300	0.580	4.028 · 10 ⁻³

* VALUES ARE EXTRAPOLATED FROM CURVE.

K OF 55304L

$$250^{\circ}\text{F} \rightarrow K = 9.6 \frac{\text{BTU}}{\text{hr ft}^2 \text{ }^{\circ}\text{F}} \Rightarrow K = 0.800 \frac{\text{BTU}}{\text{hr in } ^{\circ}\text{F}}$$

$$300^{\circ}\text{F} \rightarrow K = 9.8 \frac{\text{BTU}}{\text{hr ft}^2 \text{ }^{\circ}\text{F}} \Rightarrow K = 0.817 \frac{\text{BTU}}{\text{hr in } ^{\circ}\text{F}}$$

$$K(284^{\circ}\text{F}) = 0.811 \frac{\text{BTU}}{\text{hr in } ^{\circ}\text{F}} \therefore K_1 = 0.811 \frac{\text{BTU}}{\text{hr in } ^{\circ}\text{F}}$$

GEOMETRY KNOWN DATA :

$$R_1 = 24.375''$$

$$R_2 = 24.500''$$

$$R_3 = 28.500''$$

SUBJECT LIGO HEAT TRANSFER ANALYSIS	OFFICE CBI NOE-A		REVISION		REFERENCE NO. 930212
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VISCOSITY OF AIR $\nu =$

<u>T (K)</u>	<u>$\nu \cdot 10^6 (m^2/s)$</u>	<u>T (°F)</u>	<u>$\nu (in^2/hr)$</u>
250	11.44	-9.67	63.835
300	15.89	80.33	88.666
350	20.92	170.33	116.734
400	26.41	260.33	147.368

$$1 m^2/s = 5.58 \cdot 10^6 in^2/hr$$

VISCOSITY OF AIR $\alpha =$

<u>T (K)</u>	<u>$\alpha \cdot 10^6 (m^2/s)$</u>	<u>T (°F)</u>	<u>$\alpha (in^2/hr)$</u>
250	15.90	-9.67	88.722
300	22.50	80.33	125.550
350	29.90	170.33	166.842
400	38.30	260.33	213.714

$$g = 32.2 ft/s^2 \Rightarrow 5.007744 \cdot 10^9 in/hr^2$$

$$\beta = \frac{1}{T} \quad (T \text{ in } ^\circ R)$$

PRANDTL NUMBER $Pr = \frac{\nu}{\alpha} = \frac{\nu}{\alpha}$

CONDUCTIVITY OF AIR $K_3 =$

<u>T (K)</u>	<u>$K \cdot 10^3 (W/mK)$</u>	<u>T (°F)</u>	<u>$K_3 (BTU/hr in ^\circ F)$</u>
250	22.3	-9.67	$1.074 \cdot 10^{-3}$
300	26.3	80.33	$1.266 \cdot 10^{-3}$
350	30.0	170.33	$1.445 \cdot 10^{-3}$
400	33.8	260.33	$1.628 \cdot 10^{-3}$

$$1 W/mK = 4.81517 \cdot 10^{-2} BTU/hr in ^\circ F$$

SUBJECT LIGO HEAT TRANSFER ANALYSIS - CALTECH	OFFICE CBI NOE-A		REVISION		REFERENCE NO. 930212
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$$T_A = \frac{T_3 + T_\infty}{2}$$

$$\text{Let } T_2 = T_1$$

$$T_B = \frac{T_2 + T_3}{2}$$

$$K_2 = K(T_B)$$

BEGIN ITERATION PROCESS

$$T_\infty = 70^\circ\text{F} \quad T_1 = 284^\circ\text{F}$$

$$\text{Let } \delta_1 = 0.387 (Ra_D)^{1/6}$$

$$\delta_2 = [1 + (0.559/Pr)^{9/16}]^{8/27}$$

$$\therefore \bar{Nu}_D = [0.60 + \frac{\delta_1}{\delta_2}]^2$$

ITERATION RESULTS ON SPREADSHEET

$$\text{Eq. (A)} = Q = \delta/L$$

$$Q = \frac{T_1 - T_\infty}{U_1 + U_2 + U_3}$$

$$\text{Eq. (B)} \quad T_3 = -Q [U_1 + U_2] + T_1$$

SUBJECT LIGO HEAT TRANSFER ANALYSIS - CALTECH	OFFICE CBI NOE-A		REVISION		REFERENCE NO. 930212
	MADE BY DEP	CHKD BY KAD	MADE BY	CHKD BY	SHT <u>B6</u> OF ___
	DATE 3/29/94	DATE 3/29/94	DATE	DATE	

Heat Source Input Calculations

Assumed Conditions

Ambient Temperature = 70.00 F
 Tube Temperature T1 = 284.00 F
 Initial Estimate of T3 = 100.00 F

<u>Iteration 1</u>	<u>Iteration 2</u>	<u>Iteration 3</u>
T3 = 100.00 F	T3 = 97.16 F	T3 = 97.77 F
Ta = 85.00 F	Ta = 83.58 F	Ta = 83.88 F
v = 90.122 in ² /hr	v = 89.680 in ² /hr	v = 89.774 in ² /hr
α = 127.693 in ² /hr	α = 127.041 in ² /hr	α = 127.180 in ² /hr
Pr = 0.7058	Pr = 0.7059	Pr = 0.7059
β = 1.836E-03 1/R	β = 1.841E-03 1/R	β = 1.840E-03 1/R
RaD = 4.439E+09	RaD = 4.070E+09	RaD = 4.149E+09
γ1 = 15.689	γ1 = 15.463	γ1 = 15.513
γ2 = 1.205	γ2 = 1.205	γ2 = 1.205
NuD = 185.458	NuD = 180.408	NuD = 181.520
Tb = 192.000 F	Tb = 190.581 F	Tb = 190.883 F
K2 = 2.600E-03 BTU/hr in F	K2 = 2.588E-03 BTU/hr in F	K2 = 2.591E-03 BTU/hr in F
K3 = 1.275E-03 BTU/hr in F	K3 = 1.272E-03 BTU/hr in F	K3 = 1.273E-03 BTU/hr in F
h = 4.149E-03 BTU/hr in ² F	h = 4.027E-03 BTU/hr in ² F	h = 4.054E-03 BTU/hr in ² F
U1 = 0.001 hr in F/BTU	U1 = 0.001 hr in F/BTU	U1 = 0.001 hr in F/BTU
U2 = 9.257 hr in F/BTU	U2 = 9.299 hr in F/BTU	U2 = 9.290 hr in F/BTU
U3 = 1.346 hr in F/BTU	U3 = 1.387 hr in F/BTU	U3 = 1.377 hr in F/BTU
Q = 20.181 BTU/hr in	Q = 20.024 BTU/hr in	Q = 20.059 BTU/hr in
T3 = 97.16 F	T3 = 97.77 F	T3 = 97.63 F

SUBJECT LIGO HEAT TRANSFER ANALYSIS - CALTECH	OFFICE NOE-A	REVISION		REFERENCE NO. 930212	
	MADE BY DEP	CHKD BY KAD	MADE BY	CHKD BY	37 SHT ___ OF ___
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Heat Source Input Calculations

Assumed Conditions

Ambient Temperature = 70.00 F
 Tube Temperature T1 = 284.00 F
 Initial Estimate of T3 = 100.00 F

	<u>Iteration 4</u>	<u>Iteration 5</u>	<u>Iteration 6</u>
T3 =	97.63 F	97.66 F	97.65 F
Ta =	83.81 F	83.83 F	83.83 F
v =	89.753 in ² /hr	89.758 in ² /hr	89.756 in ² /hr
α =	127.149 in ² /hr	127.156 in ² /hr	127.154 in ² /hr
Pr =	0.7059	0.7059	0.7059
β =	1.840E-03 1/R	1.840E-03 1/R	1.840E-03 1/R
RaD =	4.131E+09	4.135E+09	4.134E+09
γ1 =	15.502	15.505	15.504
γ2 =	1.205	1.205	1.205
NuD =	181.271	181.327	181.314
Tb =	190.815 F	190.830 F	190.827 F
K2 =	2.590E-03 BTU/hr in F	2.590E-03 BTU/hr in F	2.590E-03 BTU/hr in F
K3 =	1.273E-03 BTU/hr in F	1.273E-03 BTU/hr in F	1.273E-03 BTU/hr in F
h =	4.048E-03 BTU/hr in ² F	4.050E-03 BTU/hr in ² F	4.049E-03 BTU/hr in ² F
U1 =	0.001 hr in F/BTU	0.001 hr in F/BTU	0.001 hr in F/BTU
U2 =	9.292 hr in F/BTU	9.292 hr in F/BTU	9.292 hr in F/BTU
U3 =	1.379 hr in F/BTU	1.379 hr in F/BTU	1.379 hr in F/BTU
	Q = 20.051 BTU/hr in	Q = 20.053 BTU/hr in	Q = 20.052 BTU/hr in
T3 =	97.66 F	97.65 F	97.65 F

Q (Unit Length) = 20.052 BTU/hr in

Q (Per 65' Length) = 15640.0 BTU/hr

SUBJECT LIGO HEAT TRANSFER ANALYSIS - CALTECH	OFFICE NOE-A		REVISION		REFERENCE NO. 9302/2	
	MADE BY DEP	CHKD BY KAD	MADE BY	CHKD BY	SHT 38 OF ___	
	DATE 3/29/94	DATE 3/29/94	DATE	DATE		

Free Convection Coefficient Calculations

Assumed Conditions

Ambient Temperature = 70.00 F
 Tube Temperature T1 = 284.00 F

T3 = 70.00 F	T3 = 75.00 F	T3 = 100.00 F
Ta = 70.00 F	Ta = 72.50 F	Ta = 85.00 F
v = 85.816 in ² /hr	v = 86.506 in ² /hr	v = 90.122 in ² /hr
α = 121.323 in ² /hr	α = 122.346 in ² /hr	α = 127.693 in ² /hr
Pr = 0.7073	Pr = 0.7071	Pr = 0.7058
β = 1.888E-03 1/R	β = 1.879E-03 1/R	β = 1.836E-03 1/R
RaD = 0.000E+00	RaD = 8.233E+08	RaD = 4.439E+09
γ1 = 0.000	γ1 = 11.848	γ1 = 15.689
γ2 = 1.205	γ2 = 1.205	γ2 = 1.205
NuD = 0.360	NuD = 108.838	NuD = 185.458
K3 = 1.244E-03 BTU/hr in F	K3 = 1.249E-03 BTU/hr in F	K3 = 1.275E-03 BTU/hr in F
h = 7.857E-06 BTU/hr in ² F	h = 2.385E-03 BTU/hr in ² F	h = 4.149E-03 BTU/hr in ² F
T3 = 125.00 F	T3 = 150.00 F	T3 = 175.00 F
Ta = 97.50 F	Ta = 110.00 F	Ta = 122.50 F
v = 94.021 in ² /hr	v = 97.919 in ² /hr	v = 101.817 in ² /hr
α = 133.428 in ² /hr	α = 139.163 in ² /hr	α = 144.898 in ² /hr
Pr = 0.7047	Pr = 0.7036	Pr = 0.7027
β = 1.795E-03 1/R	β = 1.755E-03 1/R	β = 1.718E-03 1/R
RaD = 7.297E+09	RaD = 9.557E+09	RaD = 1.134E+10
γ1 = 17.044	γ1 = 17.828	γ1 = 18.343
γ2 = 1.205	γ2 = 1.205	γ2 = 1.206
NuD = 217.303	NuD = 236.850	NuD = 250.127
K3 = 1.300E-03 BTU/hr in F	K3 = 1.325E-03 BTU/hr in F	K3 = 1.350E-03 BTU/hr in F
h = 4.957E-03 BTU/hr in ² F	h = 5.506E-03 BTU/hr in ² F	h = 5.924E-03 BTU/hr in ² F

SUBJECT LIGO HEAT TRANSFER ANALYSIS - CALTECH	OFFICE CBI NOE-A	REVISION		REFERENCE NO. 930212
	MADE BY DEP	CHKD BY KAD	MADE BY	CHKD BY
	DATE 3/29/94	DATE 3/29/94	DATE	DATE
				SHT 39 OF ___

Free Convection Coefficient Calculations

Assumed Conditions

Ambient Temperature = 70 F
 Tube Temperature T1 = 284 F

T3 = 200.00 F	T3 = 225.00 F	T3 = 250.00 F
Ta = 135.00 F	Ta = 147.50 F	Ta = 160.00 F
v = 105.716 in ² /hr	v = 109.614 in ² /hr	v = 113.512 in ² /hr
α = 150.633 in ² /hr	α = 156.368 in ² /hr	α = 162.103 in ² /hr
Pr = 0.7018	Pr = 0.7010	Pr = 0.7003
β = 1.682E-03 1/R	β = 1.647E-03 1/R	β = 1.614E-03 1/R
RaD = 1.273E+10	RaD = 1.381E+10	RaD = 1.464E+10
γ1 = 18.701	γ1 = 18.956	γ1 = 19.141
γ2 = 1.206	γ2 = 1.206	γ2 = 1.206
NuD = 259.557	NuD = 266.392	NuD = 271.373
K3 = 1.375E-03 BTU/hr in F	K3 = 1.400E-03 BTU/hr in F	K3 = 1.424E-03 BTU/hr in F
h = 6.260E-03 BTU/hr in ² F	h = 6.541E-03 BTU/hr in ² F	h = 6.782E-03 BTU/hr in ² F

SUBJECT LIGO HEAT TRANSFER ANALYSIS - CALTECH	OFFICE <u>NOE-A</u>		REVISION		REFERENCE NO. 930212
	MADE BY <u>DEP</u>	CHKD BY <u>KAD</u>	MADE BY	CHKD BY	SHT <u>310</u> OF <u> </u>
	DATE 3/29/94	DATE 3/29/94	DATE	DATE	



Location NOE - A

Appendix C
Miscellaneous Calculations

SUBJECT Ligo Heat Transfer Analysis	MADE BY <i>DEP</i>	CHKD BY <i>KAD</i>	REV	BY	CHARGE NO. 930212
	DATE <i>3/29/94</i>	DATE <i>3/29/94</i>		CHKD	
CalTech				DATE	SHT C1 OF

EQUIVALENT CONVECTIVE FILM COEFFICIENTS

CONVECTION RESISTANCE = $\frac{1}{h}$

CONDUCTION RESISTANCE = $\frac{L}{k}$

TOTAL RESISTANCE = $\frac{L}{k} + \frac{1}{h}$

INSULATION = DUCT WRAP

@ 250 °F $k = 3.264 \times 10^{-3} \text{ BTU/hr in } ^\circ\text{F}$
 Free Convection $h = 3.500 \times 10^{-3} \text{ BTU/hr in } ^2 \text{ } ^\circ\text{F}$

L = INSULATION THICKNESS

U = TOTAL RESISTANCE

$$U = \frac{1}{h} + \frac{L}{k} = \frac{1}{H}$$

H = EQUIVALENT CONVECTIVE FILM COEFFICIENT.

$$U = \frac{1}{3.500 \cdot 10^{-3}} + \frac{2}{3.264 \cdot 10^{-3}} \Rightarrow U = 898.459 \frac{\text{hr} \cdot \text{in}^2 \cdot ^\circ\text{F}}{\text{BTU}}$$

$$H = \frac{1}{898.4593} \Rightarrow H = 1.113016 \cdot 10^{-3} \text{ BTU/hr in } ^2 \text{ } ^\circ\text{F}$$

(Thickness of Insulation = 2.0")

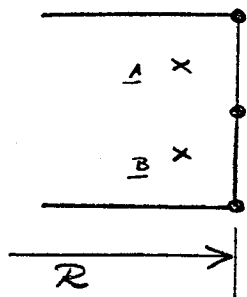
TABLE OF EQUIVALENT COEFFICIENTS (L = 2.0")

T (°F)	H (BTU/hr · in ² · °F)	T (°F)	H (BTU/hr · in ² · °F)
75	$6.513 \cdot 10^{-4}$	175	$1.018 \cdot 10^{-3}$
100	$7.786 \cdot 10^{-4}$	200	$1.099 \cdot 10^{-3}$
125	$8.559 \cdot 10^{-4}$	225	$1.206 \cdot 10^{-3}$
150	$9.298 \cdot 10^{-4}$	250	$1.315 \cdot 10^{-3}$

(Correction Factor R NOT INCLUDED)

SUBJECT LIGO HEAT TRANSFER ANALYSIS - CALTECH	OFFICE CEI NOE-A		REVISION		REFERENCE NO. 930212
	MADE BY DEP	CHKD BY KAD	MADE BY	CHKD BY	SHT C2 OF <u> </u>
	DATE 3/29/94	DATE 3/29/94	DATE	DATE	

CONCENTRATED HEAT SINK CALCULATION



Heat flux results = Column Support Model

$$A = q = 1.41198 \text{ BTU/hr in}^2$$

$$B = q = 1.38885 \text{ BTU/hr in}^2$$

$$\text{Average} = q = 1.40042 \text{ BTU/hr in}^2$$

$$Q = q \cdot t \cdot R$$

$$t = 3/16''$$

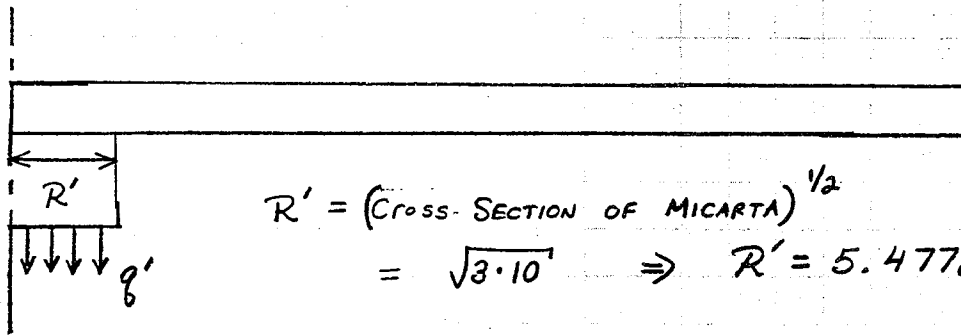
$$R = 26.25''$$

@ Location of Nodes.

$$Q = 1.40042 \times 3/16 \times 26.25$$

$$\Rightarrow Q = 6.8927 \frac{\text{BTU}}{\text{hr}}$$

IN CONCENTRATED HEAT SINK MODEL



$$R' = (\text{CROSS-SECTION OF MICARTA})^{1/2} = \sqrt{3 \cdot 10} \Rightarrow R' = 5.4772 \text{ in}$$

$$q' = \frac{q \cdot t \cdot R}{\pi (R')^2} \Rightarrow q' = \frac{Q}{\pi (R')^2} = \frac{6.8927}{\pi (30)}$$

$$\Rightarrow \underline{\underline{q' = 0.0731 \text{ BTU/hr in}^2}}$$

SUBJECT LIGO HEAT TRANSFER ANALYSIS - CALTECH	OFFICE CBI NOE-A		REVISION		REFERENCE NO. 930212
	MADE BY DEP	CHKD BY KAD	MADE BY	CHKD BY	SHT C3 OF
	DATE 3/29/94	DATE 3/29/94	DATE	DATE	